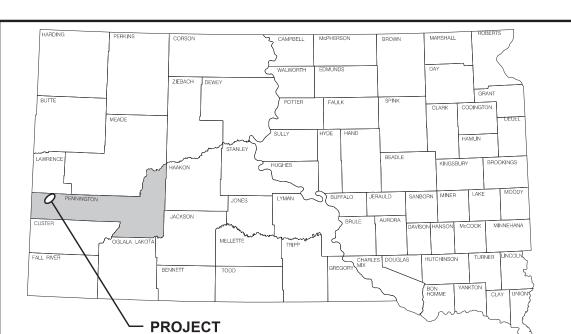
FOR RIDDING PURPOSES ONLY



STATE OF SOUTH DAKOTA
DEPARTMENT OF TRANSPORTATION
PLANS FOR PROPOSED

PROJECT P 6403(10) PENNINGTON COUNTY

GRADING, BASE COURSE, DRAINAGE, & ASPHALT CONCRETE SURFACING PLANS PCN 08P3

23 24

WHITE TAIL PEAK

FLAG M' ELEV. 6937

NIPPLE BUTTE 12

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| STATE OF | SOUTH | DAKOTA | P 6403(10) | 1 333

Sheet 1 - 1 Title Sheet

Plotting Date: 2/6/2023

Revision Date: 9/7/2022 Revision Date: 1/6/2023 Revision Date: 2/6/2023

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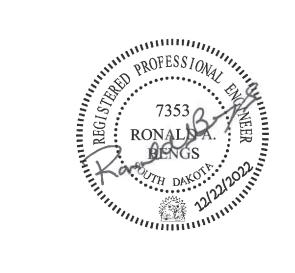
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END P 6403(10)

SOUTH ROCHFORD ROAD Station 534+30.81 1,289.29 feet South and 889.32 feet East of the Northwest corner of Section 23 -Township 2 North - Range 3 East





R 1 E

 α

13

24

25

36

13

24

25

19

29

CROW'S NEST PEAK 12

23

26

19

30

BEGIN P 6403(10)

DESIGN DESIGNATION

ADT (2017) 130 ADT (2037) 190 DHV: 30 D: 50% T DHV: 3.5% T ADT: 7.7% V 35 mph

SOUTH ROCHFORD ROAD Station 0+00.00 1,684.30 feet South and 2,909.67 feet West of the

2,909.67 feet West of the Northeast corner of Section 24 -Township 1 North - Range 2 East

STORM WATER PERMIT

Major Receiving Body of Water: Rapid Creek and North Fork Castle Creek Area Disturbed: 59.4 Acres Total Project Area: 95 Acres Approx. Begin Lat,Long: 44°02'01.68", 103°49'33.88"

Gross Length Length of Exceptions

Net Length

53430.81 Feet 0.00 Feet

53430.81 Feet

33

10.119 Miles 0.000 Miles 10.119 Miles



25

13

24

18

19

CASTLE PEAK ELEV. 6358

14

SIGNAL KNOB.

29

SURVEY BY:
INTERSTATE
ENGINEERING
SPEARFISH, SD

PLANS BY:

PLANS BY: NTERSTATE NGINEERING PEARFISH, SD

120 Industrial Drive, Suite 2 • Spearfish, South Dakota 57783 Office: (605) 642-4772 • Fax: (605) 642-4773 • www.interstateeng.com March 22, 2023

ESTIMATE OF QUANTITIES AND ENVIRONMENTAL COMMITMENTS OSES ONLY

STATE OF	PROJECT
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Revised Date: 02/13/2023

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E0010	Mobilization	Lump Sum	LS
009E3230	Grade Staking	20.48	Mile
009E3280	Slope Staking	20.480	Mile
009E3301	Engineer Directed Surveying/Staking	40.0	Hour
009E4300	Construction Schedule, Category III	Lump Sum	LS
100E0100	Clearing	Lump Sum	LS
110E0210	Remove Building(s)	Lump Sum	LS
110E0550	Remove Cattle Guard	1	Each
110E0600	Remove Fence	46,941	Ft
110E1690	Remove Sediment	86.7	CuYd
110E1700	Remove Silt Fence	22,447	Ft
110E7802	Remove Fence for Reset	1,372	Ft
120E1000	Muck Excavation	2,178	CuYd
120E1100	Unclassified/Rock Excavation	493,590	CuYd
120E1200	Blasting	1,000	CuYd
120E2000	Undercutting	98,758	CuYd
120E6100	Water for Embankment	1,250.0	MGal
120E6200	Water for Granular Material	460.0	MGal
205E0010	Dust Control Chloride	71,000	Lb
205E0030	Dust Control Nonchloride	5,000	Lb
230E0010	Placing Topsoil	31,944	CuYd
240E0010	Obliterate Old Road	58	Sta
250E0020	Incidental Work, Grading	Lump Sum	LS
260E1010	Base Course	57,702.0	Ton
260E1060	Base Course, Modified	2,393.0	Ton
260E5000	Shot Rock	15,987.0	Ton
260E6010	Granular Material	5,873.0	Ton
270E0110	Salvage and Stockpile Granular Material	35,655.0	Ton
320E0007	PG 64-28 Asphalt Binder	1,964.0	Ton
320E1050	Class E Asphalt Concrete	37,306.0	Ton
320E3000	Compaction Sample	114	Each
330E0100	SS-1h or CSS-1h Asphalt for Tack	55.0	Ton
421E0100	Pipe Culvert Undercut	23	CuYd
450E4758	18" CMP 14 Gauge, Furnish	3,014	Ft
	18" CMP, Install	3,014	Ft
	24" CMP 14 Gauge, Furnish	316	Ft
	24" CMP, Install	316	Ft
450E4778	30" CMP 14 Gauge, Furnish	160	Ft
450E4780	30" CMP, Install	160	Ft
	36" CMP 14 Gauge, Furnish	130	Ft
450E4790	36" CMP, Install	130	Ft
450E4798		76	Ft
	42" CMP, Install	76	Ft

BID ITEM NUMBER	ITEM	QUANTITY	UNIT	
450E5015	24" CMP Elbow, Furnish	1	Each	
450E5016	24" CMP Elbow, Install	1	Each	
450E5211	18" CMP Flared End, Furnish	61	Each	
450E5212	18" CMP Flared End, Install	61	Each	
450E5215	24" CMP Flared End, Furnish	12	Each	
450E5216	24" CMP Flared End, Install	12	Each	
450E5219	30" CMP Flared End, Furnish	4	Each	
450E5220	30" CMP Flared End, Install	4	Each	
450E5227	42" CMP Flared End, Furnish	1	Each	
450E5228	42" CMP Flared End, Install	1	Each	
450E5406	18" CMP Safety End, Furnish	40	Each	
450E5407	18" CMP Safety End, Install	40	Each	
450E5420	36" CMP Safety End, Furnish	4	Each	
450E5423	36" CMP Safety End, Install	4	Each	
600E0300	Type III Field Laboratory	1	Each	
620E0010	Type 1 Right-of-Way Fence	26,009	Ft	
620E0020	Type 2 Right-of-Way Fence	18,208	Ft	
20E0030	Type 3 Right-of-Way Fence	2,980	Ft	
620E0260	Modified Type 6 Right-of-Way Fence	1,116	Ft	
620E1020	2 Post Panel	65	Each	
620E1030	3 Post Panel	59	Each	
620E4100	Reset Fence	1,372	Ft	
630E1010	Straight Class A W Beam Guardrail with Wood Posts	1,290.0	Ft	
630E2010	W Beam Guardrail End Terminal	2	Each	
630E2015	W Beam Guardrail Flared End Terminal	8	Each	
634E0010	Flagging	5,040.0	Hour	
634E0020	Pilot Car	1,680.0	Hour	
634E0110	Traffic Control Signs	751.7	SqFt	
634E0120	Traffic Control, Miscellaneous	Lump Sum	LS	
634E0275	Type 3 Barricade	24	Each	
634E0320	Temporary Flexible Vertical Markers (Tabs)	10.1	Mile	
634E0630	Temporary Pavement Marking	20.4	Mile	
634E1215	Contractor Furnished Portable Changeable Message Sign	5	Each	
700E0210	Class B Riprap	12.0	Ton	
730E0100	Cover Crop Seeding	29.7	Bu	
730E0208	Type E Permanent Seed Mixture	1,188	Lb	
731E0200	Fertilizing	59.40	Ton	
732E0100	Mulching	178.2	Ton	
734E0042	Soil Stabilizer	1,000.0	SqYd	
734E0101	Type 1 Erosion Control Blanket	500	SqYd	
734E0154	12" Diameter Erosion Control Wattle	27,814	Ft	
734E0165	Remove and Reset Erosion Control Wattle	6,922	Ft	
734E0325	Surface Roughening	104.4	Acre	

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
734E0400	Rock Check Dam	105.0	CuYd
734E0510	Shaping for Erosion Control Blanket	450	Ft
734E0604	High Flow Silt Fence	22,447	Ft
734E0610	Mucking Silt Fence	1,560	CuYd
734E0620	Repair Silt Fence	5,610	Ft
831E0202	Woven Separator Fabric, Modified	17,500	SqYd
900E0010	Refurbish Single Mailbox	17	Each
900E1080	Orange Plastic Safety Fence	9700	Ft

SPECIFICATIONS

Standard Specifications for Roads and Bridges, 2015 Edition and Required Provisions, Supplemental Specifications, and Special Provisions as included in the Proposal.



ENVIRONMENTAL COMMITMENTS

The SDDOT is committed to protecting the environment and uses Environmental Commitments as a communication tool for the Engineer and Contractor to ensure that attention is given to avoid, minimize, and/or mitigate an environmental impact. Environmental commitments to various agencies and the public have been made to secure approval of this project. An agency with permitting authority can delay a project if identified environmental impacts have not been adequately addressed. Unless otherwise designated, the Contractor's primary contact regarding matters associated with these commitments will be the Project Engineer. During construction, the Project Engineer will verify that the Contractor has met Environmental Commitment requirements. These environmental commitments are not subject to change without prior written approval from the SDDOT Environmental Office.

Additional guidance on SDDOT's Environmental Commitments can be accessed through the Environmental Procedures Manual found at: https://dot.sd.gov/media/documents/EnvironmentalProceduresManual.pdf

For questions regarding change orders in the field that may have an effect on an Environmental Commitment, the Project Engineer will contact the Environmental Engineer at 605-773-3180 or 605-773-4336 to determine whether an environmental analysis and/or resource agency coordination is necessary.

Once construction is complete, the Project Engineer will review all environmental commitments for the project and document their completion.

COMMITMENT A: WETLANDS

All efforts to avoid and minimize wetland impacts from the project have resulted in approximately 0.519 acres of wetlands (includes temporary and permanent) becoming impacted. Refer to the plan sheets for location and boundaries of the impacted wetlands.

Table of Impacted Wetlands

Wetland No.	Station	Perm. Impact Left (Acres)	Perm. Impact Right (Acres)	Temp. Impact Left (Acres)	Temp. Impact Right (Acres)	Total Impact (Acres)
40 (wetland)	245+00 - 246+00 R	0.00	0.03	0.00	0.00	0.03
30 (wetland)	245+00 - 246+00 L	0.022	0.00	0.00	0.00	0.022
41 (wetland)	481+50 – 484+00 L	0.002	0.00	0.00	0.00	0.002
13 (wetland)	481+50 – 485+00 R	0.00	0.037	0.00	0.000	0.037
11c (fen)	500+00 – 506+00 R	0.00	0.011	0.00	0.00	0.011
11b (fen)	494+00 – 507+00 R	0.00	0.002	0.00	0.00	0.002
11a (fen)	498+50 - 499+50 L	0.01	0.00	0.00	0.00	0.01

9	528+00 -	0.00	0.047	0.00	0.00	0.047
(wetland)	534+50 R	0.00	0.047	0.00	0.00	0.047
8d	507+00 -	0.00	0.002	0.00	0.00	0.002
(wetland)	510+00 R	0.00	0.002	0.00	0.00	0.002
8c	509+50 -	0.00	0.006	0.00	0.00	0.006
(wetland)	510+00 R	0.00	0.006	0.00	0.00	0.006
8b	510+50 R	0.00	0.003	0.00	0.00	0.003
(wetland)	310+30 K	0.00	0.003	0.00	0.00	0.003
8a	510+50 -	0.347	0.00	0.00	0.00	0.347
(wetland)	536+00 L	0.347	0.00	0.00	0.00	0.347

Action Taken/Required:

Mitigation for wetlands will be created on-site to account for permanent project impacts to existing wetlands. The mitigation for the wetlands is shown in the plans. Mitigation consists of replacing the limestone base course adjacent to the wetlands with a granite material and there will be a restoration of fen wetlands at the Rochford Cemetery fen location.

The Contractor is not permitted to stage equipment or materials within identified wetland areas. The Contractor will notify the Project Engineer if additional easement is needed to complete the work adjacent to any wetlands. The Project Engineer will obtain an appropriate course of action from the Environmental Office before proceeding with construction activities that affect any wetlands.

Temporary impacts identified in the Table of Impacted Wetlands will not be mitigated as original contours and elevations will be re-established as designated in plan sheets. Prior to initiating temporary work in wetlands, the Contractor will submit a plan to the Project Engineer in accordance with Section 7.21 D of the Specifications.

COMMITMENT B: FEDERALLY THREATENED, ENDANGERED, AND PROTECTED SPECIES

COMMITMENT B2: WHOOPING CRANE

The Whooping Crane is a spring and fall migratory bird in South Dakota that is about 5 feet tall and typically stops on wetlands, rivers, and agricultural lands along their migration route. An adult Whooping Crane is white with a red crown and a long, dark, pointed bill. Immature Whooping Cranes are cinnamon brown. While in flight, their long necks are kept straight and their long dark legs trail behind. Adult Whooping Cranes' black wing tips are visible during flight.

Action Taken/Required:

Harassment or other measures to cause the Whooping Crane to leave the site is a violation of the Endangered Species Act. If a Whooping Crane is sighted roosting in the vicinity of the project, borrow pits, or staging areas associated with the project, cease construction activities in the affected area until the Whooping Crane departs and immediately contact the Project Engineer. The Project Engineer will contact the Environmental Office so that the sighting can be reported to USFWS.

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COMMITMENT B4: BALD EAGLE

Bald eagles are known to occur in this area.

Action Taken/Required:

If a nest is observed within one mile of the project site, notify the Project Engineer immediately so that he/she can consult with the Environmental Office for an appropriate course of action.

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COMMITMENT B5: NORTHERN LONG-EARED BAT

This project is within the range of suitable habitat for the Northern Long-Eared Bat (NLEB) and project work will avoid conflicts with NLEB roosting habitat.

Action Taken/Required:

Project activities that include tree removal, structure work, and/or work within one-quarter mile of a known hibernacula or 150 feet of a known maternity roost tree, or suitable habitat should not occur within the location(s) listed below during the NLEB seasonal work restriction timeframe without approval from the SDDOT Environmental Office.

Station	NLEB Seasonal Work Restriction			
0+00 to 534+30 (L&R)	April 1 to October 31			

Tree removal will occur between November 1st and March 31st.

COMMITMENT C: WATER SOURCE

The Contractor will not withdraw water with equipment previously used outside the State of South Dakota or previously used in aquatic invasive species (AIS) positive waters within South Dakota without prior approval from the SDDOT Environmental Office. To prevent and control the introduction and spread of invasive species into the project vicinity, all equipment will be power washed with hot water (≥140 °F) and completely dried for a minimum of 7 days prior to subsequent use. South Dakota administrative rule 41:10:04:02 forbids the possession and transport of AIS; therefore, all attached dirt, mud, debris and vegetation must be removed and all compartments and tanks capable of holding standing water must be drained. This includes, but is not limited to, all equipment, pumps, lines, hoses and holding tanks.

Action Taken/Required:

The Contractor will obtain the necessary permits from the regulatory agencies such as the South Dakota Department of Agriculture and Natural Resources (DANR) and the United States Army Corps of Engineers (USACE) prior to water extraction activities.

Additional information and mapping of water sources impacted by Aquatic Invasive Species in South Dakota can be accessed at:

< https://sdleastwanted.sd.gov/maps/default.aspx >

< South Dakota Administrative Rule 41:10:04 Aquatic Invasive Species: https://sdlegislature.gov/rules/DisplayRule.aspx?Rule=41:10:04 >

COMMITMENT D: WATER QUALITY STANDARDS

COMMITMENT D1: SURFACE WATER QUALITY

Rapid Creek is classified as a cold water permanent fishery with a total suspended solids standard of less than 30 mg/L 30-day average, less than 53 mg/L daily maximum.

The North Fork Castle Creek is classified as a cold water permanent fishery with a total suspended solids standard of less than 30 mg/L 30-day average. less than 53 mg/L daily maximum.

Action Taken/Required:

The Contractor is advised that the South Dakota Surface Water Quality Standards, administered by the South Dakota Department of Agriculture and Natural Resources (DANR), apply to this project. Special construction measures will be taken to ensure the above standard(s) of the surface waters are maintained and protected.

COMMITMENT D2: SURFACE WATER DISCHARGE

The DANR General Permit for Temporary Discharge is required for temporary dewatering and discharges to waters of the state. The effluent limit for total suspended solids will be 90 mg/L 30-day average. The effluent limit applies to discharges to all waters of the state except discharges to waters classified as cold water permanent fish life propagation waters according to the ARSD 74:51:01:45. For discharges to waters of the state classified as cold water permanent fish life propagation waters, the effluent limit for total suspended solids will be 53 mg/L daily maximum.

The permittee has the option of completing effluent testing or implementing a pollution prevention plan for compliance with this permit. If the permittee develops a pollution prevention plan instead of total suspended solids sampling, the plan must be developed and implemented prior to discontinuing total suspended solids sampling. Refer to Section 4.0 of the permit. If any pollutants are suspected of being discharged, a sample must be taken for those parameters listed in Section 3.4 of the permit.

Refer to Commitment D1: Surface Water Quality for stream classification.

Action Taken/Required:

If construction dewatering is required and this project is currently covered under a General Permit for Stormwater Discharges Associated with Construction Activities, the contractor will need to submit the dewatering information to the SDDANR using the following form:

https://danr.sd.gov/OfficeOfWater/SurfaceWaterQuality/docs/DANR AddTe mpInfoFillable.pdf >

The Contractor will provide a copy of the approved permit or the submitted dewatering information to the Project Engineer prior to proceeding with any dewatering activities. The approved permit or submitted dewatering information must be kept on-site and as part of the project records.

Effluent monitoring, as a result of dewatering activities, will be summarized for each month and recorded on a separate Discharge Monitoring Report (DMR) and submitted to DANR monthly. Additional information can be found at:

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STATE OF SDDOT: < https://dot.sd.gov/doing-business/environmental/stormwater >

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https://danr.sd.gov/OfficeOfWater/SurfaceWaterQuality/stormwater/default.a spx >

EPA: < https://www.epa.gov/npdes >

COMMITMENT E: STORM WATER

Construction activities constitute 1 acre or more of earth disturbance and/or work in a waterway.

https://danr.sd.gov/OfficeOfWater/SurfaceWaterQuality/swdpermitting/Erepo

Action Taken/Required:

rting.aspx >

The DANR General Permit for Stormwater Discharges Associated with Construction Activities is required for construction activity disturbing one or more acres of earth and work in a waterway. The SDDOT is the owner of this permit and will submit the NOI to DANR 15 days prior to project start in order to obtain coverage under the General Permit. Work can begin once the DANR letter of approval is received.

The Contractor must adhere to the "Special Provision Regarding Storm Water Discharges to Waters of the State."

The Contractor will complete the DANR Contractor Certification Form prior to the pre-construction meeting. The form certifies under penalty of law that the Contractor understands and will comply with the terms and conditions of the permit for this project. Work may not begin on this project until this form is signed and submitted to DANR.

The form can be found at:

https://danr.sd.gov/OfficeOfWater/SurfaceWaterQuality/docs/DANR_CGPAp pendixCCA2018Fillable.pdf >

The Contractor is advised that permit coverage may also be required for offsite activities, such as borrow and staging areas, which are the responsibility of the Contractor.

Storm Water Pollution Prevention Plan

The Storm Water Pollution Prevention Plan (SWPPP) will be developed prior to the submittal of the NOI and will be implemented for all construction activities for compliance with the permit. The SWPPP must be kept on-site and updated as site conditions change. Erosion control measures and best management practices will be implemented in accordance with the SWPPP.

The DOT 298 Form will be used for site inspections and to document changes to the SWPPP. A copy of the completed inspection form will be filed with the SWPPP documents and retained for a minimum of three years.

The inspection will include disturbed areas of the construction site that have not been finally stabilized, areas used for storage materials, structural control measures, and locations where vehicles enter or exit the site. These areas will be inspected for evidence of, or the potential for, pollutants entering the drainage system. Erosion and sediment control measures identified in the SWPPP will be observed to ensure that they are operating correctly, and sediment is not tracked off the site.

Information on storm water permits and SWPPPs are available on the following websites:

COMMITMENT F: SEASONAL WORK RESTRICTION

The State of South Dakota Game, Fish, and Parks has designated a cold water fishery associated with this project.

Action Taken/Required:

Construction or demolition activities should not take place during the Seasonal Work Restriction listed in the below table to avoid conflicts with spawning fish. If flows during this time are nonexistent or extremely low, the seasonal use restriction may not be applicable. The Contractor will not conduct in-stream work during the Seasonal Work Restriction without prior approval from the SDDOT Environmental Office.

Stream Name	Stream Classification	Seasonal Work Restriction	
North Fork Castle Creek	Cold Water	October 1 to April 1	

COMMITMENT H: WASTE DISPOSAL SITE

The Contractor will furnish a site(s) for the disposal of construction and/or demolition debris generated by this project.

Action Taken/Required:

Construction and/or demolition debris may not be disposed of within the Public ROW.

The waste disposal site(s) will be managed and reclaimed in accordance with the following from the General Permit for Construction/Demolition Debris Disposal Under the South Dakota Waste Management Program issued by the Department of Agriculture and Natural Resources.

The waste disposal site(s) will not be located in a wetland, within 200 feet of surface water, or in an area that adversely affects wildlife, recreation, aesthetic value of an area, or any threatened or endangered species, as approved by the Environmental Office and the Project Engineer.

If the waste disposal site(s) is located such that it is within view of any ROW, the following additional requirements will apply:

Construction and/or demolition debris consisting of concrete, asphalt concrete, or other similar materials will be buried in a trench separate from wood debris. The final cover over the construction and/or demolition debris will consist of a minimum of 1 foot of soil capable of supporting vegetation. Waste disposal sites provided outside of the Public ROW will be seeded in accordance with Natural Resources Conservation Service recommendations. The seeding recommendations may be obtained through the appropriate County NRCS Office. The Contractor will control the access to waste disposal

COMMITMENT H: WASTE DISPOSAL SITE (continued)

sites not within the Public ROW with fences, gates, and placement of a sign or signs at the entrance to the site stating, "No Dumping Allowed".

Concrete and asphalt concrete debris may be stockpiled within view of the ROW for a period not to exceed the duration of the project. Prior to project completion, the waste will be removed from view of the ROW or buried. and the waste disposal site reclaimed as noted above.

The above requirements will not apply to waste disposal sites that are covered by an individual solid waste permit as specified in SDCL 34A-6-58, SDCL 34A-6-1.13, and ARSD 74:27:10:06.

Failure to comply with the requirements stated above may result in civil penalties in accordance with South Dakota Solid Waste Law, SDCL 34A-6-

All costs associated with furnishing waste disposal site(s), disposing of waste, maintaining control of access (fence, gates, and signs), and reclamation of the waste disposal site(s) will be incidental to the various contract items.

COMMITMENT I: HISTORIC PRESERVATION OFFICE CLEARANCES

The SDDOT has obtained concurrence with the State Historic Preservation Office (SHPO) for all work included within the project limits and all department designated sources and designated option material sources, stockpile sites, storage areas, and waste sites provided within the plans.

Action Taken/Required:

All earth disturbing activities not designated within the plans require a cultural resource review prior to scheduling the pre-construction meeting. This work includes but is not limited to: Contractor furnished material sources, material processing sites, stockpile sites, storage areas, plant sites, and waste areas.

The Contractor will arrange and pay for a record search and when necessary. a cultural resource survey. The Contractor has the option to contact the state Archaeological Research Center (ARC) at 605-394-1936 or another qualified archaeologist, to obtain either a records search or a cultural resources survey. A record search might be sufficient for review if the site was previously surveyed; however, a cultural resources survey may need to be conducted by a qualified archaeologist.

The Contractor will provide ARC with the following: a topographical map or aerial view in which the site is clearly outlined, site dimensions, project number, and PCN. If applicable, provide evidence that the site has been previously disturbed by farming, mining, or construction activities with a landowner statement that artifacts have not been found on the site.

The Contractor will submit the cultural resources survey report to SDDOT Environmental Office, 700 East Broadway Avenue, Pierre, SD 57501-2586. SDDOT will submit the information to the appropriate SHPO/THPO. Allow 30 Days from the date this information is submitted to the Environmental Engineer for SHPO/THPO review.

In the event of an inadvertent discovery of human remains, funerary objects, or if evidence of cultural resources is identified during project construction activities, then such activities within 100 feet of the inadvertent discovery will

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course of action from the Environmental Office before proceeding with construction activities that affect any Section 4(f) property.

STATE OF P 6403 (10) any Section 4(f) property. The Project Engineer will obtain an appropriate

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immediately cease and the Project Engineer will be immediately notified. The Project Engineer will contact the SDDOT Environmental Office, who will contact the appropriate SHPO/THPO within 48 hours of the discovery to determine an appropriate course of action.

SHPO review does not relieve the Contractor of the responsibility for obtaining any additional permits and clearances for Contractor furnished material sources, material processing sites, stockpile sites, storage areas, plant sites, and waste areas that affect wetlands, threatened and endangered species, or waterways. The Contractor will not utilize a site known or suspected of having contaminated soil or water. The Contractor will provide the required permits and clearances to the Project Engineer at the preconstruction meeting.

COMMITMENT M: SECTION 4(f)/6(f) RESOURCES

COMMITMENT M1: SECTION 4(f) PROPERTY

A Section 4(f) Evaluation concluded there are no feasible and prudent alternatives to avoiding Section 4(f) properties located within the project.

Station	Section 4(f) Property		
0+00 - 13+73R 54+42 - 105+31R	Forest Service Management Area 8.2		
536+25 – 536+75	Mickelson Trail		
See Following Table	Environmentally Sensitive Sites		

Action Taken/Required:

The following measures are required to minimize harm to Forest Service Area

- Access to Forest Service Management Area 8.2 will always be maintained during construction activities via flagging operations and/or an approved detour.
- Construction of the Project would be phased to allow traffic continuous access to the area and campground
- Appropriate signage will be installed to alert users of Forest Service Management Area 8.2 of construction activities, access restrictions or closures, and to direct users to secondary access points.
- The staging and/or storage of construction equipment or materials will not take place outside proposed construction limits that are within the defined boundaries of the 4(f) property.
- The Contractor will be required to closely coordinate the construction schedule with SDDOT prior to the start of construction activities.

The following measures are required to minimize harm to the Mickelson Trail:

 Access to the Mickelson Trail will always be maintained during construction activities via flagging operations and/or signage

The Contractor is not permitted to stage equipment or materials within Forest Service Area 8.2 and the Mickelson Trail. The Contractor will notify the Project Engineer if additional easement is needed to complete the work adjacent to

COMMITMENT N: SECTION 404 PERMIT

The SDDOT has obtained a Section 404 Permit from the USACE for the permanent actions associated with this project.

Action Taken/Required:

The Contractor will comply with all requirements contained in the Section 404

The Contractor will also be responsible for obtaining a Section 404 Permit for any dredge, excavation, or fill activities associated with material sources, storage areas, waste sites, and Contractor work sites outside the plan work limits that affect wetlands, floodplains, or waters of the United States.

COMMITMENT P: TRADITIONAL CULTURAL PROPERTY MONITORING

As a result of a Traditional Cultural Property (TCP) Survey, sensitive sites have been identified within and/or adjacent to the project rights-of-way.

The following TCP sites have been identified that require avoidance of construction activities:

Table of Culturally Sensitive Sites

Station	Offset (Ft.)	L/R	Environmental Sensitive Site	Action
-00+90	72	L	ESS-00	Tribal Monitoring/Site Fencing
17+43	28	R	ESS-1	Tribal Monitoring /Plating Operation
24+05	34	L	ESS-2	Tribal Monitoring/Site Fencing
30+98	57	R	ESS-3	Tribal Monitoring/Site Fencing
34+46 to 43+50	Varies	L/R	ESS-4	Tribal Monitoring/Site Fencing
34+97	57	L	ESS-5	Tribal Monitoring/Site Fencing
36+56	48	R	ESS-6	Tribal Monitoring/Site Fencing
77+00 to 82+50	Varies	L	ESS-7	Tribal Monitoring/Site Fencing
146+31	60	L	ESS-8	Tribal Monitoring/Site Fencing
164+54	67	L	ESS-9	Tribal Monitoring/Site Fencing
261+53	55	L	ESS-10	Tribal Monitoring/Site Fencing
262+62	70	L	ESS-11	Tribal Monitoring/Site Fencing
334+42	97	L	ESS-12	Tribal Monitoring/Site Fencing
365+88	40	R	ESS-13	Tribal Monitoring/Site Fencing
370+76 to 379+52	Varies	L/R	ESS-14	Tribal Monitoring/Site Fencing
375+36	41	L	ESS-15	Tribal Monitoring/Site Fencing
377+38	39	R	ESS-16	Tribal Monitoring/Site Fencing
386+46 to 402+80	Varies	L/R	ESS-17	Tribal Monitoring/Site Fencing

The locations and boundaries of the site(s) for avoidance are shown in the non-section plans.

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These identified sites cannot be used for material sources, storage areas, waste sites, and/or any other project related activities outside the plan work

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Action Taken/Required:

If evidence of a TCP is identified during project construction activities, then such activities within 100 feet of the inadvertent discovery will immediately cease, and the Project Engineer will be immediately notified. The Project Engineer will contact the SDDOT Environmental Office who will consult with the Archaeological Research Center (ARC), the Rosebud Sioux Tribe (RST) THPO, Black Hills National Forest (BKNF) Heritage Program Manager, SHPO, and FHWA, to determine the appropriate course of action.

All artifacts, features, or other items of interest uncovered by project construction activities will not be displaced unless the ARC, RST THPO, BKNF Heritage Program Manager, and/or SHPO consent to it. Any artifacts found on National Forest System lands will remain property of the federal government.

Prior to the pre-construction meeting, the Contractor will contact the ARC (Phone: 605-394-1936), the RST THPO (Phone: 605-747-4255), and the BKNF Heritage Program Manager (Phone: 605-673-9265) to arrange for tribal monitors to be present during the installation of orange safety fence around the perimeter of the sensitive site(s) listed in the Table of Culturally Sensitive Sites to ensure proper location, quality, and visibility of the orange safety fence. The exact location of the safety fence will be determined later in the field by the ARC representative.

The Contractor will give written notice to the Engineer seven (7) days prior to the commencement of earth disturbing activities near listed sites identified in the Table of Culturally Sensitive Sites so the Engineer may notify ARC and tribal representative of the day work will start and schedule the installation of orange safety fence. ARC and tribal monitor are to be present during earth disturbing activities to monitor the removal of topsoil, ensure avoidance of the fenced sites, and identify any culturally sensitive sites that may be uncovered.

Work within the vicinity of the site(s) will not begin until the safety fence is installed. All costs associated with furnishing and installing the orange safety fence at the sites listed in the Table of Culturally Sensitive Sites will be incidental to the contract unit price per foot for "Orange Plastic Safety Fence".

SDDOT will provide monitoring for the site(s) identified in the above table.

Prior to the pre-construction meeting, the Contractor will notify the ARC (Phone: 605-394-1936), the RST THPO (Phone: 605-747-4255), and the BKNF Heritage Program Manager (Phone: 605-673-9265) to be present at the meeting for the purposes of coordinating the monitoring schedule of the site(s) listed in the Table of Culturally Sensitive Sites. Work within vicinity of the site(s) will not begin until monitors are present at the site(s).

All costs associated with monitoring will be submitted to the Project Engineer for reimbursement.

Before earth disturbing activities near listed sites occurs, there will be seven (7) days prior notification provided to ARC for scheduling the installation of orange safety fence and/or monitoring at sensitive sites listed in the Table of Culturally Sensitive Sites. ARC and tribal monitor are to be present during earth disturbing activities to monitor the removal of topsoil, ensure avoidance of the fenced sites, and identify any culturally sensitive sites that may be uncovered.

COMMITMENT S: FIRE PREVENTION IN THE BLACK HILLS AREA

This project is located within the Black Hills Forest Fire Protection Boundary.

Action Taken/Required:

The Contractor will adhere to the "Special Provision for Fire Plan".

COMMITMENT T: BIOLOGICAL MONITORING

As a result of the wetland delineation, fen wetlands have been identified adjacent to the project rights-of-way. SDDOT will provide biological monitoring at the fen wetland sites. The locations that need to be monitored are noted in the table below.

Table of Biological Monitoring Locations

Wetland No.	Station	Perm. Impact Left (Acres)	Perm. Impact Right (Acres)	Temp. Impact Left (Acres)	Temp. Impact Right (Acres)	Total Impact (Acres)
1	494+00 to 507+00	0.00	0.00	0.00	0.00	0.00

Action Taken/Required:

The wetland features and vegetation will be documented before, during, and after construction activities. This monitoring will determine if rapid dieback of acid intolerant species occurs and if remedial action is needed. Remedial actions will be determined by SDDOT, Pennington County, and U.S. Forest Service.

If dieback is occurring without establishment of new, acid-tolerant species, the manual re-vegetation of species such as Betula glandulosa, Carex spp. and Sphagnum spp. may be warranted.

COMMITMENT U: CONSTRUCTION SCHEDULING TO PREVENT CONFLICTS WITH CEREMONIAL ACTIVITIES

The SDDOT consulted with tribes that are parties to the Memorandum of Agreement to implement measures that limit construction noise and traffic control during ceremonies scheduled along the Project.

Action Taken/Required:

The SDDOT will provide the consulting tribes a written or electronic notice of the scheduled pre-construction meeting date. The name and contact information of the SDDOT's point of contact will also be provided, in order for tribes to coordinate ceremonial events that could be disrupted by construction noise or traffic control.

COMMITMENT U: CONSTRUCTION SCHEDULING TO PREVENT CONFLICTS WITH CEREMONIAL ACTIVITIES (continued)

In order to limit construction-related disturbances during ceremonial events, the tribes will provide a written or electronic notification to the SDDOT's point of contact. The notification will include the location, date(s), start and stop times, and contact information regarding the ceremony.

All tribes that are parties to the Memorandum of Agreement concur in good faith to:

- 1. Coordinate and schedule those ceremonies planned to be held on properties adjacent to the Project between Stations 10+00 and Station 230+00;
- 2. Coordinate and schedule only those ceremonies that would be disrupted by construction noise or traffic control, taking into consideration the location and type of ceremony;
- 3. Limit ceremonial activities, dates and times to the extent possible as to not unduly impede the construction schedule.
- 4. The SDDOT's point of contact will coordinate with the person that notified the SDDOT of the ceremony to establish boundaries for restricting construction activities. These boundaries will not extend beyond Station 10+00 to Station 230+00.
- 5. For those ceremonies scheduled prior to the pre-construction meeting date, the SDDOT will ensure that the construction contract includes provisions to temporarily cease construction activities within the established boundaries.
- 6. For ceremonies scheduled after the pre-construction meeting date and at least one week prior to the ceremony, the SDDOT will work with the contractor to minimize disruption from construction noise and traffic control activities to the extent possible. However, honoring such requests cannot be guaranteed due to contractual constraints.
- 7. Minimally, construction accommodations will be planned for three known Tribal ceremonies. These known ceremonies are held up to 4 days around the time of April 25, June 21, and September 21. Since these are seasonally based ceremonies, it will be necessary for the tribes to provide the specific dates and times of these events to the SDDOT's point of contact prior to the pre-construction meeting date for these ceremonies to be included in the construction contract provisions.

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CLEARING AND DISPOSAL OF TIMBER

A. U.S. Forest Service Land

The Contractor will dispose of the brush and stumps remaining after Pennington County has logged the merchantable timber. The merchantable timber will be removed during the winter months of 2020/2021 and will be completed prior to this contract and work commencing. Merchantable timber was defined as any standing timber greater than 6 inch in diameter at breast height (DBH).

Slash and non-merchantable timber will be disposed of by chipping and/or mastication. All residue from chipping will be spread and broadcast, per Forest Service regulations avoiding large piles or concentrations of chips greater than 3" in depth.

Stumps from right-of-way clearing will be disposed of at locations off the right-of-way; as well as off land managed and owned by the Forest Service

B. Landowner Property

Merchantable timber will be defined as any species of tree with an inside bark diameter of 6 inches or greater and length greater than 8 feet. Merchantable timber will be limbed and decked outside the right-of-way.

Slash and non-merchantable timber will be disposed of by chipping and/or mastication. All residue will be spread and broadcast, per Forest Service regulations.

Stumps from right-of-way clearing will be disposed of at locations off the right-of-way; as well as off land managed and owned by the Forest Service

GRADING OPERATIONS

Water for Embankment is estimated at the rate of 10 gallons of water per cubic yard of Embankment minus Waste.

Water for Granular Material is estimated at the rate of 5 gallons of water per ton of Base Course.

The estimated cubic yards of excavation and/or embankment required to construct outlet ditches, ditch blocks, and approaches are included in the earthwork balance notes on the profile sheets.

Special ditch grades and other sections of the roadway different than the typical section(s) will be constructed to the limits shown on the cross sections. If significant changes to the cross sections are necessary during construction, the Engineer will contact the Designer for the proposed change.

Generally, all shallow inlet and outlet ditches as noted on the plan sheets will be cut with a 10-foot wide bottom with 5:1 backslopes. However, the Engineer may direct the Contractor to adjust the ditch width for proper alignment with the drainage structure.

Temporary fence and/or permanent fence will be placed ahead of the grading operation unless otherwise directed by the Engineer.

A copy of the soils profile is available for review at the Rapid City Area Office, the Local Government Assistance Office in the SDDOT Central Office in Pierre as well as by emailing SDDOT Bid Letting Office at DOTBids@state.sd.us. The soils profile was created in 2009 and may not be completely accurate due to changes in alignment.

TYPE III FIELD LABORATORY

Substitution of a cellular telephone for the hard-wired touch-tone telephone is not allowed, as state personnel need the ability to download information over direct phone lines. The phone is intended for state personnel usage only. Contractor personnel are prohibited from using this phone unless pre-approved by the Project Engineer. The Contractor will submit a copy of each monthly bill for calls charged to this phone at the end of each month. The Engineer will then audit the bills to ensure all calls are legitimate and then initiate a Construction Change Order (CCO) to reimburse the Contractor for the actual phone calls made, including local and long distance calls. Reimbursement will not be made for fees associated with the purchase, installation, disconnection, monthly line charges, and incidentals involved in the installation, maintenance, and disconnection of the phone (including attachments). These items will be incidental to the contract unit price per each for "Type III Field Laboratory".

UTILITIES

The Contractor will be aware that the existing utilities shown in the plans were surveyed prior to the design of this project and might have been relocated or replaced by a new utility facility prior to construction of this project, might be relocated or replaced by a new utility facility during the construction of this project, or might not require adjustment and may remain in its current location. The Contractor will contact each utility owner and confirm the status of all existing and new utility facilities. The utility contact information is provided elsewhere in the plans or bidding documents.

SHRINKAGE FACTOR: Embankment

				%	
_	Station	to	Station	Shrinkage	
	0+00		36+00	35	
	36+00		74+00	25	
	74+00		190+00	30	
	190+00		221+00	25	
	221+00		290+00	10	
	290+00		364+00	25	
	364+00		382+00	15	
	382+00		414+00	20	
	414+00		532+00	15	
	532+00		583+50	40	

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TABLE OF UNCLASSIFIED EXCAVATION

	(CuYd)
Excavation	341,752
Undercut	98,758
Topsoil	31,944
Unstable Material Excavation	2,667
Salvage & Stockpile Granular Material	18,469
Total	493,590



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PROJECT SHEET TOTAL SHEET

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TABLE OF EXCAVATION QUANTITIES BY BALANCES

		Excavation	* Undercut	* Muck Exc.	Total Excavation	** Waste	** Haul
Station	Station	(CuYd)	(CuYd)	(CuYd)	(CuYd)	(CuYd)	(CuYdSta)
to							
0+00	88+00	14,684	19,254	0	33,938	0	859,506
88+00	158+25	11,623	15,371	0	26,994	57	376,933
158+25	189+25	10,101	6,782	0	16,883	8,973	3,158
189+25	213+50	2,546	5,306	0	7,852	13	14,716
213+50	259+50	41,404	10,065	0	51,469	32,668	43,942
259+50	275+25	7,000	1,722	0	8,722	527	25,633
275+25	301+00	60,492	1,724	0	62,216	43,520	42,598
301+00	381+50	25,285	9,001	0	34,286	195	1,018,161
381+50	477+00	116,001	22,836	0	138,837	98,849	89,010
477+00	455+00	6,393	0	0	6,393	5,920	0
455+00	520+00	15,200	11	1,555	16,766	908	191,511
520+00	534+30	31,023	6,686	623	38,332	26,174	8,778
	Totals:	341,752	98,758	2,178	442,688	217,803	2,673,946

- * The quantities for these items are in the Estimate of Quantities under their respective contract items.
- ** The quantities for these items are for information only.

PROCEDURES FOR DETERMINING UNCLASSIFIED/ROCK EXCAVATION QUANTITY

When plan quantities are used for payment, the Unclassified Excavation quantity will be used for final payment and the plans quantity of Topsoil and salvaged surfacing items listed in the Table of Unclassified Excavation will not be adjusted according to field measurements.

The following paragraphs are general earthwork information and information in regard to computing the Unclassified Excavation quantity when final cross sections are taken in the field:

The Unstable Material Excavation quantity is included in the Excavation quantity listed in the Table of Unclassified Excavation. When finalizing a project, the Unstable Material Excavation quantity will be added to the Excavation quantity to compute the Unclassified Excavation quantity.

The Topsoil quantity in the Table of Unclassified Excavation is an estimate. When finaling a project, the total quantity of field measured Topsoil will be used in place of the estimated Topsoil quantity. The quantity of Topsoil from the cuts will be paid for twice as Unclassified Excavation, as it will be in both the Excavation and Topsoil quantities. This will be full compensation for Excavation, which includes necessary undercutting to provide space for placement of topsoil.

Rock will be encountered within the project limits. All materials except those classified as Muck Excavation encountered during the construction of this project, regardless of their nature or the manner in which they are excavated, will be considered Unclassified/Rock Excavation.

HAUL

Included in the Table of Excavation Quantities by Balances is Haul. It is not a pay item and is for informational purposes only.

<u>Haul</u>: Estimated quantity (CuYdSta) for moving unclassified excavation material to the locations where it is needed throughout the earthwork balance.

OBLITERATING OLD ROAD

The Contractor will obliterate the existing roadway at the locations listed in the Table of Obliterating Old Road.

The surfacing material of the existing roadway will be salvaged.

The Contractor will obliterate the existing roadway in accordance with Section 240 of the Specifications when the existing roadway is not being removed in accordance with the template section.

The earthwork necessary for obliterating the existing road will be accomplished to such an extent that placing topsoil and seeding can be done in a satisfactory manner. Quantities of topsoil, fertilizing, mulching, and seeding for the obliterated sections of the old road are included in the Erosion and Sediment Control Plans Estimate of Quantities.

TABLE OF OBLITERATING OLD ROAD

Station	to	Station	L/R	Length (Station)
276+50		290+00	R	13.5
290+00		295+00	L	5
295+00		299+00	R	4
378+00		385+00	R	7
407+50		412+50	L	5
415+00		420+00	L	5
448+00		454+00	L	6
472+00		480+00	R	8
523+00		525+00	L	4
			Total:	58

HISTORICAL MARKER TURNOUT

A historical marker turnout exists near the north project end approximate station 534+00, care will be taken not to disturb or damage the historical marker.



UNDERCUTTING

In all cut sections the earthen subgrade will be undercut 2 feet below the earthen subgrade surface. The undercut material or other suitable material, as directed by the Engineer, will then be replaced and compacted to the density specified for the section being constructed.

Shallow embankment sections, fills less than 2 feet in height measured at the finished subgrade shoulders, will be undercut to ensure a minimum 2-foot height of earth embankment for the entire width of roadbed. The upper 6 inches of undercut material that consists of topsoil with a high humus content will be used as topsoil, placed in the fill slopes outside the shoulders of the earthen subgrade, or placed in the lower portion (below 4-foot depth) in fills which are greater than 4 feet in height. The remaining undercut soil and soil obtained from adjacent excavation (excluding the upper 6 inches) will then be replaced and compacted to the density specified for the section being constructed.

An exception to the undercut requirements will be made in sections that encounter in place rock. Cut sections made through in place rock will be excavated to the top of the subgrade surface only. Shallow embankment sections (as described above) placed over in place rock with less than 2 feet of soil cover will be excavated to the surface of the rock prior to placing any fill.

The plan shown quantity will be the basis of payment. However, if there are additional areas of undercut other than what is shown in the plans, the Engineer will direct removal of these areas and the additional areas will be measured according to the Engineer.

TABLE OF UNDERCUTTING

TABLE OF UNDERCUTTING					
Station	to	Station	Quantity (CuYd)		
0+00		292+00	60,224		
292+00		294+00	0		
294+00		339+00	8,857		
339+00		342+00	2		
342+00		346+00	142		
346+00		355+00	0		
355+00		409+00	8,469		
409+00		412+00	10		
412+00		416+00	578		
416+00		419+00	0		
419+00		486+00	13,779		
486+00		488+00	11		
488+00		534+31	6,686		
		Total	98,758		

BASE COURSE

Additional quantity has been added to the Base Course Item to grade approaches and driveways to the new asphalt surface. Upon completion of asphalt paying the approaches will be graded to transition to the new payement. 1,500 tons has been included for this purpose.

BASE COURSE, MODIFIED

Due to the presence of Acidic Fen Wetlands in certain project areas and as part of the environmental commitments, the roadway in the following sections will be built with Base Course. Modified.

Base Course, Modified:

Begin Station	End Station	Length (ft)	Tons
463+00	469+00	600	653
490+00	506+00	1,600	1,740
	Total	2,200	2,393

The existing base course in these areas will be required to be moved up or down station thereby removing the negative effects the existing base has on the Acidic Fen Wetlands adjacent to the roadway. Base Course, Modified, will have base course gradation, yet will have acidic properties. Limestone and similar carbonate materials have a pH outside the acidic range, which is detrimental to the Acidic Fen Wetlands. As such the Base Course, Modified will produce an acidic pH of 5 or lower, such as crushed granite, quartzite, rhyolite or similar material.

Test Method 9045C or approved equal will be used to determine the pH of Base Course, Modified (Acidic)

GRANULAR MATERIAL

Due to the presence of Acidic Fen Wetlands in certain project areas and as part of the environmental commitments, the roadway in the following sections will be built with Granular Material. In addition to the acidic nature the material will be porous, allowing a more sheet like flow of lower flow runoff events.

Begin Station	End Station	Length (ft)	Tons
245+13	247+13	200	653
490+00	506+00	1,600	5,220
	Total	1,800	5,873

Granular Material will have the following gradation:

Passing 3 inch sieve	100%
Passing ¾ inch sieve	50-70%
Passing #4 sieve	20-40%
Passing #200 sieve	0-8%

Granular Material will have acidic properties. Limestone and similar carbonate materials have a pH outside the acidic range, which is detrimental to the Acidic Fen Wetlands. As such the Granular Material will produce an acidic pH of 5 or lower, such as crushed granite, quartzite, rhyolite or similar material.

Test Method 9045C or approved equal will be used to determine the pH of Granular Material.

Due to the moisture conditions likely to be encountered in the above locations, excavation/undercut operations and preparation for embankment/granular material placement will be followed immediately by placement of the embankment and granular material. At no time will the excavation be allowed to remain open without moving toward completing the section to top of granular material elevation.

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The following Stations will have Shot Rock, See Typical Section.

Begin Station	End Station	Length (ft)	Tons
386+00	388+50	250	1,631
395+50	396+50	100	653
404+75	406+25	150	979
447+75	449+25	150	979
455+50	469+00	1,200	8,809
482+25	484+75	250	1,631
510+00	512+00	200	1,305
	Total	2,300	15,987

Shot Rock will have acidic properties. Limestone and similar carbonate materials have a pH outside the acidic range, which is detrimental to the Acidic Fen Wetlands. As such the Granular Material will produce an acidic pH of 5 or lower, such as crushed granite, quartzite, rhyolite or similar material.

Test Method 9045C or approved equal will be used to determine the pH of Granular Material.

Shot Rock will consist of broken or crushed ledge rock produced from blasting or quarrying operations. Shot Rock material utilized in subgrade stabilization will be less than 8" in diameter with a nominal size of 4".

Compaction will be to the satisfaction of the Engineer. Acceptance of Shot rock will be by Visual Inspection. Independent Assurance will not be required.

Once Shot Rock placement operations have been stabilized the subgrade, normal grading operations will resume.



WOVEN SEPERATOR FABRIC, MODIFIED

A geotextile fabric will be used to provide separation between the various permeable bases that are being used and the native soils. This fabric will also have the ability to move, wick, water through the fabric. TenCate Mirafi H2Ri is one such fabric, it or an approved equal will be used. The geotextile fabric will meet the following requirements:

			Minimum A	verage
			Roll Value	
Mechanical Properties	Test Method	Unit	MD	CD
Wide Width Tensile Strength	ASTM D4595	lbs/ft (kN/m)	5280 (77.0)	5280 (77.0)
Wide Width Tensile Strength @ 2% strain	ASTM D4596	lbs/ft (kN/m)	480 (7.0)	1080 (15.8)
			Maximum Size	Opening
Apparent Opening Size (AOS)	ASTM D4751	US Sieve (mm)	40 (0.425)	
		•	Minimum R	oll Value
Permittivity	ASTM D4491	sec ⁻¹	0.4	
Flow Rate	ASTM D4491	gal/min/ft ² (l/min/m ²)	30 (1222)	
		<u> </u>	Minimum Te	est Value
Pore Size (050)	ASTM D6767	microns	85	
Pore Size (095)	ASTM D6767	microns	195	
Wet Front Movement ¹ (24 minutes)	ASTM C1559 ²	inches	6.0 Vertical	direction
Wet Front Movement (983 minutes) Zero Gradient	ASTM C1559 ²	inches	73.3 direction	Horizontal

- 1 STP: Standard Temperature & Pressure
- 2 Modified Wet Front Movement (ASTM C1559) is not covered by our current A2LA accreditation.

UNSTABLE MATERIAL EXCAVATION

The areas of unstable material excavation are drawn on the cross sections with a normal depth of 2 feet. The estimated quantity of 2,667 cubic yards of unstable material excavation will be paid for at the contract unit price per cubic yard for "Unclassified/Rock Excavation".

All areas designated as Unstable will be excavated. The unstable material excavated on this project will be placed outside the subgrade shoulder in fill sections or stockpiled and used as topsoil.

Field measurement of unstable material excavation will not be made. However. if there are additional areas of unstable material excavation other than what is shown in the plans, the Engineer will direct removal of these areas and the additional areas will be measured according to the Engineer.

TABLE OF UNSTABLE MATERIAL EXCAVATION

Station to	Station	L/R	Depth (Ft)	Qty (CuYd)
37+50	38+50	Rt	2.0	111
55+50	56+00	Lt	2.0	56
57+00	58+00	Rt	2.0	111
114+00	114+75	Lt & Rt	2.0	167
199+50	200+50	Rt	2.0	111
260+00	269+00	Rt	2.0	1,000
273+00	276+00	Rt	2.0	333
291+00	294+50	Rt	2.0	389
401+50	403+00	Rt	2.0	167
408+50	409+50	Rt	2.0	111
411+00	412+00	Rt	2.0	111
			Total	2,667

MUCK EXCAVATION

The areas of muck excavation are drawn on the cross sections with a normal depth of 3 feet. The estimated quantity of 2,178 cubic yards of muck excavation will be paid for at the contract unit price per cubic yard for "Muck Excavation".

Muck excavation consists of the removal of highly organic and/or highly saturated material from the designated areas shown on the cross sections. Highly organic muck material will not be used in the embankment but may be used as topsoil. Non-organic muck material may be used as embankment outside of the fill subgrade shoulder if it is properly handled and dried prior to placement in the embankment.

Field measurement of muck excavation will not be made unless the Engineer orders additional excavation, or when the Engineer determines, in accordance with Section 120.3 A.1 of the Specifications, that the classification of excavation be changed.

If the areas designated as muck excavation can be removed with similar equipment and procedures as used for unclassified/Rock excavation, the material will be measured and paid for as "Unclassified/Rock Excavation".

TABLE OF MUCK EXCAVATION

Station to	Station	L/R	Depth (Ft)	Qty (CuYd)
480+00	484+00	Rt	3.0	444
508+00	510+00	Rt	3.0	222
510+00	511+00	Lt	3.0	111
514+00	521+00	Lt	3.0	778
529+50	530+50	Rt	3.0	111
532+00	534+30	Lt & Rt	3.0	511
	_		Total	2,178
	480+00 508+00 510+00 514+00 529+50	480+00 484+00 508+00 510+00 510+00 511+00 514+00 521+00 529+50 530+50	480+00 484+00 Rt 508+00 510+00 Rt 510+00 511+00 Lt 514+00 521+00 Lt 529+50 530+50 Rt	480+00 484+00 Rt 3.0 508+00 510+00 Rt 3.0 510+00 511+00 Lt 3.0 514+00 521+00 Lt 3.0 529+50 530+50 Rt 3.0 532+00 534+30 Lt & Rt 3.0

SALVAGE AND STOCKPILE GRANULAR MATERIAL

An estimated 35,655 tons (18,469 Cubic Yards) of existing granular material, anticipated to be 4 inches in thickness, will be salvaged from the entire length of the existing roadway and stockpiled at a site furnished by the Contractor and satisfactory to the Engineer. The material will be used on the project as salvage granular material per typical section. No gradation requirement for the salvage

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Qty (CuYd)	11641360 02/00/2023
111	
56	and stockpile granular material, a proctor will be run on the existing material
111	establishing the ability to test the material in place density.
167	The quantity of salvage granular base material may vary from the plans. No
111	adjustment will be made to the contract unit price for variations of the quantity
1,000	of salvage granular material. Provided the existing granular base material quantity is less than anticipated the base course material quantity will be
333	increased to maintain a total of 12" base course and salvaged granular material
389	total thickness. Conversely if the existing granular material is greater than
167	anticipated the base course material will be decreased to maintain a total of 12" base course and salvaged granular material total thickness.
111	base course and salvaged grandar material total trickness.

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PIPE CULVERT UNDERCUT

Pipe culvert undercut may be required for this project. The Engineer will determine which pipe will be undercut in accordance with Section 421 of the Specifications.

If pipe culvert undercut is required, the table below contains the rate for onefoot depth of pipe culvert undercut per foot of pipe length. When calculating pipe culvert undercut, the length of pipe ends should be included in the overall pipe length.

The table includes undercut for 36 inch and larger pipe culverts. The depth of undercut is an estimate and the actual depth necessary will be determined during construction. Pipes listed may or may not require undercutting and pipes not listed may require undercutting. The Engineer will determine which pipe will be undercut in accordance with Section 421 of the Specifications.

	Undercut Depth	Quantity
Station	(Ft)	(CuYd)
246+12	1	23
	Total:	23

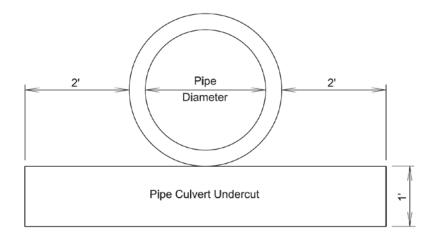


PIPE CULVERT UNDERCUT (continued)

The table below contains the rate for one-foot depth of pipe culvert undercut per foot of pipe length and should be used as an aid in determining the actual amount of undercut to be performed during construction. The table is derived from the drawing below and conforms to the Specifications. When calculating pipe culvert undercut, the length of pipe ends should be included in the overall pipe length.

Storm sewer and approach pipes do not require undercutting unless specified otherwise in these plans.

Pipe Diameter	Round Pipe Undercut Rate for 1' Depth	Arch Pipe Undercut Rate for 1' Depth			
(In)	(CuYd/Ft)	(CuYd/Ft)			
24	0.2407	0.2577			
30	0.2623	0.2847			
36	0.2840	0.3110			
42	0.3056	0.3337			
48	0.3272	0.3596			
54	0.3488	0.3827			
60	0.3704	0.4105			
66	0.3920				
72	0.4136	0.4630			
78	0.4352				
84	0.4568	0.5123			
90	0.4784				



SEISMIC DATA

The seismic data, as shown on the cross sections, are the results of single channel (Bison 1570-C) seismograph traverses run on representative segments of the excavation sections to estimate subsurface conditions and augment surface observations. These tests were conducted in areas where conditions and terrain would permit. The traverses averaged 100 feet in length and the data obtained from each traverse was extrapolated over the length of each segment to estimate the seismic velocities and depth to the change in velocities for each segment. Each individual traverse represents only subsurface conditions directly below the middle two-thirds of each traverse. The calculated results were placed on the cross sections nearest to the placement of the geophone. The calculated depths cannot be assumed to parallel the ground surface, nor can the interpretations be projected laterally with a high degree of reliability. The interpretations are subject to as much as 15 percent deviation in actual depths and 20 percent deviations in velocities. Correlation between seismic velocity and difficulty of excavation FOR BIDDING PURPOSES ONLY DAKOTA and rock based on seismic velocity alone can be quite variable. Rippability can vary with other factors, such as orientation of bedding and/or jointing in bedrock; size, type, and conditions of equipment; and skill of equipment operator.

BLASTING OPERATIONS

The Contractor will exercise utmost care so as not to endanger life or property while using explosives.

Before any drilling operations in preparation for blasting are started, the Contractor will furnish the Engineer a detailed plan of operations showing the method proposed for the prevention of damage. In order to ensure adequate protection, the plan may be modified to meet the conditions that may develop. The Contractor will also consider the location of adjacent structures in preparation of this plan.

Blasting operations will be conducted under the most careful supervision. Ordinarily only light shooting will be permitted. In using explosives, the Contractor will adopt precautions that will prevent damage to surrounding objects. The Contractor will use suitable mats or other approved means to smother the blasts as directed by the Engineer. Nothing herein will release the Contractor from full responsibility for damage or injury resulting from the use of

When using explosives, the Contractor will adopt precautions which will prevent damage to landscape features and other surrounding objects, and which will prevent the scattering of rocks, stumps or other debris outside the

finished roadway slopes. When directed by the Engineer, trees within an area designated to be cleared will be left as protective screen for surrounding vegetation during blasting operations. Trees left as a protective screen will be removed and disposed of after blasting has been completed.

Blasting may be required near the following list of houses. This list is not considered to be a complete listing of areas of concern; also, blasting may not be required near all of the listed locations. These house locations are included in order to indicate that good blasting control and observation will be required on the project.

Station	L/R
493+00	R
535+00	R

A. BLASTING CONSULTANT

The Contractor will retain a recognized blasting consultant to assist in the blast and pre-shear design. The blast design will include both the controlled and production blasting. The consultant will be an expert in the field of drilling and blasting who derives his primary source of income from providing education in an institution of higher education and/or specialized blasting and/or blasting consultant services. The consultant will not be an employee of the Contractor, explosives manufacturer, or explosives distributor.

B. PRE-BLAST CONDITION SURVEY

The Contractor will arrange for a pre-blast survey of any nearby buildings, structures, or utilities that may potentially be at risk from blasting damage. The survey method used will be acceptable to the Contractor's insurance company. The Contractor will be responsible for any damage resulting from blasting. The pre-blast survey records will be made available to the Engineer for review. Occupants of local buildings will be notified by the Contractor prior to the commencement of blasting.

C. VIBRATION CONTROL AND MONITORING

When blasting near buildings, structures, or utilities which may be subject to damage from blast induced ground vibrations, the ground vibrations will be controlled by the use of properly designed delay sequences and allowable charge weights per delay. Allowable charge weights per delay will be based on vibration levels which will not cause damage, and will be established by carrying out trial blasts and measuring vibration levels. The trial blasts will be carried out in conformance with blasting test sections, and modified as required to limit ground vibrations to a level which will not cause damage.

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Whenever vibration damage to adjacent structures is possible, the Contractor will monitor each blast with an approved seismograph located, as approved, between the blast area and the closest structure subject to blast damage. The seismograph used will be capable of recording particle velocity for the three mutually perpendicular components of vibration in the range generally found with controlled blasting.

Peak particle velocity of each component will not be allowed to exceed the safe limits of the nearest structure subject to vibration damage. The Contractor will employ a qualified vibration specialist to establish safe vibration limits. The vibration specialist will also interpret the seismograph records to insure that the seismograph data will be effectively utilized in the control of the blasting operations with respect to the existing structures. The vibration specialist used will be subject to the Engineer's approval.

Data recorded for each shot will be furnished to the Engineer prior to the next blast and will include the following:

- Identification of instrument used.
- 2. Name of Qualified observer and interpreter.
- Distance and direction of recording station from blast area.
- 4. Type of ground at recording station and material on which the instrument is sitting.
- 5. Maximum particle velocity in each component.
- 6. A dated and signed copy of photographic records of seismograph readings.

SCALING OF ROCK SLOPES

No loose rocks will be left on shot or ripped and dozed rock slopes. Loose or detached rock will be removed from the slopes as the depth of the cut progresses. Any rock protrusions on the slopes will be removed with the use of equipment or light blasting as the slope is developed.



REMOVAL OF BUILDING(S)

Included in these plans is the removal and disposal of one (1) building. The location and type of building is as follows:

Station	L/R	Туре	
241+50	R	Calving Shed/Barn	

These buildings will be removed in accordance with Section 110 of the Specifications and all local codes. The disconnecting and capping of utility services will be the responsibility of the Contractor.

These buildings have not been inspected for asbestos. The Contractor will be responsible for providing an asbestos inspection and following all local, state, and federal regulations regarding the removal of asbestos, if found. All costs for the inspection will be incidental to the contract lump sum price for "Remove Building(s)". All costs for removal of the asbestos will be handled during construction by CCO.

TABLE OF CATTLE GUARD REMOVAL

Station	L/R	Size
31+60	Center	30 ft Cattle Guard with Wings

The existing Cattle Guards will be salvaged and stockpiled on site at a location approved by the Engineer. Pennington County will take possession of the salvaged cattle guards at the agreed upon stockpile location. Pennington County will then deliver the salvaged guardrail to the US Forest Service.

INCIDENTAL WORK, GRADING

Existing pipe removed will become the property of the Contractor, and be disposed of outside the ROW.

Pipe Removals

Pipe Removals		379+92	CL	18	40	CMP Mainline Crosspipe			
		Size	Length		388+20	CL	18	40	CMP Mainline Crosspipe
Station	L/R	(in)	(ft)	Remarks	_ 396+13	CL	18	44	CMP Mainline Crosspipe
0+03	CL	21	40	CMP Mainline Crosspipe	408+45	CL	18	42	CMP Mainline Crosspipe
8+51	CL	18	38	CMP Mainline Crosspipe	411+51	CL	18	42	CMP Mainline Crosspipe
16+73	CL	18	36	CMP Mainline Crosspipe	417+53	L	18	40	CMP Approach Pipe
23+06	CL	21	36	CMP Mainline Crosspipe	418+99	L	18	40	CMP Approach Pipe
27+68	L	21	32	CMP Approach Pipe	424+27	CL	18	42	CMP Mainline Crosspipe
29+23	L	21	56	CMP Mainline Crosspipe	435+96	CL	18	40	CMP Mainline Crosspipe
57+05	CL	21	52	CMP Mainline Crosspipe	442+70	CL	18	36	CMP Mainline Crosspipe
81+55	CL	18	46	CMP Mainline Crosspipe	448+97	CL	18	60	CMP Mainline Crosspipe
113+60	CL	24	40	CMP Approach Pipe	454+72	CL	24	40	CMP Mainline Crosspipe
114+58	CL	36	80	CMP Mainline Crosspipe	456+98	CL	18	40	CMP Mainline Crosspipe
115+04	L	18	30	CMP Approach Pipe - Assumed 18"	464+22	CL	18	44	CMP Mainline Crosspipe
116+33	L	18	28	CMP Approach Pipe	468+09	CL	18	44	CMP Mainline Crosspipe
139+83	CL	36	72	CMP Mainline Crosspipe	492+97	CL	24	52	CMP Mainline Crosspipe
142+94	CL	36	64	CMP Mainline Crosspipe	499+39	CL	21	40	CMP Mainline Crosspipe
165+00	CL	18	70	CMP Mainline Crosspipe	510+36	CL	21	54	CMP Mainline Crosspipe
175+29	CL	21	48	CMP Approach Pipe	511+81	CL	21	46	CMP Mainline Crosspipe
185+74	CL	24	50	CMP Mainline Crosspipe	522+21	CL	18	30	CMP Mainline Crosspipe
197+28	R	18	36	CMP Approach Pipe	527+50	CL	18	40	CMP Mainline Crosspipe

197+38

200+44

219+14

230+82

232+32

240+77

241+74

242+54

243+28

243+75

256+43

277+89

285+72

291+12

297+08

301+43

314+54

320+30

356+88

362+99

371+76

308+04 CL

352+14 CL

373+72 CL

246+16 CL

269+56 CL

283+19 CL

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CMP Approach Pipe

CMP Mainline Crosspipe

CMP Mainline Crosspipe

CMP Mainline Crosspipe

CMP Mainline Crosspipe

CMP Mainline Crosspipe

CMP Mainline Crosspipe

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CMP Approach Pipe

CMP Approach Pipe

CMP Approach Pipe

CMP Approach Pipe

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Γ	STATE OF	PROJECT	SHEET	TOTAL SHEETS
Ν.	SOUTH DAKOTA	P 6403(10)	13	333

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530+10	CL	18	40	CMP Mainline Crosspipe
531+25	CL	18	20	CMP Mainline Crosspipe
535+44	CL	24	44	CMP Mainline Crosspipe

As a portion of the Incidental Work, Grading; the Contractor will salvage and stockpile peat material from the excavation near approximate stations 498+00 to 502+00. The material will contain no limestone or carbonate properties and be approved the engineer prior to the work commencing, as will the stockpile location. The stockpile location will be located off the ROW and within the limits of the above mentioned stations. The approximate quantity of stockpiled material is ten (10) cubic vards. As the availability of peat material is undetermined, alternate sources may be required. If alternate sources are required, the methodology and possible additional payment will be addressed during construction. The Incidental Work, grading item will be full compensation for the above mentioned pipe removal work and peat salvage and stockpile.

CORRUGATED METAL PIPE

Corrugated metal pipes will have 2 %-inch x ½-inch corrugations for 42-inch and smaller round pipe and 48-inch and smaller arch pipe unless otherwise stated in the plans. Corrugated metal pipes will have 3-inch x 1-inch or 5-inch x 1-inch corrugations for 48-inch and larger round pipe and 54-inch and larger arch pipe unless otherwise stated in the plans.

The soils within the project area are highly corrosive to steel. Corrugated metal pipe in these areas are specified in the Table of Pipe Quantities and the pipe will be 14 gauge steel. Corrugated metal pipe in these areas including the connection bands and elbows will be polymer coated and will be in conformance with AASHTO M245 and AASHTO M36. Riveted pipe will not be allowed. The connection bands will be 24 inches wide.

All damage to the polymer coating will be repaired in accordance with the manufacturer's recommendations prior to installation of the pipe.

All costs associated with the polymer coating including repair of polymer coating will be incidental to the corresponding CMP contract items.

Metal pipe end sections connected to polymer coated CMP will be aluminumcoated (Type 2) in accordance with AASHTO M36 as specified in the Table of Pipe Quantities. All costs associated for gauge, coating, and connections will be incidental to the corresponding CMP End Section contract items



PIPE FOR APPROACHES AND INTERSECTING ROADS

Class 2 reinforced concrete pipe, high density polyethylene pipe, corrugated polypropylene pipe, or steel reinforced polyethylene pipe may be substituted for corrugated metal pipe at approaches and intersecting roads at no additional cost to the State.

INTERSECTING ROADS AND ENTRANCES

Intersecting roads and entrances will be satisfactorily cleared of vegetation, shaped and compacted prior to placement of mainline surfacing. This work will be considered incidental to other contract items. Separate measurement and payment will not be made.

CLASS E ASPHALT CONCRETE

Mineral Aggregate for Class E Asphalt Concrete will conform to the requirements for Class E, Type 1. All other requirements for Class E will

RATES OF MATERIALS

The Estimate of Quantities is based on the following quantities of materials

Base Course 105.87 tons

Water for Granular Material at the rate of 0.85 MGal

CLASS E ASPHALT CONCRETE - Each 2 inch LIFT

Crushed Aggregate		33.49 tons
PG 64-28 Asphalt Binder		1.76 tons
. Т	otal Mix	35.25tons

The exact proportions of these material will be determined on construction.

Tack Application Rate: (Rate = 0.075 gallon per square yard).

TABLE OF GUARDRAIL

<u>Station</u>	<u>to</u>	<u>Station</u>	L/R
105+00		109+00	R
146+00		148+00	R
209+00		213+00	L
228+00		230+50	L
231+10		232+50	L

W BEAM GUARDRAIL TANGENT END TERMINAL

Payment for the W beam guardrail tangent end terminal will be based on a length of 50 feet and will be in accordance with Specifications Section 630.5 B.

MAILBOXES

If required, the Contractor will reset the existing mailboxes on new posts with the necessary support hardware for single mailbox assemblies. The local Postmaster will determine the recommended mounting height of the mailboxes throughout the project. The Contractor will coordinate with the Engineer on the proper postal representative to contact and where Refurbish Mailbox is required.

STATE OF SHEE1 FOR BIDDING PURPOSES ONLY DAKOTA P 6403(10) 14 333 All costs for removing existing mailboxes, providing temporary mailboxes, and

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TABLE OF REFURBISH MAILBOX

		Single
R/L	Offset	(Each)
R	16.07	1
R	22.06	1
R	13.58	1
R	14.44	1
R	18.93	1
R	11.45	1
R	11.96	1
L	0.82	1
R	21.32	1
R	21.4	1
R	13.2	1
R	13.12	1
R	13.38	1
R	19.03	1
R	19.45	1
R	15.28	1
R	14.28	1
	Total	17
	R R R R R R R R R R R R R R R R R R R	R 16.07 R 22.06 R 13.58 R 14.44 R 18.93 R 11.45 R 11.96 L 0.82 R 21.32 R 21.4 R 13.2 R 13.12 R 13.12 R 13.38 R 19.03 R 19.45 R 15.28 R 14.28

resetting mailboxes with new posts and necessary support hardware will be

incidental to the contract unit price per each for "Refurbish Single Mailbox".

BRACE PANELS FOR ROW FENCE

The E-Z Brace or an approved equal may be utilized as an alternate horizontal brace in the brace panels if approved by the Engineer. The E-Z Brace will be attached to each wood post utilizing two 5/16" x 3" lag screws. Holes of appropriate diameter, based on wood post condition, will be drilled before placement of lag screws. The following are contacts regarding the E-Z Brace:

	Straight	W Beam	W Beam
	Class B	Guardrail	Guardrail
	Guardrail	End	Flared
	w/ Wood	Terminal	End
Location	Posts		Terminal
	(Ft)	(Each)	(Each)
105+00 to 109+00R	400	0	2
146+00 to 148+00 R	200	0	2
209+00 to 213+00 L	400	0	2
228+00 to 230+50 L	150	1	1
231+10 to 232+50 L	140	1	1
Totals:	1290	2	8

Roger Papka E-Z Brace 1160 Karen St. Watertown, SD 57201 605-881-6142

PLACING TOPSOIL

The thickness will be approximately 4 inches within the right-of-way and 6 inches on temporary easements.

The estimated amount of topsoil to be placed is as follows:

				l opsoil	
Station	to	Station		(CuYd)	
0+00		534+31		31,944	
			Total:	31.944	

SEQUENCE OF OPERATIONS

The Contractor will submit a sequence of operations for approval two weeks prior to the preconstruction meeting. If changes to the sequence of operations are proposed during the project, these must be submitted for review a minimum of one week prior to potential implementation. Approval for changes to the sequence of operations will only be allowed when the proposed changes meet with the Department's intent for traffic control and sequencing of the work.

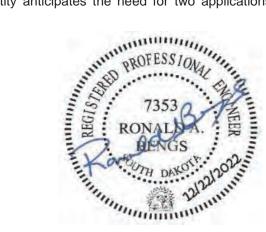
Asphalt paving operations may not begin until the grading, subbase, and base course operations are complete throughout the entire project length most likely during season 2, at which time asphalt paving can commence. Paving must be completed by **September 1**st, **2024.** Paving sequence will minimize truck haul on previously placed asphalt.

During grading, sub-base, & aggregate base operations it is anticipated that the Water for Embankment or Water for Granular will also control dust creation. Once the gravel within a section is completed the Contractor will maintain the surface in a state that will not produce dust. This water is separate from the above-mentioned items and will be incidental to the project.

Dust control chloride will be used in the following locations upon the beginning of haul operations through the area.

Begin Station	End Station	Length (fi
14+00	30+00	1,600
113+00	117+00	400
216+00	291+00	7,500

The dust control chloride quantity anticipates the need for two applications throughout these areas.



SEQUENCE OF OPERATIONS (continued)

Dust control chlorides will not be used in areas adjacent to the wetlands as listed in the Environmental Commitments and tabled in "Commitment A Wetlands". Non-Chloride

Snow removal between the 1st Season and the 2nd Season (between October 14th 2023 and April 28th 2024) will be conducted by Pennington County staff, no gravel maintenance or correcting grade slope or crowns will be completed by Pennington County staff.

The Contractor is advised that the County will implement Spring Load Limits on various routes surrounding the project. The exact dates and duration are set based on how severe the winter has been as well as the spring weather the area is experiencing at the time.

In addition, dependent upon the haul routes used haul road agreements will be required as per SDDOT procedure.

GENERAL TRAFFIC CONTROL

Existing guide, route, informational logo, regulatory, and warning signs will be temporarily reset and maintained during construction. Removing, relocating, covering, salvaging, and resetting of existing traffic control devices, including delineation, will be the responsibility of the Contractor. Cost for this work will be incidental to the contract unit prices for the various items unless otherwise specified in the plans. Any delineators and signs damaged or lost will be replaced by the Contractor at no cost to the State.

All temporary traffic control sign locations will be set in the field by the Contractor and verified by the Engineer prior to installation.

All temporary speed limit signs will have a minimum mounting height of 5 feet in rural locations, even when mounted on portable supports.

All construction operations will be conducted in the general direction of traffic movement.

If there is a discrepancy between the traffic control plans, standard plates, and the MUTCD, whichever is more stringent will be used, as determined by the Engineer.

Unless otherwise stated in these plans, work will not be allowed during hours of darkness.

Fixed location signing placed more than 4 calendar days prior to the start of construction will be covered or laid down until the time of construction. The covers must be approved by the Engineer prior to installation. The cost of materials, labor, and equipment necessary to complete this work will be incidental to other contract items. No separate payment will be made.

All fixed location signs, sign posts, and breakaway bases will be removed within 7 calendar days following pavement marking.

All haul trucks will be equipped with an additional flashing amber light that is visible from the backside of the haul truck. The costs for the flashing amber lights will be incidental to the various related contract items.

At no time will a vertical drop-off of greater than 3 inches be left overnight adjacent to the traveled way. The Contractor will utilize embankment material to ensure a 3-inch vertical drop-off is not exceeded. The slope of the embankment material will not be steeper than a 4:1 within 30 feet of the traveled way.

The Contractor will furnish, install, maintain, and remove TRUCK CROSSING BIDDING PURPOSES ONLY DAKOTA (W8-6) signs daily. The TRUCK CROSSING signs will be displayed always when haul vehicles are hauling material. When hauling conditions no longer exist, the signs will be covered or removed from view. The exact number and location will be determined during construction. Payment for additional signs will be based on the contract unit price per square foot for "Traffic Control

GROOVED PAVEMENT (W8-15) signs with MOTORCYCLE (W8-15P) plaques are required in advance of areas that have been cold milled and are not resurfaced the same day. The GROOVED PAVEMENT sign assemblies will be installed a minimum of 1000 feet in advance of cold milled sections and remain in place until the sections have been resurfaced.

A mobile work operation will be allowed provided the rumble strip or rumble stripe grooving, flush sealing, and pavement marking can be completed satisfactorily by a continuously moving work operation. A mobile work operation will require approval by the Engineer.

TEMPORARY PAVEMENT MARKING

Signs".

Temporary Flexible Vertical Markers (Tabs) will be used on the top lift of asphalt surfacing for centerline delineation, lane lines, skips, and as directed by the Engineer. Tabs will be offset 6-inches from the location shown for permanent pavement markings. Centerline will be double yellow lines with tabs spaced at 5' the entire project length.

Temporary flexible vertical markers (tabs) will be required on the top lift of asphalt concrete surfacing.

Temporary pavement marking paint will not be allowed on the final lift of asphalt surfacing. Temporary pavement marking paint will not be allowed on the chip seal, fog seal, or flush seal. Temporary flexible vertical markers (tabs) must be used on the final lift of asphalt surfacing. The Contractor may use tabs with covers, uncovering them for the chip seal, fog seal, or flush seal. As an alternative, the Contractor may install new tabs for the fog seal or flush seal.

Covers on the tabs will be sufficiently secured to prevent traffic from dislodging the cover and when removed, the covers will be properly disposed of. The Contractor will remove and properly dispose of the tabs after permanent pavement marking is applied. Method of removal will be nondestructive to the road surface and will be accomplished within one week of completion of the permanent pavement marking.

Full reflectivity of all temporary flexible vertical markers (tabs) is required at all times. The Contractor will be required to replace any missing or non-reflective tabs after each installation as detailed below at no additional cost to the State.

MYCORRHIZAL INOCULUM

Mycorrhizal inoculum will consist of mycorrhizal fungi spores and mycorrhizal fungi-infected root fragments in a solid carrier. The carrier may include organic materials, calcinated clay, or other materials consistent with application and good plant growth. The supplier will provide certification of the fungal species claimed and the live propagule count. The inoculum will include the following fungal species:

Glomus intraradices

Glomus aggregatum or deserticola

25% Glomus mosseae 25% Glomus etunicatum STATE OF

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All seed will be inoculated by the seed supplier with a minimum of 100,000 live propagules of mycorrhizal fungi per acre. All costs of inoculating the seed will be incidental to the contract unit price per pound for the corresponding permanent seed mixture.

The mycorrhizal inoculum will be as shown below or an approved equal:

Product Manufacturer

Mycorrhizal Applications, Inc. MycoApply

Grants Pass, OR Phone: 1-866-476-7800

www.mycorrhizae.com

Reforestation Technologies Int. AM 120 Multi Species Blend

Gilroy, CA

Phone: 1-800-784-4769 www.reforest.com

FERTILIZING

The Contractor will apply an all-natural slow release fertilizer prior to seeding or placing sod. The all-natural fertilizer will have a minimum guaranteed analysis of 4-4-4 and be USDA Certified BioBased. It should provide a minimum of 4% (N) nitrogen with a minimum water insoluble nitrogen (WIN) fraction of 2.07%, a minimum of 4% (P2O5) available phosphate, a minimum of 4% (K2O) soluble potash, and a maximum carbon to nitrogen ratio (C:N ratio) of 5:1. The all-natural fertilizer will be free of weed-seed and pathogens accomplished through thermophilic composting, and not mechanical or chemical sterilization, to assure presence of beneficial soil microbiology. The fertilizer will have a near neutral pH, a low salt index, a low biological oxygen demand, contain organic humic and fulvic acids, and have high aerobic organism counts. The fertilizer will also be stable, free of bad odors, and be unattractive as a food source for animals. It should also be in a granular form that is easily spread.

The fertilizer will be applied at a rate of 2,000 pounds per acre in accordance with the manufacturer's recommended method of application.



FERTILIZING (continued)

The all-natural slow release fertilizer will be as shown below or an approved equal:

<u>Product</u> <u>Manufacturer</u>

Sustane Corporate Headquarters

Cannon Falls, Minnesota Phone: 1-800-352-9245 www.sustane.com

Perfect Blend, LLC

Bellevue, WA

Phone: 1-866-456-8890 www.perfect-blend.com

COVER CROP SEEDING

Cover crop seeding may be used on this project as a temporary erosion control measure. The actual limits and use of cover crop seeding will be determined by the Engineer during construction.

EROSION CONTROL

The limits of erosion control work will be determined by the Engineer during construction.

PERMANENT SEEDING

The areas to be seeded consist of all newly graded areas within the project limits except for the top of roadways and temporary easements under cultivation.

Type E Permanent Seed Mixture will consist of the following:

	1				
		Pure Live Seed			
Grass Species	Variety	(PLS) (Pounds/Acre)			
Western Wheatgrass	Arriba, Flintlock, Rodan, Rosana, Walsh	7			
Green Needlegrass	Lodorm, AC Mallard Ecovar	4			
Sideoats Grama	Butte, Pierre	3			
Blue Grama	Bad River	2			
Canada Wildrye	Mandan	2			
Wildflowers					
Dotted Gayfeather (<i>Liatris punctata</i>) 0.5					
Black-eyed Susan (Rudh	0.5				
Blue Flax (Linum lewisii)	0.5				
Pale Purple Coneflower	Pale Purple Coneflower (Echinacea angustifolia)				
	Total:	20			

SURFACE ROUGHENING

Surface roughening will be done after topsoil placement and before permanent seeding, fertilizing, and mulching applications. Refer to Standard Plate 734.25 for details.

Surface roughening, temporary dirt berms and existing vegetation will be integral to the Project Erosion Control and will be used as grading work progresses. The goal is to reduce the quantity of 12" Erosion Control Wattle used during the construction sequence. Allowing the grading to progress

without temporary BMPs interrupting the work, while providing erosion PURPOSES ONL SOUTH DAKOTA during the grading process.

A total disturbed area of 59.4 acres will be surface roughened. An additional 45 acres has been included to address the temporary surface roughening completed during construction to comply with the Erosion Control requirements.

The perimeter at the grading limits will be roughened for a distance/width of 8-10 feet. In the field areas will be defined where flows may compromise the roughened perimeter temporary dirt berms will supplement the roughened area. These combined efforts will be the perimeter controls.

MULCHING (GRASS HAY OR STRAW)

An additional 29.7 tons of Grass Hay or Straw Mulch has been added to the Estimate of Quantities for temporary erosion control on areas determined by the Engineer during construction.

If the Contractor uses a no-till drill, mulch may be applied prior to seeding and the mulch can then be punched into the soil by the no-till drill. If the Contractor uses this process, the no-till drill seeding will be completed immediately following the mulch application and the mulch will be punched into the soil at a 3-inch depth.

SOIL STABILIZER

An estimated quantity of 1,000 SqYd of soil stabilizer has been included in the Estimate of Quantities. The soil stabilizer will be applied on permanently seeded areas and areas deemed necessary by the Engineer.

The Contractor will apply soil stabilizer in accordance with the manufacturer's application instructions and at the rate specified in the list of approved soil stabilizers.

Wood fiber mulch that contains a green dye will be mixed with the soil stabilizer to be used as a tracer when the soil stabilizer is applied hydraulically. Wood fiber mulch will be added at a rate of 300 pounds per acre to all of the approved soil stabilizers listed in the table except for the Pam-12 Plus product. The wood fiber mulch will be a 100% wood fiber product and does not need to contain a tackifier.

All costs for furnishing and applying the soil stabilizer including wood fiber mulch, hauling, materials, equipment, labor, and incidentals necessary will be paid for at the contract unit price per SqYd for "Soil Stabilizer".

The soil stabilizer will be from the list below or an approved equal:

Product

StarTak 600 Applied at a rate of 150 Lb/Acre

> Pam-12 Plus Applied at a rate of:

Slope
None to 4:1 1000 Lb/Acre
4:1 to 3:1 1000 to 2000 Lb/Acre
3:1 to 2:1 2000 to 3000 Lb/Acre

Manufacturer

Chemstar Products Company Minneapolis, MN Phone: 1-800-328-5037 www.chemstar.com

ENCAP, LLC Green Bay, WI

Phone: 1-877-405-5050 http://professional.encap.net/ STATE OF PROJECT
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M-Binder Applied at a rate of 150 Lb/Acre Ecology Controls Carpinteria, CA Phone: 1-805-684-0436 SHEET

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www.ssseeds.com

FiberRX Applied at a rate of:

Slope

None to 4:1 50 Lb/Acre 3:1 60 Lb/Acre 2:1 70 Lb/Acre 1:1 or steeper 80 Lb/Acre Hydrostraw, LLC Manteno, IL

Phone: 1-800-545-1755 hydrostraw.com

Enviropam
Applied at a rate of 9 Lb/Acre

Cincinnati, OH
Phone: 1-513-317-8311
www.innovativeturfsolutions.com

Innovative Turf Solutions, LLC

Innovative Turf Solutions, LLC

HydraTack, Tack Plus, Tack-P, or Tack-P Plus Applied at a rate of 30 Lb/Acre

Phone: 1-513-317-8311 www.innovativeturfsolutions.com

Phone: 1-216-475-8488

JRM Chemical, Inc.

www.soilmoist.com

Cincinnati, OH

Cleveland, OH

FI-1045 Hydrobond or FI-1046 Hydrobond Applied at a rate of 15 Lb/Acre

Rantec Corporation

HF5000 Tack
Applied at a rate of 60 Lb/Acre
Rantec Corporation
Ranchester, WY
Phone: 1-307-655-9565
www.ranteccorp.com

R-Tack Applied at a rate of 150 Lb/Acre Rantec Corporation Ranchester, WY Phone: 1-307-655-9565 www.ranteccorp.com



SpecTac Rantec Corporation
Applied at a rate of: Ranchester, WY
Slope Phone: 1-307-655-9565
None 30 to 80 Lb/Acre www.ranteccorp.com

None 30 to 80 Lb/Acre 4:1 50 to 100 Lb/Acre 3:1 80 to 120 Lb/Acre 2:1 100 to 170 Lb/Acre

Super Tack Rantec Corporation
Applied at a rate of 60 Lb/Acre Ranchester, WY

Phone: 1-307-655-9565 www.ranteccorp.com

EarthGuard SFM
Applied at a rate of 60 LB/Acre
(approx. 6 Gallons/Acre)

Terra Novo Inc. Bakersfield, CA

Phone: 1-661-747-5956 www.terranovo.com

EROSION CONTROL BLANKET

Erosion control blanket will be installed at locations determined by the Engineer during construction.

The erosion control blanket provided will be from the approved product list. The approved product list for erosion control blanket may be viewed at the following internet site:

http://apps.sd.gov/HC60ApprovedProducts/main.aspx

The quantity of Type 1 Erosion Control Blanket was included as a contingent quantity establishing a unit price in the event that it is determined that Type 1 Erosion Blanket is appropriate.

SHAPING FOR EROSION CONTROL BLANKET

The ditches will be shaped for the erosion control blanket as specified on Standard Plate 734.01.

http://apps.sd.gov/HC60ApprovedProducts/main.aspx

HIGH FLOW SILT FENCE

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The high flow silt fence fabric provided will be from the approved product list. The approved product list for high flow silt fence may be viewed at the following internet site:

http://apps.sd.gov/HC60ApprovedProducts/main.aspx

High flow silt fence will be placed at the inlet to all Storm Water Pipes and drainage areas as determined by the Engineer during construction. Refer to Standard Plate 734.05 for details.

High Flow Silt Fence will also act as delineation and an indication as to the location of sensitive wetland areas. The silt fence has only been used in areas where sensitive wetlands approach the project limits. As such install of all BMPs will have the engineer's approval.

Table of High Flow Silt Fence

Begin Station	End Station	L/R	Location	Quantity (Ft)
114+00	115+00	L	Grading Limit	100
114+00	115+25	R	Grading Limit	125
138+75	140+60	L	Grading Limit	185
138+75	140+70	L	Grading Limit	195
232+00	239+75	L	Grading Limit	775
244+00	287+55	R	Grading Limit	4,355
244+75	246+91	L	Grading Limit	216
372+50	511+66	R	Grading Limit	13,916
481+25	484+05	L	Grading Limit	280
498+10	499+55	L	Grading Limit	145
510+00	523+30	L	Grading Limit	1,330
Various Pipe Locations		L/R	Pipes	825
			Total:	22,447

EROSION CONTROL WATTLE

Erosion control wattles for restraining the flow of runoff and sediment will be installed at locations noted in the table and at locations determined by the Engineer during construction. Refer to Standard Plate 734.06 for details.

The Contractor will provide certification that the erosion control wattles do not contain noxious weed seeds.

Erosion control wattles will remain on the project to decompose.

The erosion control wattle provided will be from the approved product list. The approved product list for erosion control wattle may be viewed at the following internet site:

http://apps.sd.gov/HC60ApprovedProducts/main.aspx



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TABLE OF EROSION CONTROL WATTLE

Begin Station	End Station	L/R	Diameter (Inch)	Location	Quantity (Ft)
0+00	0+58	R	12	Slope	5
0+08	1+85	L	12	Slope	17
1+87	2+26	L	12	Slope	3
3+17	3+58	R	12	Slope	4
3+60	4+20	R	12	Slope	6
4+18	5+83	R	12	Slope	16
5+20	7+26	L	12	Slope	20
7+18	8+17	L	12	Slope	9
10+97	11+75	L	12	Slope	7
13+40	14+50	L	12	Slope	11
14+45	16+25	L	12	Slope	18
18+58	20+25	L	12	Slope	16
57+78	63+85	L	12	Slope	60
146+10	148+47	R	12	Slope	23
218+48	229+55	L	12	Slope	1,10
237+00	239+21	R	12	Slope	22
263+20	269+23	L	12	Slope	60
276+09	279+61	L	12	Slope	35
280+63	285+25	L	12	Slope	46
285+79	289+97	L	12	Slope	41
295+00	298+95	L	12	Slope	39
298+64	301+65	R	12	Slope	30
301+66	314+56	L	12	Slope	1,29
320+18	323+20	R	12	Slope	30
322+56	326+26	L	12	Slope	37
327+80	329+02	L	12	Slope	12
329+17	331+93	R	12	Slope	27
330+86	333+10	L	12	Slope	22
337+19	340+70	R	12	Slope	35
356+50	358+90	L	12	Slope	24
367+91	369+53	L	12	Slope	16
373+82	375+53	L	12	Slope	17
379+98	385+14	L	12	Slope	51
386+72	397+65	L	12	Slope	1,09
398+15	401+25	L	12	Slope	31
403+16	407+65	L	12	Slope	44
412+08	414+96	L	12	Slope	28
412+08	412+70	L	12	Slope	6
413+14	414+96	L	12	Slope	18
426+37	433+33	L	12	Slope	69
437+24	442+51	L	12	Slope	52
444+00	453+25	L	12	Slope	92
445+00	453+25	L	12	Slope	82

460+40	463+32	L	12	Slope	<u> 59</u> 2 F
460+40	462+95	L	12	Slope	255
466+35	467+92	L	12	Slope	157
466+35	467+46	L	12	Slope	111
468+13	471+88	L	12	Slope	375
468+13	471+88	L	12	Slope	375
472+21	478+76	L	12	Slope	655
472+21	477+54	L	12	Slope	533
484+48	486+27	L	12	Slope	179
500+02	502+22	L	12	Slope	220
503+54	504+67	L	12	Slope	113
505+63	509+69	L	12	Slope	406
521+76	524+95	R	12	Slope	319
523+02	524+13	L	12	Slope	111
530+32	531+50	L	12	Slope	118
0+00	0+58	L	12	Ditch	45
1+55	2+70	R	12	Ditch	75
32+91	37+63	R	12	Ditch	285
33+12	34+67	L	12	Ditch	105
46+25	56+96	R	12	Ditch	645
69+26	72+76	R	12	Ditch	210
92+29	93+92	R	12	Ditch	105
93+20	94+87	L	12	Ditch	105
104+94	109+10	R	12	Ditch	255
113+56	115+20	L	12	Ditch	105
113+56	115+20	R	12	Ditch	105
116+45	120+96	R	12	Ditch	285
117+77	124+05	L	12	Ditch	390
144+96	148+65	R	12	Ditch	225
187+69	191+35	L	12	Ditch	225
188+00	191+15	R	12	Ditch	195
224+20	230+16	L	12	Ditch	360
284+51	287+75	L	12	Ditch	195
292+60	310+60	L	12	Ditch	1,080
321+00	334+25	R	12	Ditch	795
321+45	331+68	L	12	Ditch	615
337+41	341+55	R	12	Ditch	255
350+70	351+85	R	12	Ditch	75
356+54	358+69	L	12	Ditch	135
380+15	400+96	L	12	Ditch	1,260
				Total	27,814

ROCK CHECK DAM

The rock for the rock check dam will be Pit Run per Specification 882 Table 1. The rock check dam will be constructed to the limits shown on Standard Plate 734.03. All costs for constructing the rock check dam including labor,

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equipment, excavation, and rock will be incidental to the contract unit price per cubic yard for "Rock Check Dam".

Begin Station	End Station	L/R	Location	Quantity (cuyd)	
500+00	535+00	L	Ditch	54	
501+45	535+00	R	Ditch	51	
			_	105	

Through the above Stations Rock Check Dams will be installed every 500 ft in the ditch.

DEWATERING AND SEDIMENT COLLECTING

The Contactor has the option to treat sediment laden water trapped within the project limits or the Contractor may elect to transport sediment laden water off the project. Refer to the OPTIONS FOR DEWATERING AND SEDIMENT COLLECTING detail sheet for more information.

Water transported off the project limits will not be disposed of in an area where it can enter a waterway. The disposal site must be approved by the Engineer.



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TABLE OF CONSTRUCTION STAKING FOR PROJECT P 6403(10) (See Special Provision for Contractor Staking)

						G	rade Staking	ļ	
Roadway and Description	Begin Station	End Station	Number of Lanes	Length (Ft)	Length (Mile)	Lane Factor	*Sets of Stakes	**Grade Staking Quantity (Mile)	Slope Staking Quantity (Mile)
S. Rochford Road (2 Lanes AC Pavement)	0+00	534+30	2	53,540	10.1	1	2	20.2	20.2
S. Rochford Road (Musick Driveway)	0+00	7+14		714	0.14	1	2	0.28	0.28
							Totals:	20.48	20.48



^{* 1 =} Blue Top Stakes Only (Subgrade & Base Course)
** Grade Staking Quantity = (Length) x (Lane Factor) x (Sets of Stakes)

STORMWATER POLLUTION PREVENTION PLAN CHECKLIST

(The numbers left of the title headings are **reference numbers** to the GENERAL PERMIT FOR STORM WATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITIES (Stormwater Permit))

5.3 (2): STAFF TRAINING/SWPPP IMPLEMENTATION

To promote stormwater management awareness specific for this project, the Contractor's Erosion Control Supervisor should provide correspondence of how the SWPPP will be implemented. The Contractor's Erosion Control Supervisor is responsible for providing this information at the preconstruction meeting, and subsequently completing an attendance log, which should identify site-specific implementation of the SWPPP and the names of the personnel who attended the preconstruction meeting. Documentation of the preconstruction meeting will be filed with the SWPPP documents.

5.3 (3): DESCRIPTION OF CONSTRUCTION ACTIVITIES

- > 5.3 (3a): Project Limits (See Title Sheet)
- 5.3 (3a): Project Description (See Title Sheet)
- 5.3 (4): Site Map(s) (See Title Sheet and Plans)
- Major Soil Disturbing Activities (check all that apply)
 - Clearing and grubbing
 - ⊠Excavation/borrow
 - ⊠Grading and shaping

 - Other (describe):
- 5.3 (3b): Total Project Area 95 Acres
- 5.3 (3b): Total Area to be Disturbed 59.4 Acres
- > 5.3 (3c): Maximum Area Disturbed at One Time 30 Acres
- > 5.3 (3d): Existing Vegetative Cover (%) 70%
 - 5.3 (3d): Description of Vegetative Cover Native Grasses
- > 5.3 (3e): Soil Properties: Silty Sand, Sandy Silt, Clay Silt, Gravelly Silty Sand
- > 5.3 (3f): Name of Receiving Water Body/Bodies Rapid Creek, North Fork Castle Creek
- > 5.3 (3g): Location of Construction Support Activity Areas To be Determined, Contractor Responsibility

5.3 (3h): ORDER OF CONSTRUCTION ACTIVITIES

The Contractor will enter the Estimated Start Date.

Description	Estimated Start Date
Install stabilized construction entrance(s).	
Install perimeter protection where runoff may exit site.	
Install perimeter protection around stockpiles.	
Install channel and ditch bottom protection.	
Clearing and grubbing.	
Remove and stockpile topsoil.	
Stabilize disturbed areas.	
Install utilities, storm sewers, curb and gutter.	
Install inlet and culvert protection after completing storm drainage and other utility installations.	
Final grading.	
Final paving.	
Removal of protection devices.	
Reseed areas disturbed by removal activities.	

5.3 (5): DESCRIPTION AND MAINTENANCE OF CONTROL MEASURES All controls will be maintained in good working order. Necessary repairs will

be initiated within 24 hours of the site inspection report. Include the tech FIGHR BIDDING PURPOSES ONLY BOAKOTA

reasoning for selecting each control. (check all that apply)

Perimeter Controls (See Detail Plan Sheets)

Description	Estimated Start Date
□ Natural Buffers (within 50 ft of Waters of State)	
☐ Silt Fence	
□ Temporary Berm / Windrow	
☐ Floating Silt Curtain	
☐ Stabilized Construction Entrances	
☐ Entrance/Exit Equipment Tire Wash	
Other:	

Structural Erosion and Sediment Controls

Description	Estimated Start Date
⊠ Silt Fence	
☐ Temporary Berm/Windrow	
☐ Temporary Sediment Barriers	
☐ Erosion Bales	
☐ Temporary Slope Drain	
☐ Turf Reinforcement Mat	
Riprap	
Gabions	
□ Rock Check Dams	
Culvert Inlet Protection	
☐ Transition Mats	
☐ Median/Area Drain Inlet Protection	
☐ Curb Inlet Protection	
☐ Interceptor Ditch	
☐ Concrete Washout Facility	
☐ Work Platform	
☐ Temporary Water Barrier	
☐ Temporary Water Crossing	
Permanent Stormwater Ponds	
☐ Permanent Open Vegetated Swales	
Sequential Systems that combine several practices	
Other:	

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Description	Estimated Start Date
☐ Tarps & Wind impervious fabrics	
Watering Watering	
☐ Stockpile location/orientation	
□ Dust Control Chlorides	
Other	

Dewatering BMPs

Description	Estimated Start Date
☐ Sediment Basins	
☐ Dewatering bags	
☐ Weir tanks	
☐ Temporary Diversion Channel	
Other:	

Stabilization Practices (See Detail Plan Sheets)

(Stabilization measures will begin the following work day whenever earth disturbing activity on any portion of the site has temporarily or permanently ceased. Temporary stabilization will be completed as soon as practicable but no later than 14 days after initiating soil stabilization activities (3.18))

Description	Estimated Start Date
☐Vegetation Buffer Strips	
☐ Temporary Seeding (Cover Crop Seeding)	
□ Permanent Seeding	
Sodding	
☐ Planting (Woody Vegetation for Soil Stabilization)	
☐ Fiber Mulching (Wood Fiber Mulch)	
Soil Stabilizer Soil Stabilizer	
☐ Bonded Fiber Matrix	
☐ Fiber Reinforced Matrix	
Surface Roughening (e.g. tracking)	
☐ Other:	

Wetland Avoidance

Will construction and/or erosion and sediment controls impinge on regulated wetlands? Yes
No
If yes, the structural and erosion and sediment controls have been included in the total project wetland impacts and have been included in the 404 permit process with the USACE.

5.3 (6): PROCEDURES FOR INSPECTIONS

- Inspections will be conducted at least once every 7 days.
- All controls will be maintained in good working order. Necessary repairs will be initiated within 24 hours of the site inspection report
- Silt fence will be inspected for depth of sediment and for tears to ensure the fabric is securely attached to the posts and that the posts are well anchored. Sediment buildup will be removed from the silt fence when it reaches 1/3 of the height of the silt fence.
- Sediment basins and traps will be checked. Sediment will be removed when depth reaches approximately 50 percent of the structure's capacity, and at the conclusion of the construction.
- Check dams will be inspected for stability. Sediment will be removed when depth reaches ½ the height of the dam.
- All seeded areas will be checked for bare spots, washouts, and vigorous growth free of significant weed infestations.
- Inspection and maintenance reports will be prepared on form DOT 298 for each site inspection, this form will also be used to document changes to the SWPPP. A copy of the completed inspection form will be filed with the SWPPP documents.
- The SDDOT Project Engineer and Contractor's Erosion Control Supervisor are responsible for inspections. Maintenance and repair activities are the responsibility of the Contractor. The SDDOT Project Engineer will complete the inspection and maintenance reports and distribute copies per the distribution instructions on DOT 298.

5.3 (7): POST CONSTRUCTION STORMWATER MANAGEMENT

Stormwater management will be handled by temporary controls outlined in "DESCRIPTION AND MAINTENANCE OF CONTROL MEASURES" above, and any permanent controls needed to meet permanent stormwater management needs in the post construction period will be shown in the plans and noted as permanent.

5.3 (8): POLLUTION PREVENTION PROCEDURES

5.3 (8a): Spill Prevention and Response Procedures

Material Management

- Housekeeping
 - Only needed products will be stored on-site by the Contractor.
 - Except for bulk materials the Contractor will store all materials under cover and/or in appropriate containers.
 - Products must be stored in original containers and labeled.
 - Material mixing will be conducted in accordance with the manufacturer's recommendations.
 - When possible, all products will be completely used before properly disposing of the container off-site.
 - The manufacturer's directions for disposal of materials and containers will be followed.
 - The Contractor's site superintendent will inspect materials storage areas regularly to ensure proper use and disposal.
 - Dust generated will be controlled in an environmentally safe manner.

Hazardous Materials

- Products will be kept in original containers unless the container is not resealable and provide secondary containment as applicable.
- Original labels and material safety data sheets will be retained in a safe place to relay important product information.
- If surplus product must be disposed of, manufacturer's label directions for disposal will be followed.

Maintenance and repair of all equipment and vehicles invalve BIDDING PURPOSES ONL PAKOTA oil changes, hydraulic system drain down, de-greasing operations, fuel tank drain down and removal, and other activities which may result in the accidental release of contaminants will be conducted on an impervious surface and under cover during wet weather to prevent the release of

* Spill kits containing appropriate management of the response and cleanup will be maintenance.

contaminants onto the ground.

- Wheel wash water will be collected and allowed to settle out suspended solids prior to discharge. Wheel wash water will not be discharged directly into any stormwater system or stormwater treatment system.
- Potential pH-modifying materials such as: bulk cement, cement kiln dust, fly ash, new concrete washings, concrete pumping, residuals from concrete saw cutting (either wet or dry), and mixer washout waters will be collected on site and managed to prevent contamination of stormwater runoff.

> Spill Control Practices

In addition to the previous housekeeping and management practices, the following practices will be followed for spill prevention and cleanup if needed.

- For all hazardous materials stored on site, the manufacturer's recommended methods for spill cleanup will be clearly posted. Site personnel will be made aware of the procedures and the locations of the information and cleanup supplies.
- Appropriate cleanup materials and equipment will be maintained by the Contractor in the materials storage area on-site. As appropriate, equipment and materials may include items such as brooms, dust pans, mops, rags, gloves, goggles, kitty litter, sand, sawdust, and plastic and metal trash containers specifically for cleanup purposes.
- All spills will be cleaned immediately after discovery and the materials disposed of properly.
- The spill area will be kept well ventilated and personnel will wear appropriate protective clothing to prevent injury from contact with a hazardous substance.
- After a spill a report will be prepared describing the spill, what caused it, and the cleanup measures taken. The spill prevention plan will be adjusted to include measures to prevent this type of spill from reoccurring, as well as clean up instructions in the event of reoccurrences.
- The Contractor's site superintendent, responsible for day-to-day operations, will be the spill prevention and cleanup coordinator.

> Spill Response

The primary objective in responding to a spill is to quickly contain the material(s) and prevent or minimize migration into stormwater runoff and conveyance systems. If the release has impacted on-site stormwater, it is critical to contain the released materials on-site and prevent their release into receiving waters. If a spill of pollutants threatens stormwater or surface water at the site, the spill response procedures outlined below must be implemented in a timely manner to prevent the release of pollutants.

- The Contractor's site superintendent will be notified immediately when a spill or the threat of a spill is observed. The superintendent will assess the situation and determine the appropriate response.
- If spills represent an imminent threat of escaping erosion and sediment controls and entering receiving waters, personnel will be directed to respond immediately to contain the release and notify the superintendent after the situation has been stabilized.

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- Spill kits containing appropriate materials and equipment for spill response and cleanup will be maintained by the Contractor at the site.
- If oil sheen is observed on surface water (e.g. settling ponds, detention ponds, swales), action will be taken immediately to remove the material causing the sheen. The Contractor will use appropriate materials to contain and absorb the spill. The source of the oil sheen will also be identified and removed or repaired as necessary to prevent further releases.
- If a spill occurs the superintendent or the superintendent's designee will be responsible for completing the spill reporting form and for reporting the spill to SDDANR.
- Personnel with primary responsibility for spill response and cleanup will receive training by the Contractor's site superintendent or designee. The training must include identifying the location of the spill kits and other spill response equipment and the use of spill response materials.
- Spill response equipment will be inspected and maintained as necessary to replace any materials used in spill response activities.

5.3 (8b): WASTE MANAGEMENT PROCEDURES

Waste Disposal

 All liquid waste materials will be collected and stored in approved sealed containers. All trash and construction debris from the site will be deposited in the approved containers. Containers will be serviced as necessary, and the trash will be hauled to an approved disposal site or licensed landfill. All onsite personnel will be instructed in the proper procedures for waste disposal and notices stating proper practices will be posted. The Contractor is responsible for ensuring waste disposal procedures are followed.

> Hazardous Waste

 All hazardous waste materials will be disposed of in a manner specified by local or state regulations or by the manufacturer. Site personnel will be instructed in these practices, and the Contractor will be responsible for seeing that these practices are followed.

> Sanitary Waste

Portable sanitary facilities will be provided on all construction sites.
 Sanitary waste will be collected from the portable units which must be secured to prevent tipping and serviced in a timely manner by a licensed waste management Contractor or as required by any local regulations.

5.3 (9): CONSTRUCTION SITE POLLUTANTS The following materials or substances are expected to be present on the site during the construction period. These materials will be handled as noted under the heading "POLLUTION PREVENTION PROCEDURES" (check all that apply). ☐ Concrete and Portland Cement Detergents

□ Paints Metals ⊠ Bituminous Materials Petroleum Based Products Diesel Exhaust Fluid ☐ Cure ☐ Texture □ Chemical Fertilizers Other:

Product Specific Practices

Petroleum Products

All on-site vehicles will be monitored for leaks and receive regular preventive maintenance to reduce the chance of leakage. Petroleum products will be stored in tightly sealed containers which are clearly labeled.

Fertilizers

Fertilizers will be applied only in the amounts specified by the SDDOT. Once applied, fertilizers will be worked into the soil to limit the exposure to stormwater. Fertilizers will be stored in an enclosed area. The contents of partially used fertilizer bags will be transferred to sealable containers to avoid spills.

Paints

All containers will be tightly sealed and stored when not required for use. The excess will be disposed of according to the manufacturer's instructions and any applicable state and local regulations.

Concrete Trucks

Contractors will provide designated truck washout facilities on the site. These areas must be self-contained and not connected to any stormwater outlet of the site. Upon completion of construction, the area at the washout facility will be properly stabilized.

5.3 (10): NON-STORMWATER DISCHARGES

The following non-stormwater discharges are anticipated during the course of this project (check all that apply).

>	Discharges	from	water	line	flushing.

- Pavement wash-water, where no spills or leaks of toxic or hazardous materials have occurred.
- Uncontaminated ground water associated with dewatering activities.

5.3 (11): INFEASIBILITY DOCUMENTATION

If it is determined to be infeasible to comply with any of the requirements of

the Stormwater Permit, the infeasibility determination must be thoroughly documented in the SWPPP.

7.0: SPILL NOTIFICATION

In the event of a spill, the Contractor's site superintendent will make the appropriate notification(s), consistent with the following procedures:

- A release or spill of a regulated substance (includes petroleum and petroleum products) must be reported to SDDANR immediately if any one of the following conditions exists:
 - The release or spill threatens or is able to threaten waters of the state (surface water or ground water)
 - The release or spill causes an immediate danger to human health or safety
 - The release or spill exceeds 25 gallons
 - The release or spill causes a sheen on surface water
 - The release or spill of any substance that exceeds the ground water quality standards of ARSD Chapter 74:54:01
 - The release or spill of any substance that exceeds the surface water quality standards of ARSD Chapter 74:51:01
 - The release or spill of any substance that harms or threatens to harm wildlife or aquatic life
 - The release or spill is required to be reported according to Superfund Amendments and Reauthorization Act (SARA) Title III List of Lists, Consolidated List of Chemicals Subject to Reporting Under the Emergency Planning and Community Right to Know Act, US Environmental Protection Agency.
- To report a release or spill, call SDDANR at 605-773-3296 during regular office hours (8 a.m. to 5 p.m. Central Standard Time). To report the release after hours, on weekends or holidays, call South Dakota Emergency Management at 605-773-3231. Reporting the release to SDDANR does not meet any obligation for reporting to other state, local, or federal agencies. Therefore, you must also contact local authorities to determine the local reporting requirements for releases. A written report of the unauthorized release of any regulated substance. including quantity discharged, and the location of the discharge will be sent to SDDANR within 14 days of the discharge.

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5.4: SWPPP CERTIFICATIONS

Certification of Compliance with Federal, State, and Local Regulations

The Storm Water Pollution Prevention Plan (SWPPP) for this project reflects the requirements of all local municipal jurisdictions for storm water management and sediment and erosion control as established by ordinance, as well as other state and federal requirements for sediment and erosion control plans, permits, notices or documentation as appropriate.

> South Dakota Department of Transportation

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

The Between

Authorized Signature (See the General Permit, Section 7.4 (1))

Prime Contractor

This section is to be executed by the General Contractor after the award of the contract. This section may be executed any time there is a change in the Prime Contractor of the project.

I certify under penalty of law that this document and all attachments will be revised or maintained under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Authorized	Signature
Auii 1011260	Juliature

CONTACT INFORMATION

The following personnel are duly authorized representatives and have signatory authority for modifications made to the SWPPP:

> Contractor Information:

	Prime Contractor Name:		
	Contractor Contact Name: _		
	Address:		_
	City:	State:	Zip:
	Office Phone:	Field:	
	Cell Phone:	Fax:	
Er	osion Control Supervisor		
•	Name:		-
	Address:		
	City:	State:	Zip:
	Office Phone:	Field:	
	Cell Phone:	Fax:	
SI	DDOT Project Engineer		
•	Name:		-
	Business Address:		
	Job Office Location:		

City: _____State: ____Zip: ____

Office Phone: ______Field: _______

Cell Phone:
Fax:

> SDDANR Contact Spill Reporting

- Business Hours Monday-Friday (605) 773-3296
- Nights and Weekends (605) 773-3231

> SDDANR Contact for Hazardous Materials.

(605) 773-3153

> National Response Center Hotline

(800) 424-8802.

> SDDANR Stormwater Contact Information

- SDDANR Stormwater (800) 737-8676
- Surface Water Quality Program (605) 773-3351

5.5: REQUIRED SWPPP MODIFICATIONS

> 5.5 (1): Conditions Requiring SWPPP Modification

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Ľ	Y SOUTH DAKOTA	P 6403(10)	23	333

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The SWPPP must be modified, including the site map(s), in response to any of the following conditions:

- When a new operator responsible for implementation of any part the SWPPP begins work on the site.
- When changes to the construction plans, sediment and erosion control measures, or any best management practices on site that are no longer accurately reflected in the SWPPP. This includes changes made in response to corrective actions triggered by inspections.
- To reflect areas on the site map where operational control has been transferred (including the date of the transfer) or has been covered under a new permit since initiating coverage under this general permit.
- If inspections by site staff, local officials, SDDANR, or U.S. EPA determine that SWPPP modifications are necessary for compliance with the Stormwater Permit.
- To reflect any revisions to applicable federal, state, or local requirements that affect the control measures implemented at the site
- If approved by the Secretary, to reflect any changes in chemical water treatment systems or controls, including the use of a different water treatment chemical, age rates, different areas, or methods of application.

> 5.5 (2): Deadlines for SWPPP Modification

Any required revisions to the SWPPP must be completed within 7 calendar days following any of the items listed above.

> 5.5 (3): Documentation of Modifications to the Plan

All SWPPP modification records are required to be maintained showing the dates of when the modification occurred. The records must include the name of the person authorizing each change and a brief summary of all changes.

> 5.5 (4): Certification Requirements

All modifications made to the SWPPP must be signed and certified as required in Section 7.4.

> 5.5 (5): Required Notice to Other Operators

If there are multiple operators at the site, the Contractor's Erosion Control Supervisor must notify each operator that may be impacted by the change to the SWPPP within 24 hours.

When modifications as described above occur, the SWPPP will be modified to provide appropriate protection to disturbed areas, all storm water structures, and adjacent waters. The SDDOT Project Engineer will modify the SWPPP using the DOT 298 form and drawings on the plan will be modified to reflect the needed changes. Copies of the DOT 298 forms and the SWPPP will be retained on site in a designated place for review throughout the course of the project. A copy of the DOT 298 form will be given to the Contractor Erosion Control Supervisor and a copy will be emailed to the SDDOT Environmental Section in accordance with the DOT 298 Form.

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FENCE QUANTITIES

Г	STATE OF	PROJECT	SHEET	TOTAL
ı	SOUTH		OHLL	SHEETS
	DAKOTA	P 6403(10)	24	333
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Plotting Date: 2/6/2023

					TAB	LE C	FFE	NCE	QUA	ANTI	TIES						
		Т		Right-of-\	Way Fence					Post Panels			Gates				
Station t	o Station	Side (L/R)	620E0010 Type 1 (FT)	620E0020 Type 2 (FT)	620E0030 Type 3 (FT)	620E0260 Type 6M (FT)	620E4100 Fence Reset (FT)	110E600 Remove Fence (FT)	110E7802 Remove Fence for Reset (FT)	620E1020 2 Post (Each)	620E1030 3 Post (Each)	Barb Wire 16 ft (Each)	Barb Wire 18 ft (Each)	Barb Wire 20 ft (Each)	Barb Wire 24 ft (Each)	Wood 28 ft (Each)	Barb Wire 32 ft (Each)
31+34	133+17	L		10,183				10,183		13	3		1	1	1		
Horizontal Jog	104+75	L		50				50									
218+25	271+16	L	5,291					5,291		3	6		2				
Horizontal Jog	230+05	L	80					80									
Horizontal Jog	246+85	L	150					150									
415+48	431+44	L	1,596					1,596		1	12	1			1		1
Horizontal Jog	415+48	L	86					86									
13+68	23+94	R				1,026		1,026			2						
24+15	25+52	R*					137		137								
25+52	54+42	R			2,890			2,890		3	6	1			1		
54+42	132+52	R		7,810				7,810		5	6		1	2			
Horizontal Jog	105+30	R		75				75									
218+40	225+10	R	670					670			1			1			
225+10	229+82	R**					472		472								
229+82	232+67	R*					285		285							2	
232+67	288+85	R	5,618					5,618		14	12	1		1	3		
Horizontal Jog	236+12	R	80					80									
Horizontal Jog	239+85	R	40					40									
Horizontal Jog	287+35	R	80					80									
334+02	455+40	R	12,138					12,138		26	11	6	3	5			
455+62	460+40	R*					478		478								
Horizontal Tie-Ins		L/R	180	90	90	90		450									
	Т	OTALS:	26,009	18,208	2,980	1,116	1,372	48,313	1,372	65	59	9	7	10	6	2	1

Post Type and Sequence:

Right-of-way fence shall be constructed using alternate wood and steel posts except as noted.

- * Reset Wood Fence
- ** Reset Steel Panel Fence

Existing Bison Fence from approximate Station 133+00 to approximate Station 218+50 shall remain undisturbed



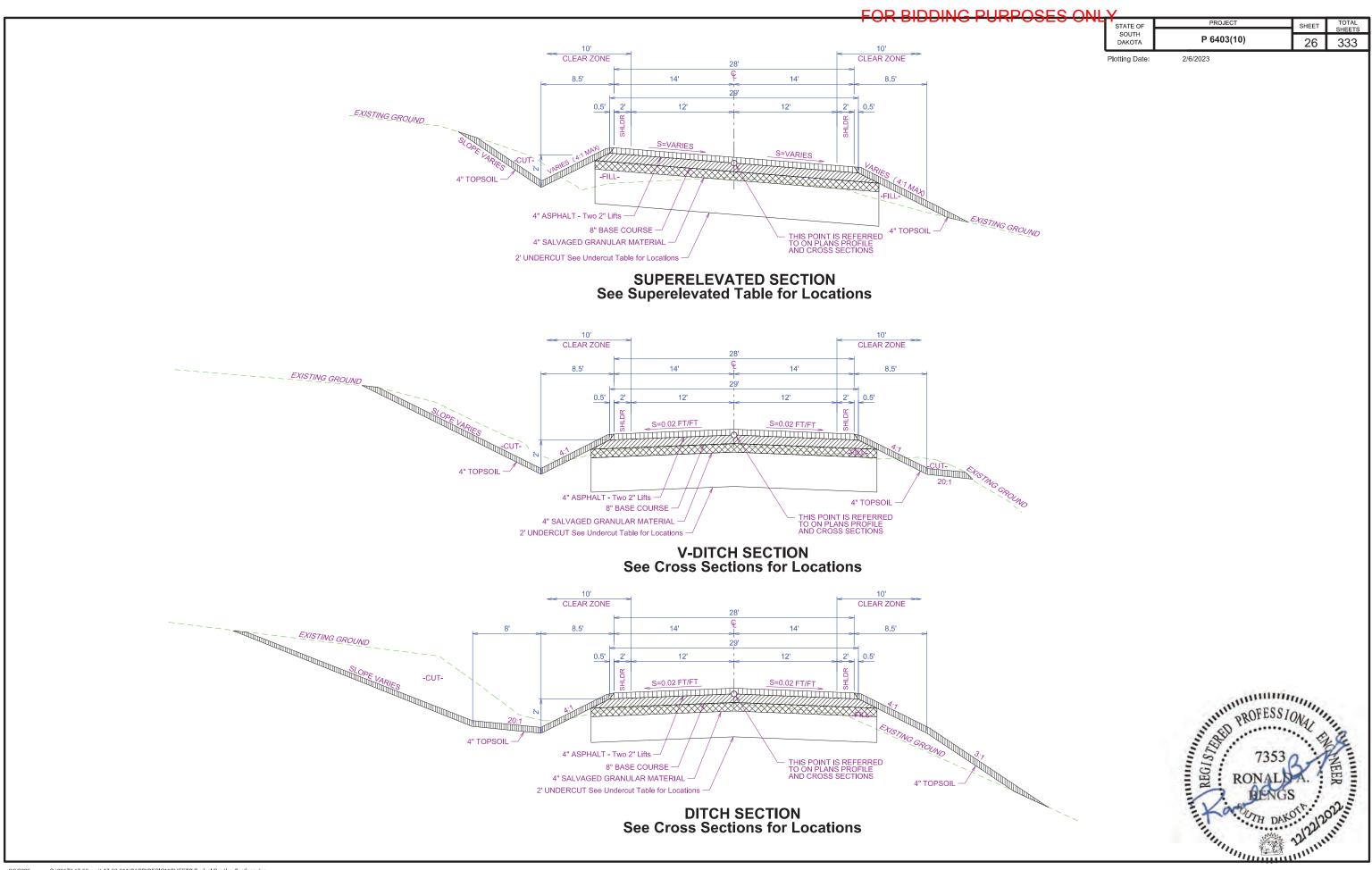
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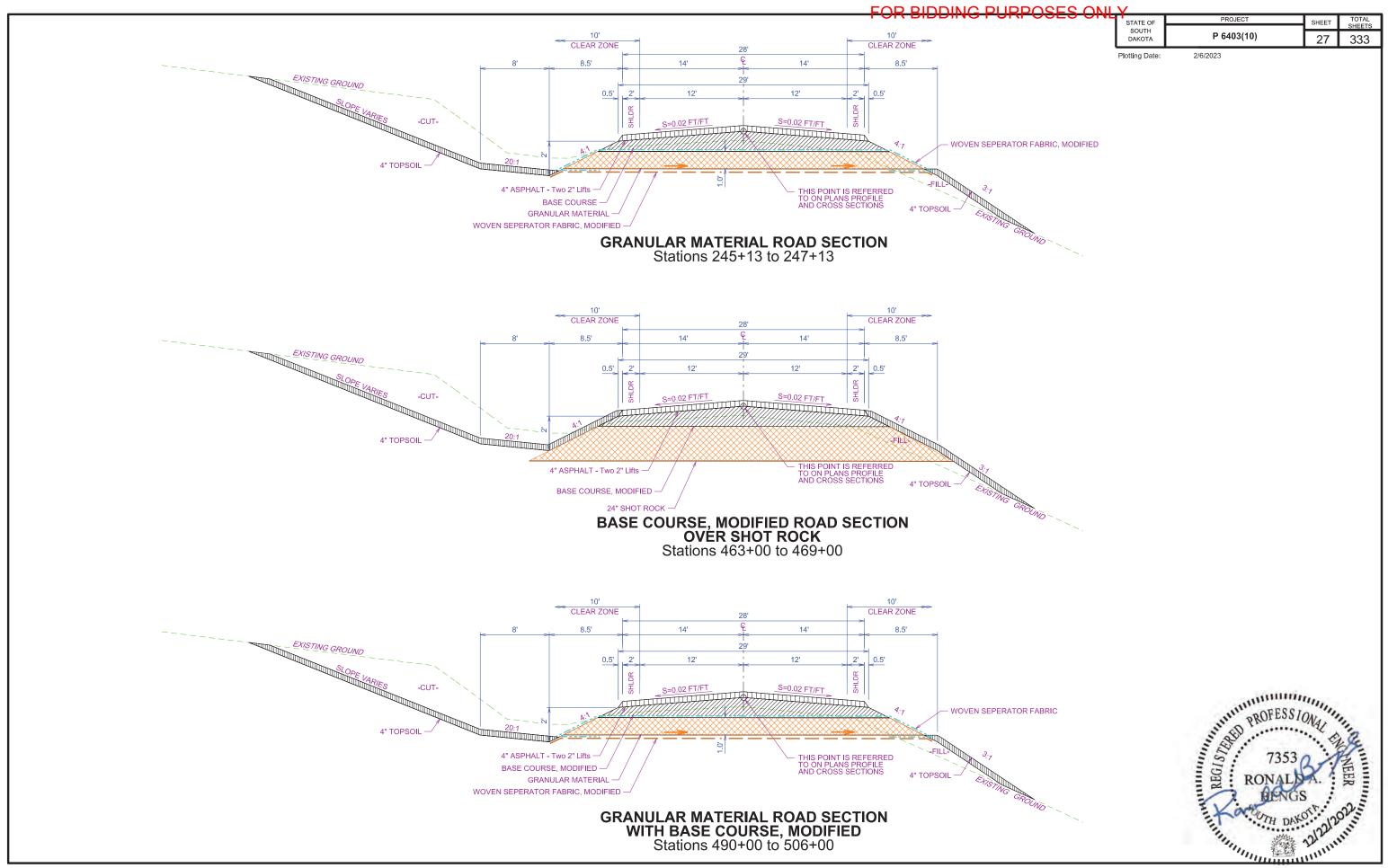
1	STATE OF SOUTH	PROJECT	SHEET	TOTAL SHEETS
	DAKOTA	P 6403(10)	25	333

Plotting Date:	2/6/202

				PIPE (NAUÇ	TITIES	3					
1	450E4758	450E4768	450E4778	450E 4788	450E4798	450E 5211	450E 5406	450E 5215	450E 5015	450E 5219	450E 5420	450E 5227
	450E 4760	450E 4770	450E 4780	450E 4790	450E 4800	450E 5212	450E 5407	450E 5216	450E 5016	450E 5220	450E 5423	450E 5228
<u> </u>	18"	24"	30"	36"	42"	18"	18"	24"	24"	30"	36"	42"
	14 Ga	14 Ga	14 Ga	14 Ga	14 Ga	Flared End	Safety End	Flared End	Elbow	Flared End	Safety End	Flared End
					3x1				45°			
Station Offset (L/R)	Ft	Ft	Ft	Ft	Ft	Each	Each	Each	Each	Each	Each	Each
4+76 Cross Pipe	62.00					2						
9+48 Cross Pipe 9+84 23ft L Approach Pipe	50.00 56.00					2	2					
16+73 Cross Pipe	50.00					2						
22+94 Cross Pipe	62.00					2						
27+69 Cross Pipe	44.00					2						
29+21 22 ft L Approach Pipe	66.00						2					
32+68 54 ft R Approach Pipe	54.00						2					
57+12 Cross Pipe	72.00					2						
67+79 Cross Pipe	120.00					2						
72+00 25 ft R Approach Pipe 81+76 Cross Pipe	106.00 72.00					2	2			+	-	
86+06 24 ft R Approach Pipe	62.00						2					
113+58 44 ft L Approach Pipe	50.00						2					
114+59 Cross Pipe			74.00							2		
115+00 54 ft L Approach Pipe		50.00						2				
116+29 43 ft L Approach Pipe	40.00						2					
139+84 Cross Pipe		74.00				_		2				
142+78 Cross Pipe	88.00					2						
165+00 Cross Pipe	74.00 40.00					2						
177+28 Cross Pipe 186+18 Cross Pipe	60.00					2			-	+	1	
197+27 23 ft R Approach Pipe	70.00					2	2					
197+37 33 ft L Approach Pipe	40.00						2					
200+44 Cross Pipe	54.00					2						
230+78 37 ft L Approach Pipe	26.00						2					
232+33 Cross Pipe	92.00					2						
242+04 32 ft L Approach Pipe		48.00						2				
242+75 39 ft L Approach Pipe		18.00						2				
243+66 Cross Pipe 246+12 Cross Pipe		70.00			76.00			2	1			1
256+44 Cross Pipe	64.00				70.00	2						'
268+35 Cross Pipe	56.00					2						
271+46 21 ft L Approach Pipe	66.00						2					
283+23 Cross Pipe	70.00					2						
290+94 Cross Pipe	94.00					2						
292+12 23 ft L Approach Pipe	58.00						2					
320+31 23 ft R Approach Pipe	48.00						2					
332+32 23 ft L Approach Pipe 351+88 33 ft L Approach Pipe	56.00 60.00						2			 	 	
363+09 Cross Pipe	50.00	56.00						2			 	
371+71 31 ft L Approach Pipe	38.00	55.00					2					
373+73 Cross Pipe	54.00					2						
379+78 Cross Pipe	66.00					2						
388+18 Cross Pipe	50.00					2						
408+47 Cross Pipe	56.00					2						
417+53 94 ft L Approach Pipe	36.00 24.00						2					
418+99 47 ft L Approach Pipe 424+30 Cross Pipe	56.00					2						
424+88 26 ft L Approach Pipe	52.00						2					
435+83 Cross Pipe	42.00					2						
442+79 22 ft L Approach Pipe	52.00						2					
448+92 Cross Pipe	72.00					2						
456+96 Cross Pipe	42.00					2						
468+06 Cross Pipe	62.00					2						
483+66 Cross Pipe	62.00					2						
492+93 Cross Pipe	56.00					2						
499+36 Cross Pipe 503+30 Cross Pipe	56.00 56.00					2			-	-		
510+48 Cross Pipe	50.00		86.00							2		
527+65 Cross Pipe				66.00						<u> </u>	2	
				64.00					1		2	
530+17 Cross Pipe												



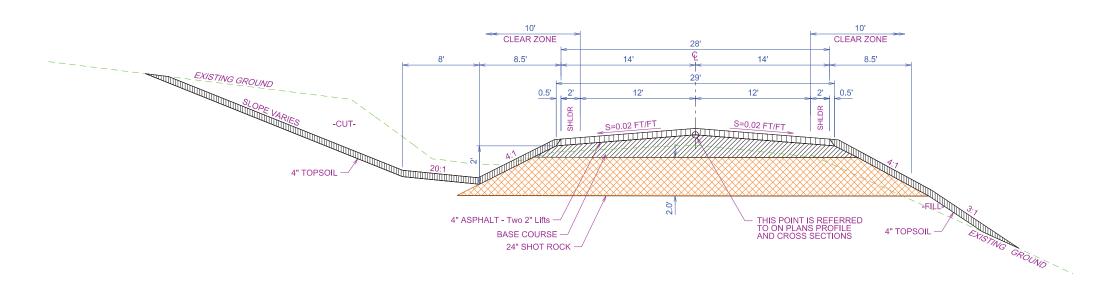




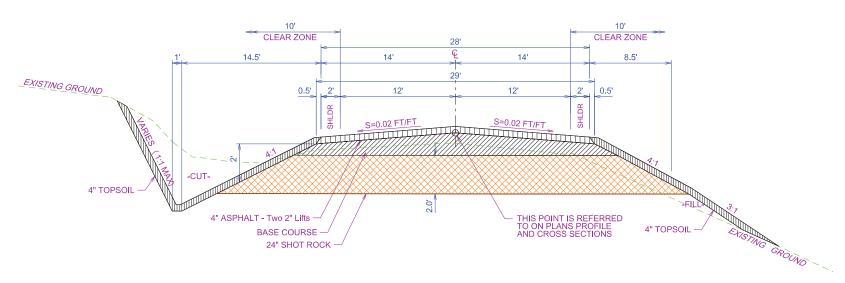
FOR RIDDING PURPOSES ONLY

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	P 6403(10)	28	333

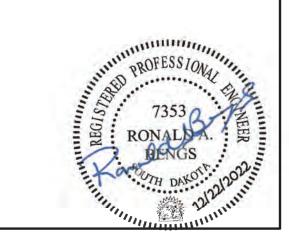
2/6/2023 Plotting Date:



SHOT ROCK ROAD SECTION
Stations 386+00 to 388+50, 395+50 to 396+50, 404+75 to 406+25, 447+75 to 449+25, 455+50 to 463+00, & 482+25 to 484+75



SHOT ROCK ROAD SECTION Station 510+50 to 512+00

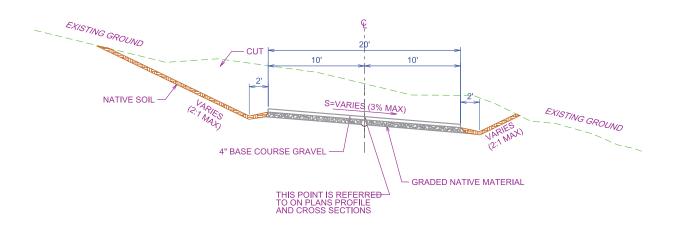


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STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	P 6403(10)	29	333

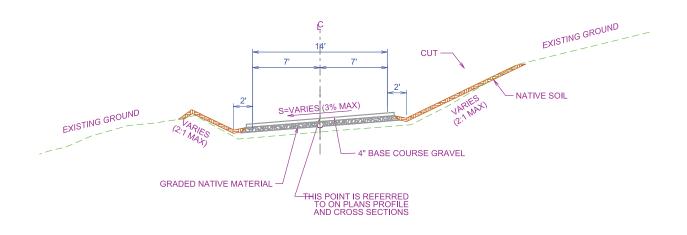
Plotting Date: 2/6/2023

Google Earth Image: 2016 Image Not To scale



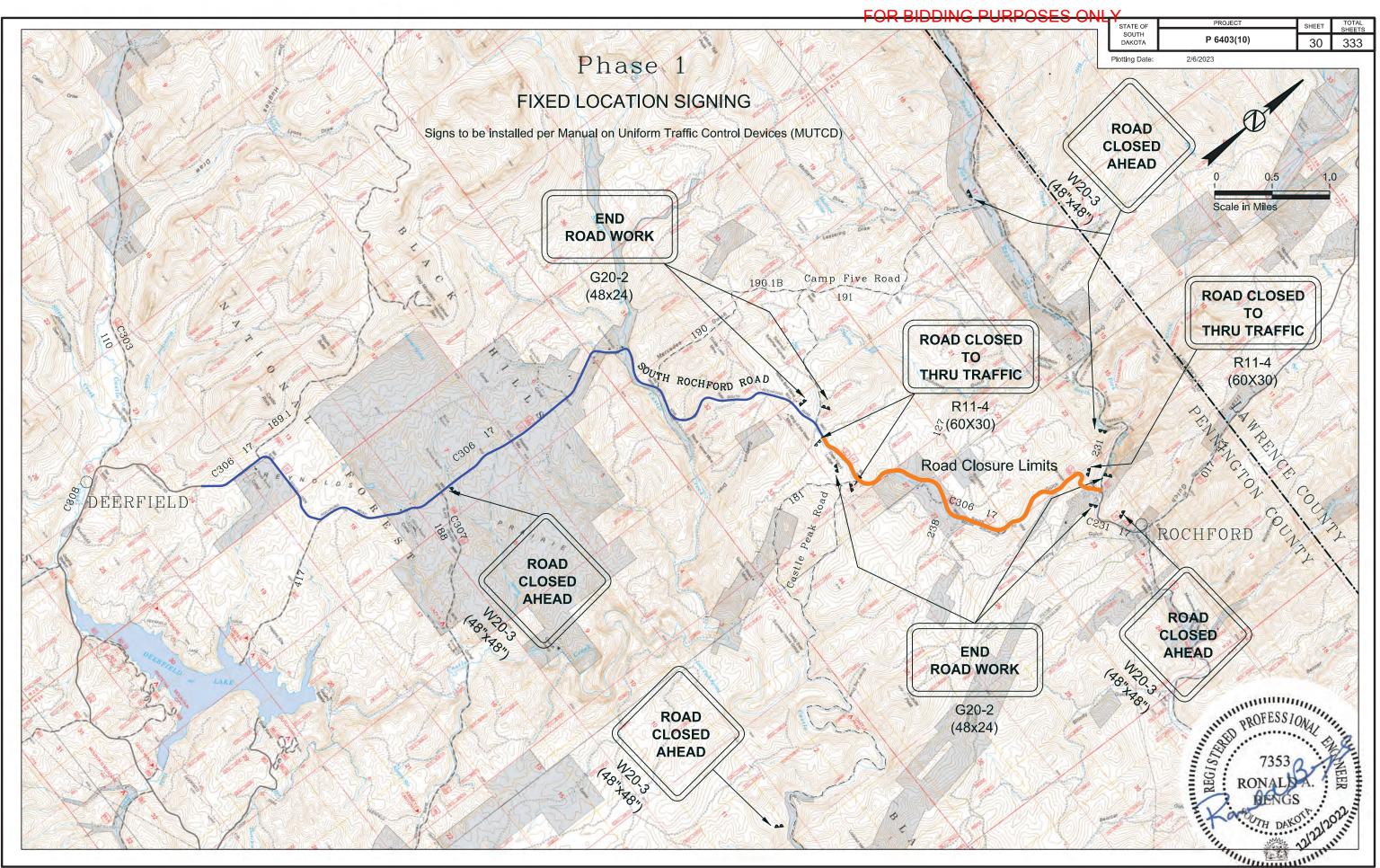
MUSICK DRIVEWAY SECTION STATION 4+75* TO 6+50

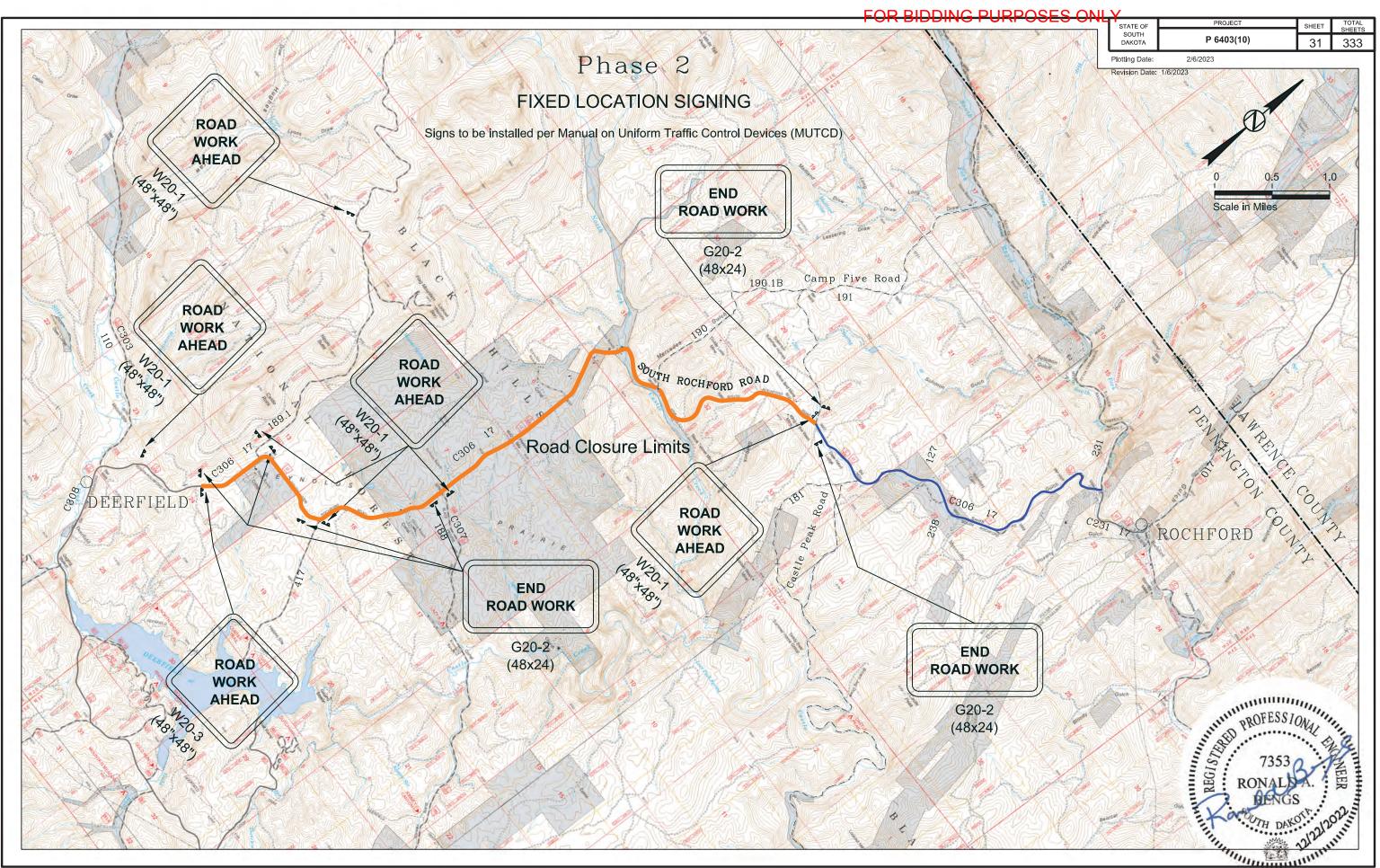
*TRANSITION FROM 3+20 TO 4+75

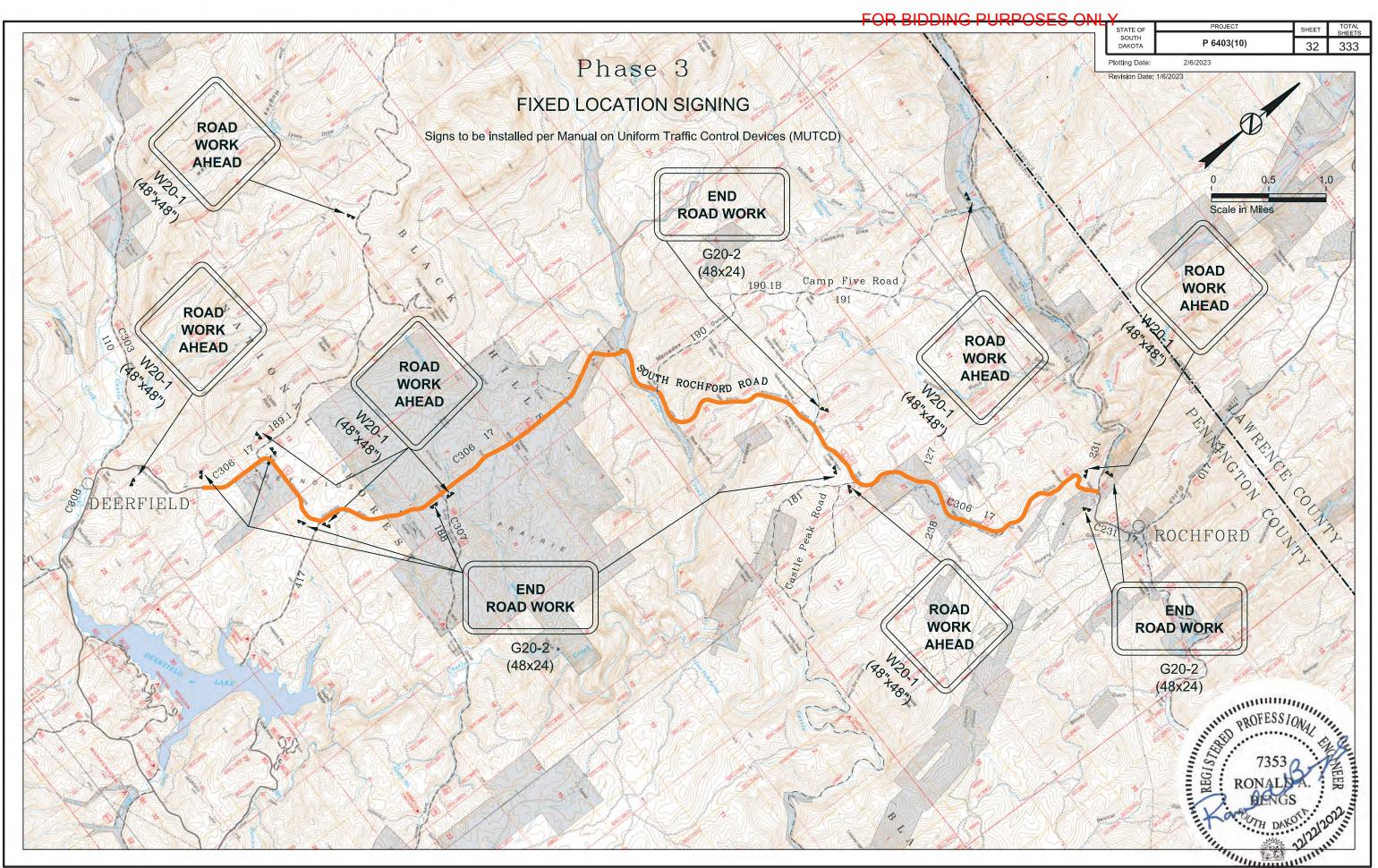


MUSICK DRIVEWAY SECTION STATION 0+00 TO 3+20*







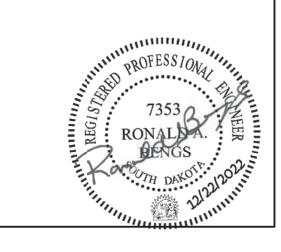


FOR BIDDING PURPOSES ONLY

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₹	STATE OF	PROJECT	SHEET	TOTAL
п				SHEETS
-	SOUTH	D 6402/40\		
ı	DAKOTA	P 6403(10)	33	I 333

Plotting Date: 2/6/2023 Revised: 02/06/2023

ITEMIZED LIST FOR TRAFFIC CONTROL SIGNS					
		CONVENTIONAL ROAD			
				SQFT PER	
SIGN CODE	SIGN DESCRIPTION	QUANTITY	SIGN SIZE	SIGN	SQFT
R11-4	ROAD CLOSED TO THRU TRAFFIC	3	60"X30"	12.5	37.5
W3-4	BE PREPARED TO STOP	2	48"x48"	16	32
W8-1	BUMP	2	48"x48"	16	32
W8-7	LOOSE GRAVEL	2	48"x48"	16	32
W8-11	UNEVEN LANES	2	48"x48"	16	32
W20-1	ROAD WORK AHEAD	10	48"x48"	16	160
W20-3	ROAD CLOSED AHEAD	5	48"x48"	16	80
W20-4	ONE LANE ROAD AHEAD	2	48"x48"	16	32
W20-7	FLAGGER (Symbol)	2	48"x48"	16	32
W21-3	ROAD MACHINERY AHEAD	2	48"x48"	16	32
W22-1	BLASTING ZONE AHEAD	2	48"x48"	16	32
W22-2	TURN OFF 2-WAY RADIO AND CELL PHONE	2	42"X36"	10.5	21
W22-3	END BLASTING ZONE	2	36"x18"	5	10
G20-2	END ROAD WORK	10	36"x18"	5	50
SPECIAL	WINDROW	2	48"x48"	16	32
SPECIAL	WAIT FOR PILOT CAR	2	36"x36"	9	18
SPECIAL*	ADVANCED CLOSURE*	5	93"x27"	17.44	87.2
	ROAD TRAFFIC CONTROL SIGNS SQFT				751.7



EROSION AND SEDIMENT CONTROL LEGEND

STATE OF SHEET P 6403(10) 34 333 DAKOTA

2/6/2023

Plotting Date:

SYMBOLOGY FOR BEST MANAGEMENT PRACTICES STORM WATER DISCHARGE POINT LOW FLOW SILT FENCE HIGH FLOW SILT FENCE SILT TRAP otSEDIMENT CONTROL AT INLET WHEN SURFACING IS IN PLACE TEMPORARY SEDIMENT BARRIER ∞ TEMPORARY WATER BARRIER $\wedge \wedge \wedge$ FLOATING SILT CURTAIN SEDIMENT FILTER BAGS TRIANGULAR SILT BARRIERS **EROSION CONTROL WATTLES EROSION BALES** SURFACE ROUGHENING $\times\!\!\times\!\!\times$ SOIL STABILIZER / TEMPORARY MULCH / DUST CONTROL >>>> **CUT INTERCEPTOR DITCH** TEMPORARY SLOPE DRAIN SEDIMENT CONTROL AT INLET BEFORE PLACEMENT OF SURFACING HYDRAULIC STRAW MULCH / FIBER MULCHING / BONDED FIBER MATRIX / FIBER REINFORCED MATRIX SS0 ROCK CHECK DAM SODDING TYPE 1 EROSION CONTROL BLANKET \bowtie TYPE 2 EROSION CONTROL BLANKET TYPE 3 EROSION CONTROL BLANKET TYPE 4 EROSION CONTROL BLANKET TYPE 1 TURF REINFORCEMENT MAT TYPE 2 TURF REINFORCEMENT MAT ATYPE 3 TURF REINFORCEMENT MAT SYNTHETIC CHANNEL PROTECTION (TS) TOPSOIL STOCKPILES В **BORROW AREAS** (c<u>i</u>) STABILIZED CONSTRUCTION ENTRANCES (CV) CONCRETE WASHOUTS (BS) **VEGETATED BUFFER STRIPS** AP **ASPHALT PLANT SITE** CP CONCRETE PLANT SITE ON-SITE CONSTRUCTION MATERIAL STORAGE AREAS (sk) SPILL KIT (w) WORK PLATFORM (TP) PORTABLE TOILET VEHICLE AND EQUIPMENT PARKING, FUELING, AND MAINTENANCE AREAS

DUMPSTER OR OTHER TRASH AND DEBRIS CONTAINERS

BEST MANAGEMENT PRACTICES

BEST MANAGEMENT PRACTICES (BMP'S) SHOULD BE USED THROUGHOUT CONSTRUCTION. TO REMIND CONTRACTORS AND FIELD PERSONNEL THAT BMP'S FOR WATER QUALITY SHOULD BE UTILIZED THROUGHOUT THE CONSTRUCTION PROCESS, THE SYMBOLOGY IS COLORED AS FOLLOWS:

RED BMPS ARE TO BE INSTALLED BEFORE EARTH MOVING ACTIVITES COMMENCE. RED BMPS ARE USED. FOR PERIMETER CONTROL, THEY PREVENT SEDIMENT FROM LEAVING THE SITE OR ENTERING FROM ANOTHER SITE. THEY MAY ALSO DETER WATER AWAY FROM OR AROUND THE SITE. THEY MAY BE LEFT IN PLACE AND MAINTAINED FOR THE REMAINDER OF CONSTRUCTION OR UNTIL VEGETATION HAS REACHED 70% OF THE BACKGROUND LEVEL.

BLUE BMPS ARE TO BE INSTALLED DURING CONSTRUCTION. BLUE BMPS ARE USED FOR TEMPORARY STABILIZATION. THEY PREVENT EROSION DURING CONSTRUCTION. THEY MAY ALSO BE SEDIMENT CONTROLS UTILIZED AFTER DRAIN PIPES AND STORM SEWERS ARE IN PLACE, THEY MAY BE LEFT IN PLACE AND MAINTAINED FOR THE REMAINDER OF CONSTRUCTION OR UNTIL VEGETATIONHAS REACHED 70% OF THE BACKGROUND LEVEL. SOME YELLOW BMPS WILL BE REMOVED OR REPLACED DURING CONSTRUCTION.

GREEN BMPS ARE TO BE INSTALLED WHEN GRADING IS COMPLETE, GREEN BMPS ARE USED FOR FINAL STABILIZATION, THEY ARE PERMANENT EROSION CONTROL MEASURES THAT ARE NOT REMOVED.

IF THE CONTRACTOR OR ENGINEER DECIDE TO USE ADDITIONAL BEST MANAGEMENT PRACTICES OR LABEL THE LOCATIONS OF THEM THEY SHOULD USE THE SYMBOLOGY SHOWN. OTHER BEST MANAGEMENT PRACTICES FOR WHICH THERE IS NO SYMBOLOGY INCLUDE:

PERMANENT SEEDING IS DONE BEFORE THE APPLICATION OF ALL TYPES OF MULCHING AND HYDRAULICALLY APPLIED SOIL MULCHES AND MATRIXS. PERMANENT GRASS HAY/ STRAW MULCH IS NOT SHOWN ON PLAN SHEETS, BUT IT CAN BE ASSUMED THAT ALL AREAS THAT ARE NOT ROADWAYS ON RURAL PROJECTS WILL BE SEEDED THEN MULCHED. AREAS WHERE AN ALTERNATE TO GRASS HAY /STRAW MULCH IS USED WILL BE SHOWN WITH THE APPROPRIATE SYMBOLOGY.

SEDIMENT BASINS UTILIZED DURING CONSTRUCTION WILL BE SHOWN ON PLAN SHEETS AND IN SECTION X.

GEOTEXTILE FABRIC USUALLY SUPPLEMENTS OTHER BMPS, BUT IT MAY BE USED TO TEMPORARILY COVER AREAS FOR EROSION PROTECTION UNTIL IT IS PERMENANTLY INSTALLED.

STREET SWEEPING SHOULD BE DONE AS NEEDED TO KEEP SEDIMENT ON ROADWAYS FROM LEAVING THE SITE.

DEWATERING AND SEDIMENT COLLECTING IS SHOWN ON A DETAIL SHEET WHEN IT IS NEEDED. DEWATERING WITHOUT SEDIMENT COLLECTING DOES NOT HAVE A DETAIL, JUST A DETAILED NOTE. SEDIMENT LADEN WATER SHOULD NEVER BE PUMPED OFF THE SITE.

GABIONS AND RIP RAP AT PIPE AND CULVERT OUTLETS ARE DETAILED IN SECTION B.

PROJECT PHASING

PROJECT PHASING MAY BE ONE OF THE MOST IMPORTANT BMPS. DURING PHASING REMEMBER THE FOLLOWING:

ALWAYS INSTALL PERIMETER CONTROLS BEFORE BEGINNING EARTH MOVING ACTIVITIES.

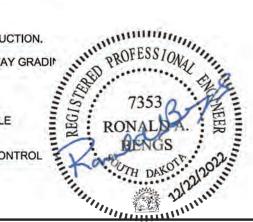
DO NOT DISTURB MORE AREA THAN WHAT IS NEEDED TO COMPLETE EACH PHASE OF CONSTRUCTION.

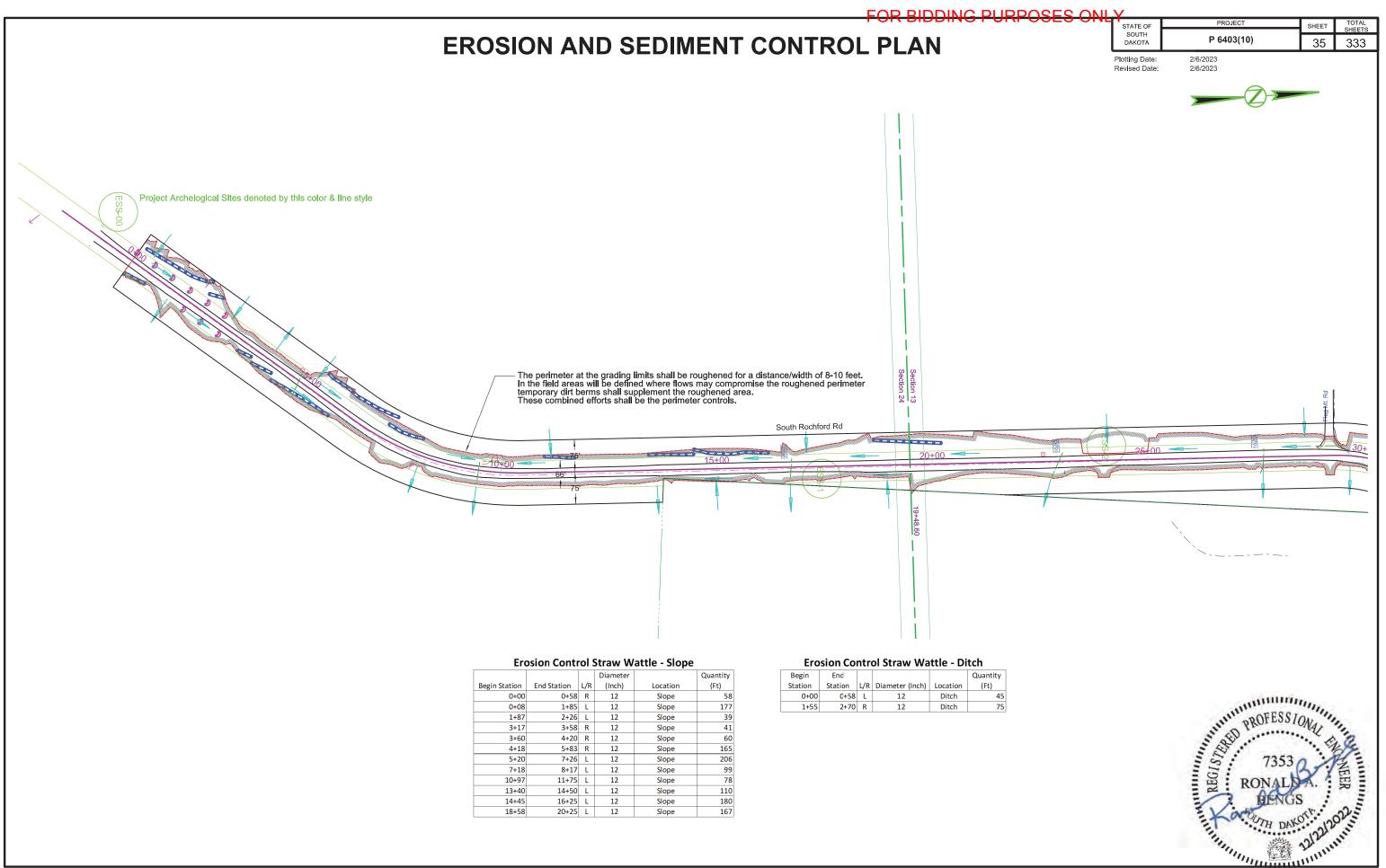
IF POSSIBLE CONSTRUCT SEDIMENT BASINS AND STABILIZE THEM BEFORE BEGINNING ROADWAY GRADIN

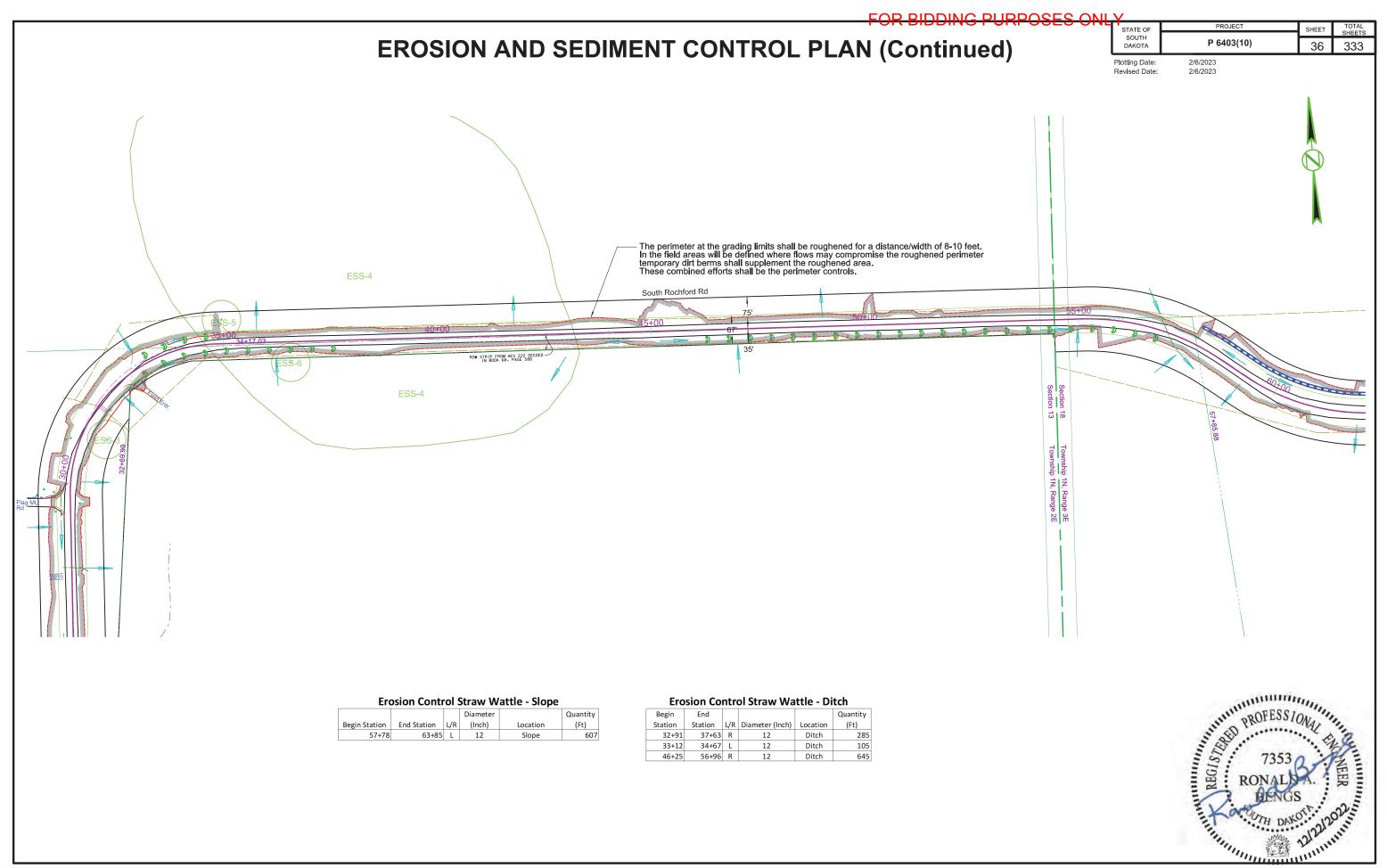
TEMPORARILY STABILIZE AREAS THAT WILL NOT BE TOUCHED WITHIN 14 DAYS.

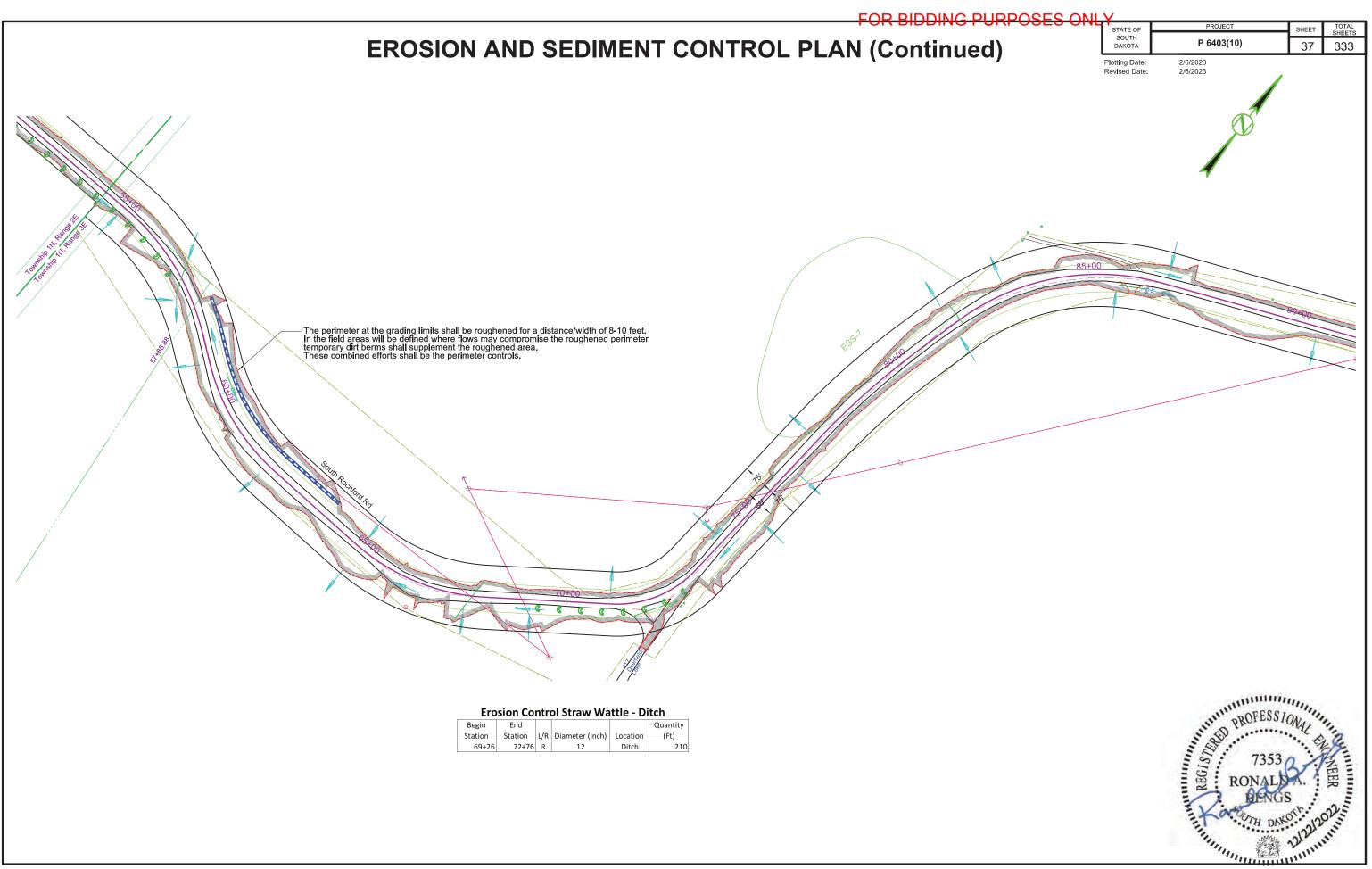
PERMANENTLY STABILIZE AREAS WHEN GRADING IN THAT AREA IS COMPLETE. PERMANENT STABILIZATION CAN BE COMPLETED IN PHASES AND DOES NOT HAVE TO WAIT UNTIL THE WHOLE ROADWAY HAS BEEN CONSTRUCTED.

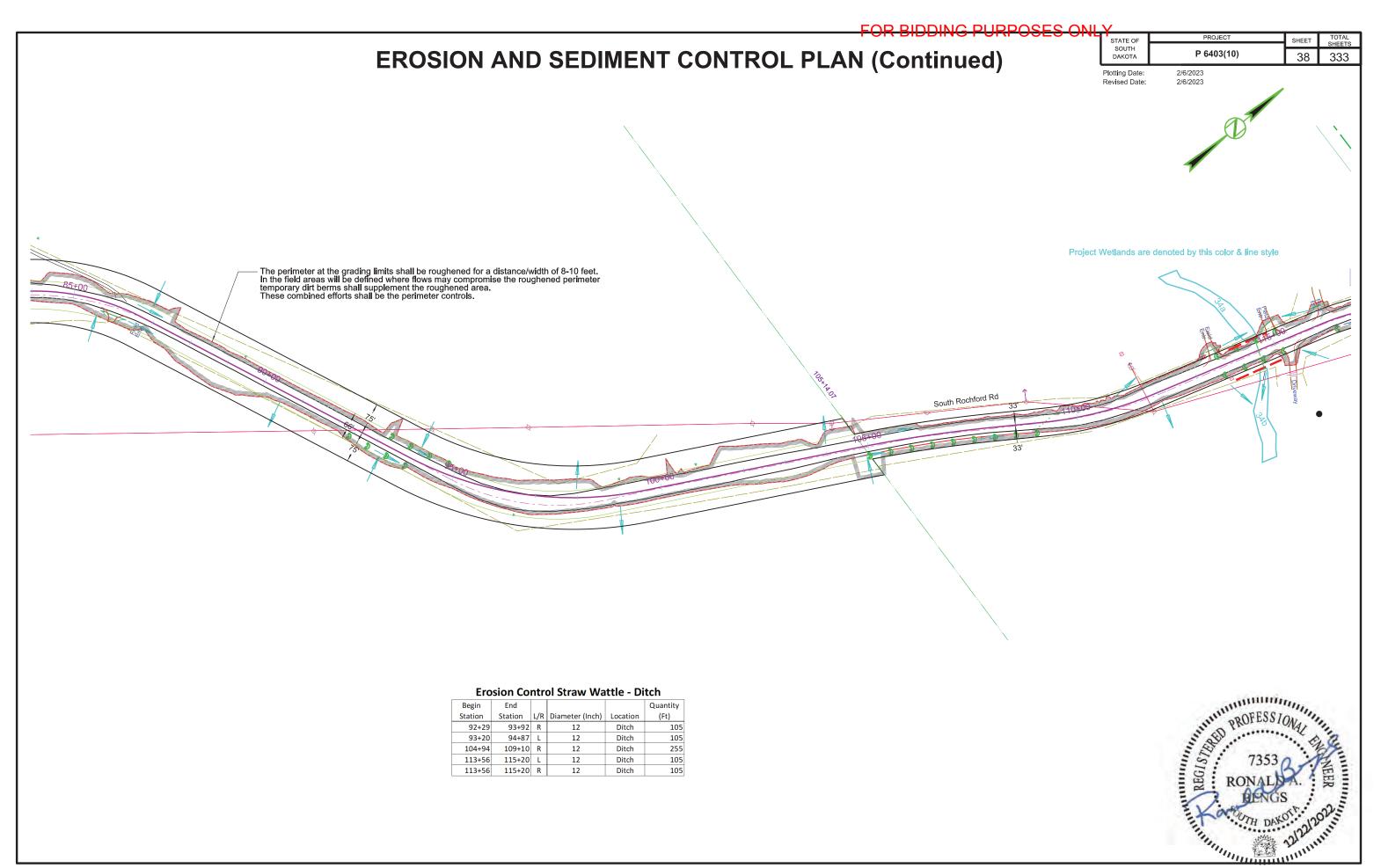
CONTINUALLY MAINTAIN ALL SEDIMENT CONTROLS AND MONITOR AREAS WHERE EROSTION CONTROL HAS BEEN INSTALLED.

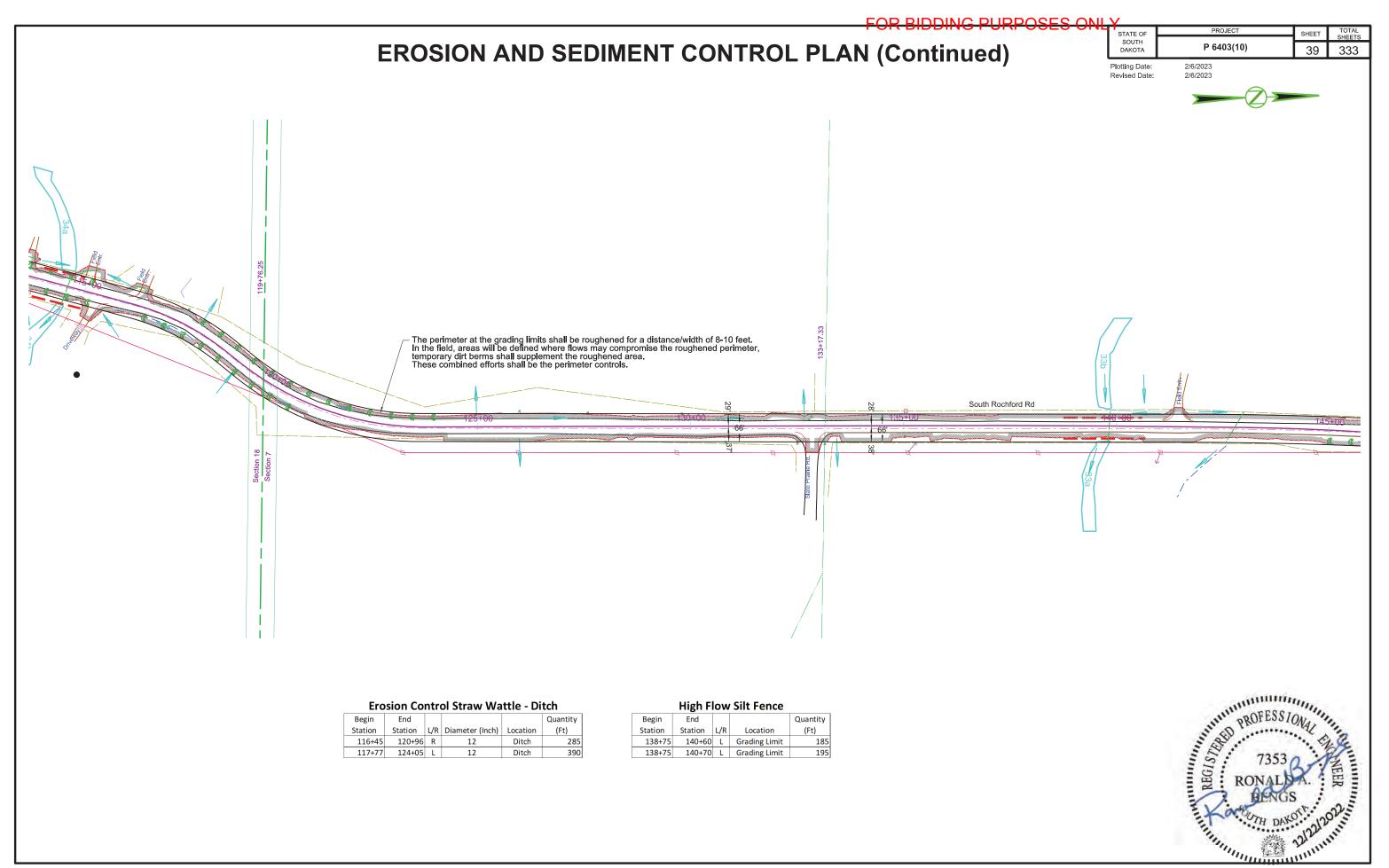


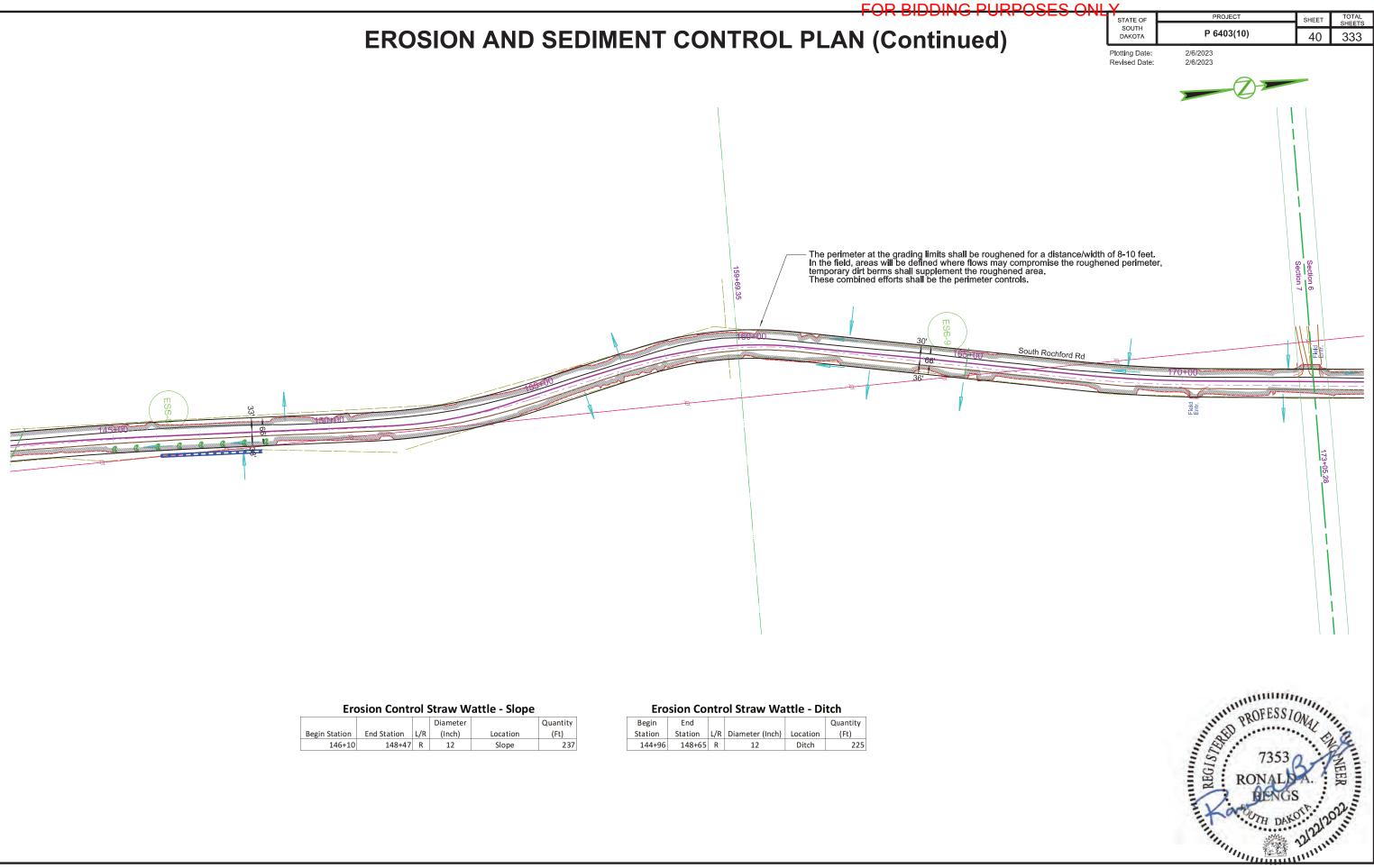












FOR RIDDING PURPOSES ONLY STATE OF SHEET SOUTH **EROSION AND SEDIMENT CONTROL PLAN (Continued)** P 6403(10) 41 333 Plotting Date: Revised Date: - The perimeter at the grading limits shall be roughened for a distance/width of 8-10 feet. In the field, areas will be defined where flows may compromise the roughened perimeter, temporary dirt berms shall supplement the roughened area. These combined efforts shall be the perimeter controls. South Rochford Rd **Erosion Control Straw Wattle - Ditch** Begin End (Ft) Station | Station | L/R | Diameter (Inch) | Location 187+69 191+35 L Ditch 225 12 188+00 191+15 R 195

FOR RIDDING PURPOSES ONLY STATE OF SOUTH **EROSION AND SEDIMENT CONTROL PLAN (Continued)** P 6403(10) 42 Plotting Date: Revised Date: - The perimeter at the grading limits shall be roughened for a distance/width of 8-10 feet. In the field, areas will be defined where flows may compromise the roughened perimeter, temporary dirt berms shall supplement the roughened area. These combined efforts shall be the perimeter controls. **Erosion Control Straw Wattle - Slope Erosion Control Straw Wattle - Ditch High Flow Silt Fence** Quantity Diameter Begin Quantity Begin Quantity (Inch) (Ft) Station L/R Diameter (Inch) (Ft) L/R (Ft) Begin Station End Station L/R Location Station Location Station Station Location 218+48 229+55 L Slope 1,107 224+20 230+16 L Ditch 360 232+00 239+75 L Grading Limits 775

STATE OF SOUTH DAKOTA **EROSION AND SEDIMENT CONTROL PLAN (Continued)** P 6403(10) 43 Plotting Date: Revised Date: - The perimeter at the grading limits shall be roughened for a distance/width of 8-10 feet. In the fleld, areas will be defined where flows may compromise the roughened perimeter, temporary dirt berms shall supplement the roughened area. These combined efforts shall be the perimeter controls. re denoted by this color & line styl **Erosion Control Straw Wattle - Slope High Flow Silt Fence**

			Diameter		Quantity
Begin Station	End Station	L/R	(Inch)	Location	(Ft)
237+00	239+21	R	12	Slope	221
263+20	269+23	L	12	Slope	603

Begin	End			Quantity
Station	Station	L/R	Location	(Ft)
244+00	287+55	R	Grading Limits	4,355
244+75	246+91	L	Grading Limits	216

FOR RIDDING PURPOSES ONLY STATE OF **EROSION AND SEDIMENT CONTROL PLAN (Continued)** P 6403(10) 44 333 2/6/2023 2/6/2023 Plotting Date: Revised Date: - The perimeter at the grading limits shall be roughened for a distance/width of 8-10 feet. In the field, areas will be defined where flows may compromise the roughened perimeter, temporary dirt berms shall supplement the roughened area. These combined efforts shall be the perimeter controls. **Erosion Control Straw Wattle - Slope Erosion Control Straw Wattle - Ditch** Diameter Quantity Quantity Begin Station Begin Station (Inch) (Ft) Station L/R Diameter (Inch) Location End Station L/R Location 284+51 287+75 L Ditch

279+61 L

285+25 L

289+97 L

12

12

12

Slope

Slope

Slope

352 462

418

276+09

280+63

285+79

EROSION AND SEDIMENT CONTROL PLAN (Continued)

STATE OF SOUTH DAKOTA P 6403(10) SHEET TOTAL SHEETS

9 6403(10) 45 333

Plotting Date: 2/6 Revised Date: 2/6





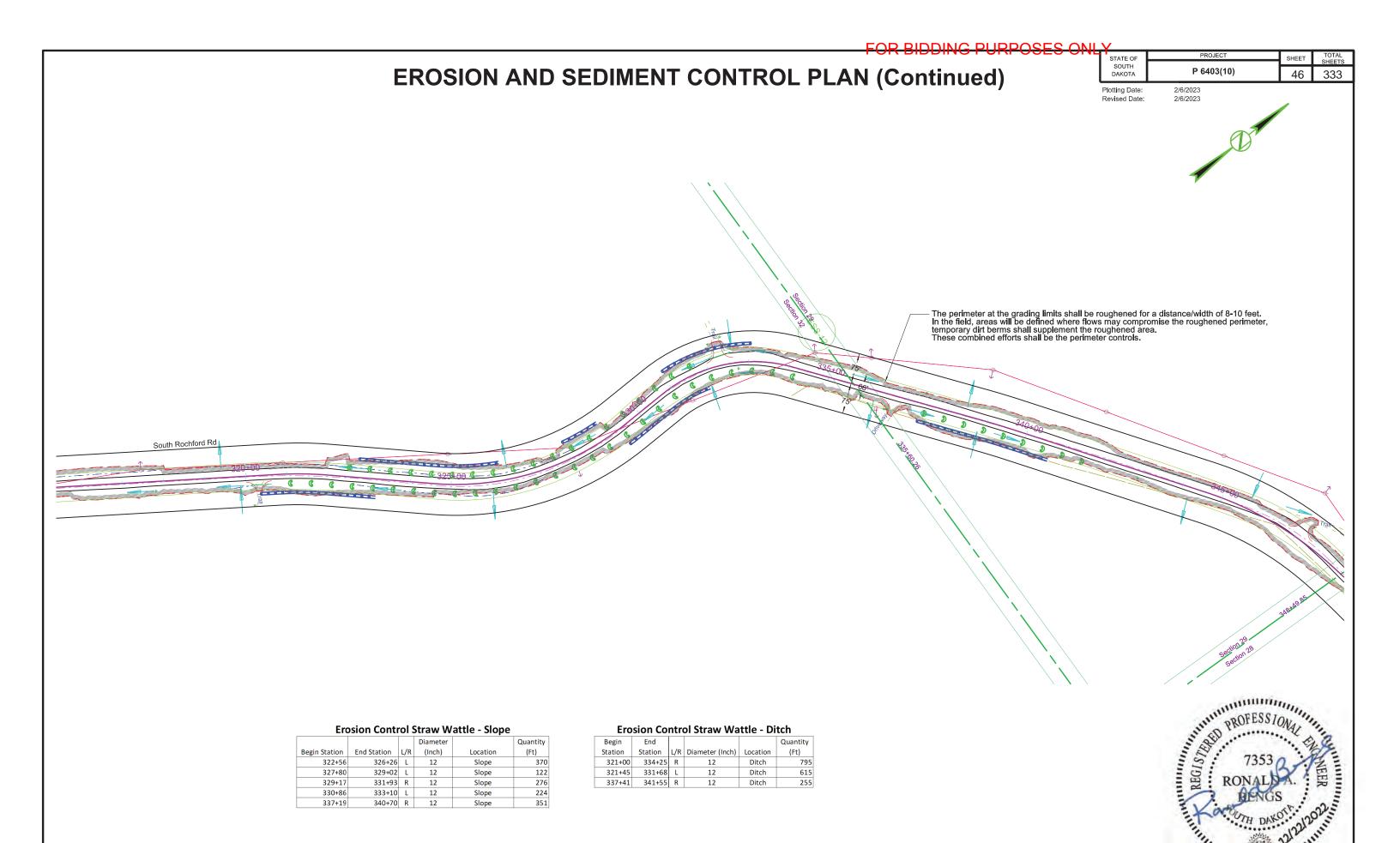
Erosion Control Straw Wattle - Slope

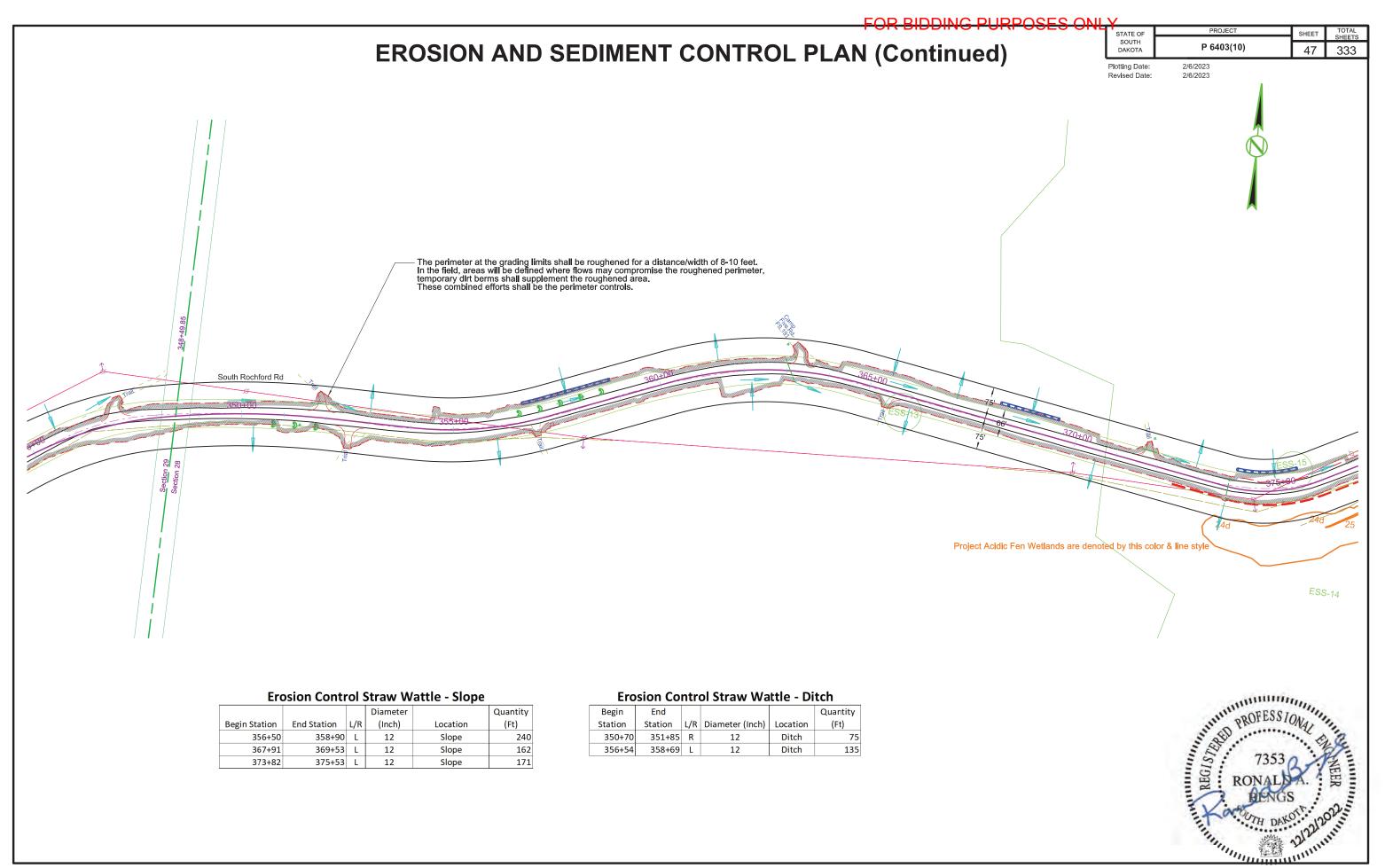
			Diameter		Quantity
Begin Station	End Station	L/R	(Inch)	Location	(Ft)
295+00	298+95	L	12	Slope	395
298+64	301+65	R	12	Slope	301
301+66	314+56	L	12	Slope	1,290
320+18	323+20	R	12	Slope	302

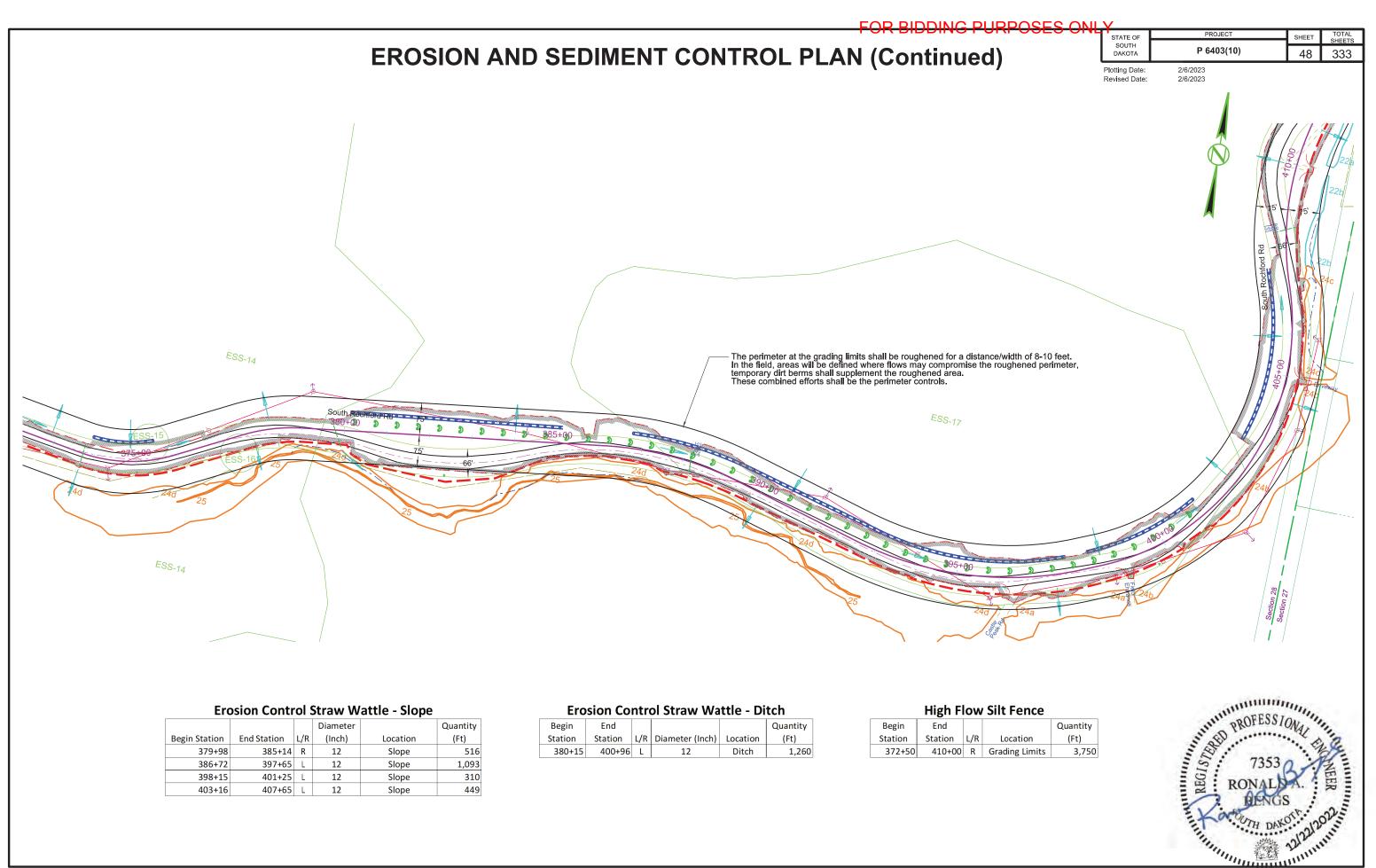
Erosion Control Straw Wattle - Ditch

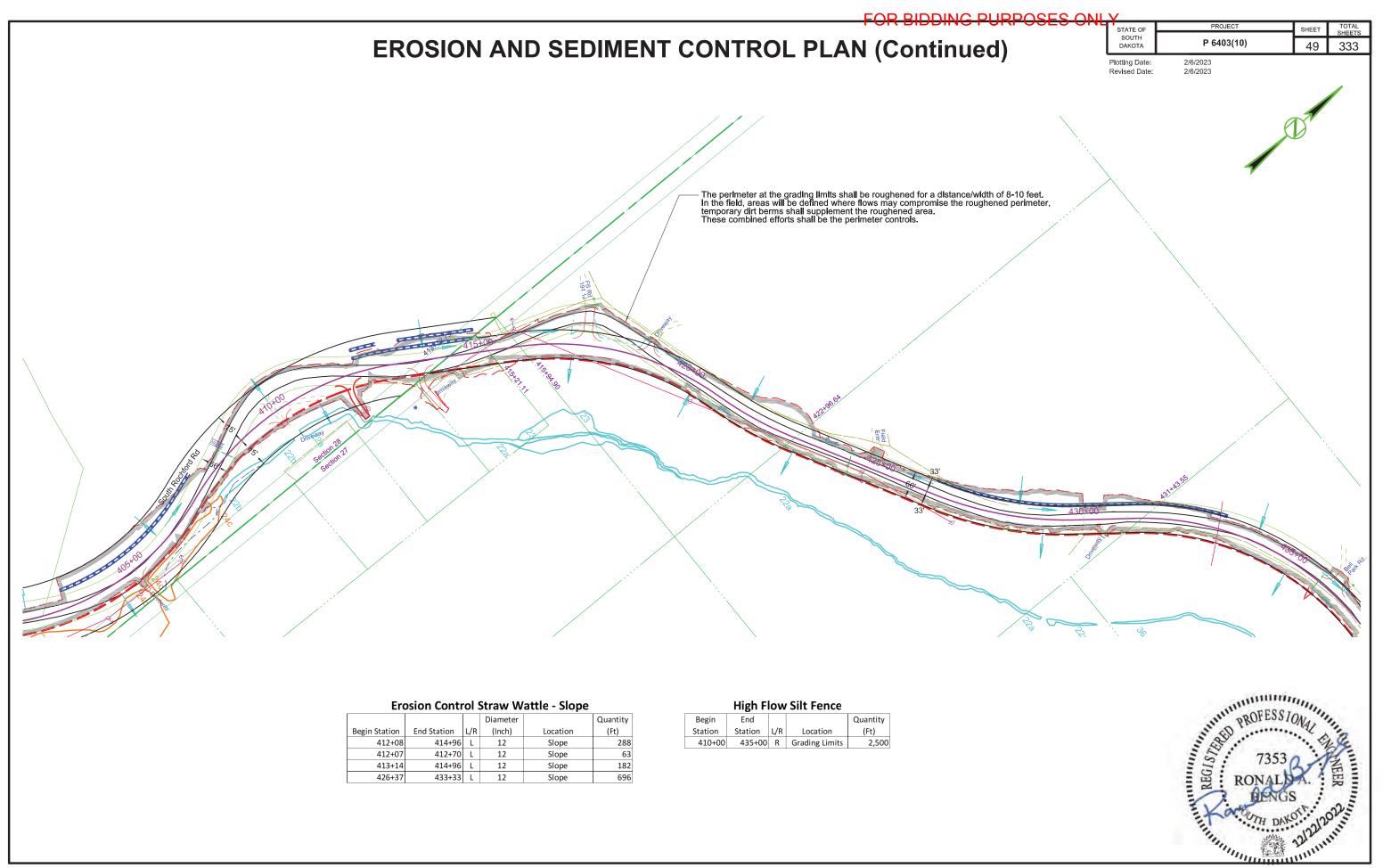
Begin	End				Quantity
Station	Station	L/R	Diameter (Inch)	Location	(Ft)
292+60	310+60	L	12	Ditch	1,080

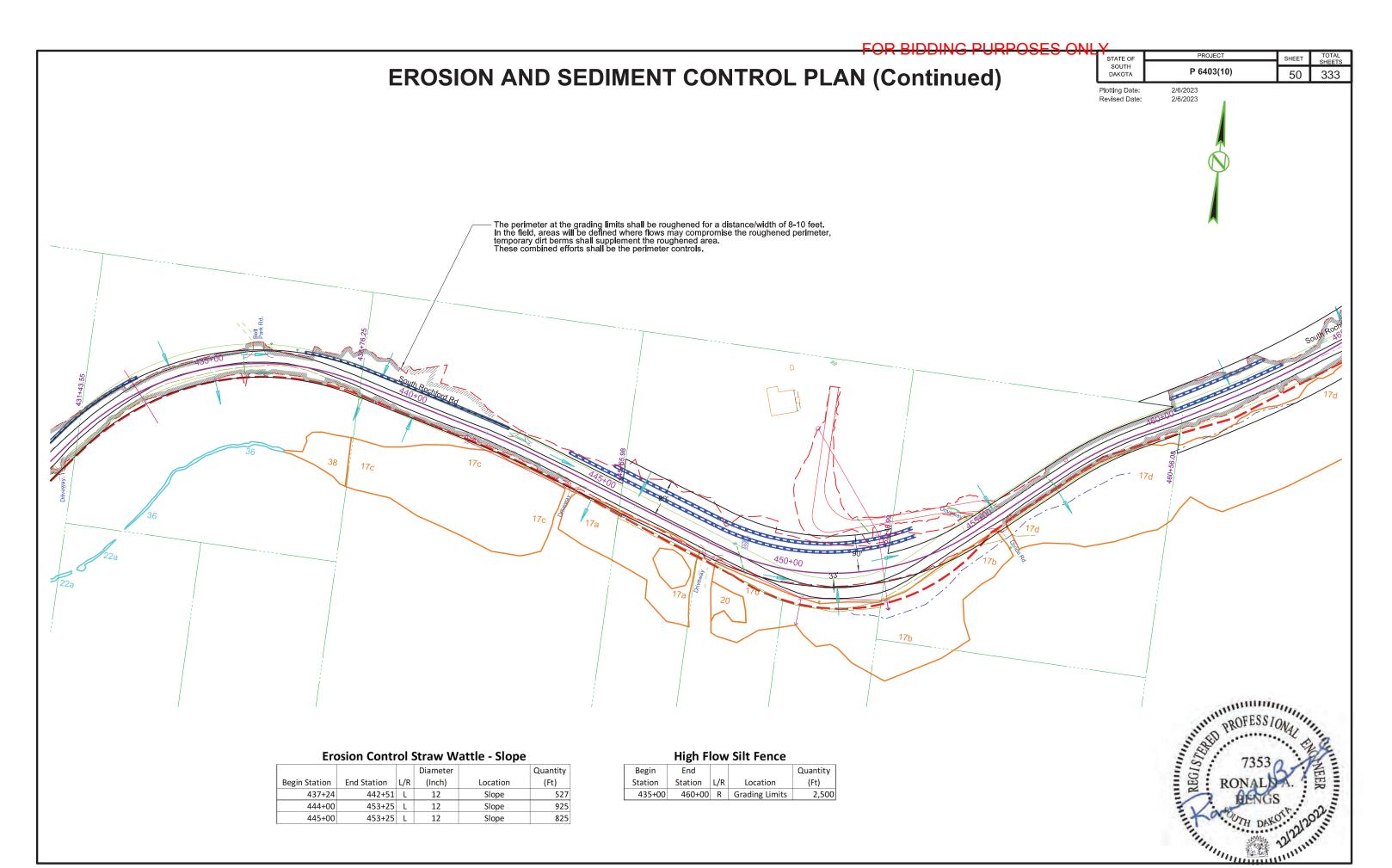


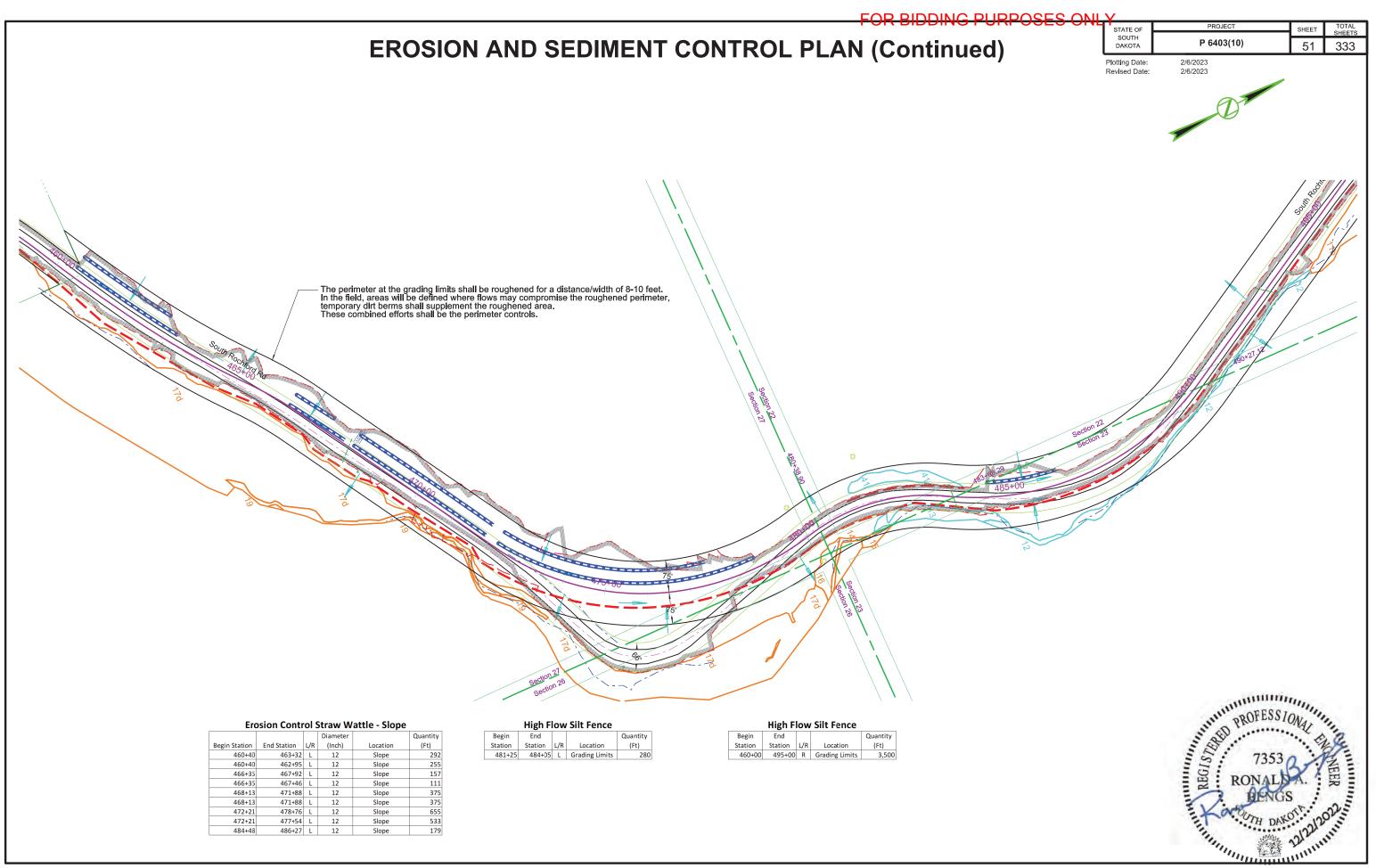












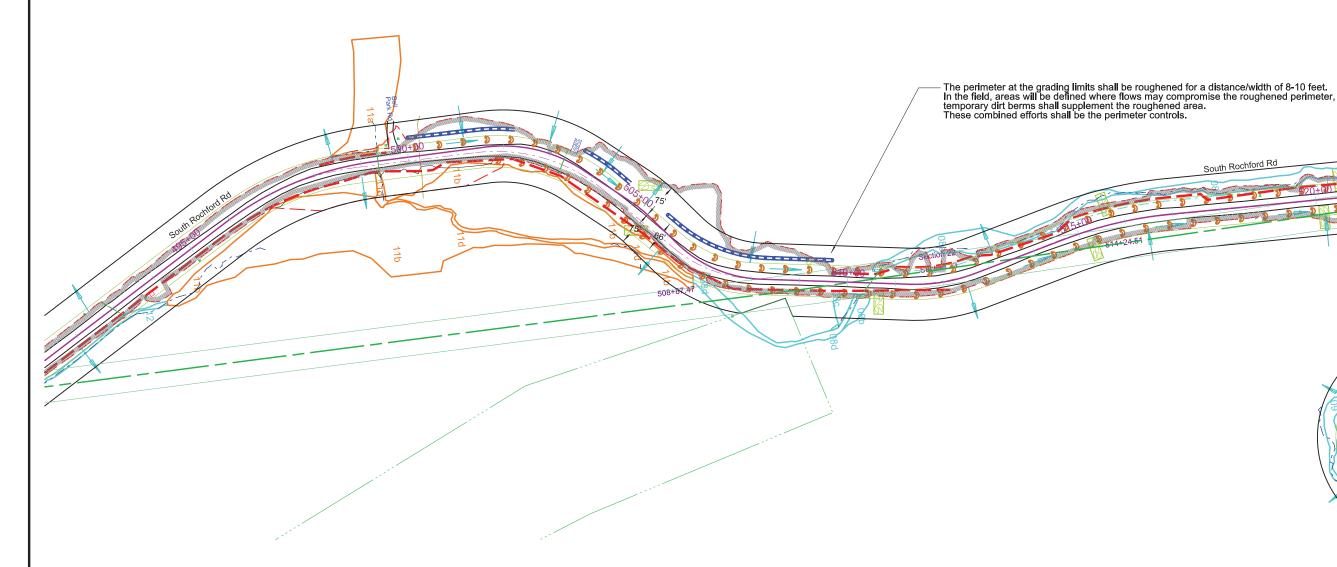
EROSION AND SEDIMENT CONTROL PLAN (Continued)

STATE OF SOUTH DAKOTA P 6403(10) SHEET TOTAL SHEETS

P 6403(10) 52 333

Plotting Date: Revised Date:





Erosion Control Straw Wattle - Slope

			Diameter		Quantity
Begin Station	End Station	L/R	(Inch)	Location	(Ft)
500+02	502+22	L	12	Slope	220
503+54	504+67	L	12	Slope	113
505+63	509+69	L	12	Slope	406
521+76	524+95	R	12	Slope	319
523+02	524+13	L	12	Slope	111

High Flow Silt Fence

Begin	End			Quantity
Station	Station	L/R	Location	(Ft)
498+10	499+55	L	Grading Limits	14

High Flow Silt Fence

Begin	End			Quantity
Station	Station	L/R	Location	(Ft)
495+00	511+66	R	Grading Limits	1,66

521+97.55

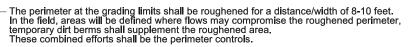
EROSION AND SEDIMENT CONTROL PLAN (Continued)

STATE OF SOUTH DAKOTA P 6403(10) 53 333

Plotting Date: Revised Date:

2/6/2023 2/6/2023







Rock Check Dams

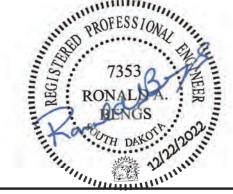
Begin	End			Quantity	Number				
Station	Station	L/R	Location	(CuYd)	of Checks				
500+00	535+00	L	Ditch	54	7				
501+45	535+00	R	Ditch	51	7				
	Dams spaced at 500 ft								

Erosion Control Straw Wattle - Slope

			Diameter		Quantity
Begin Station	End Station	L/R	(Inch)	Location	(Ft)
530+32	531+50	L	12	Slope	118

High Flow Silt Fence

Begin	End			Quantity
Station	Station	L/R	Location	(Ft)
510+00	523+30	L	Grading Limits	1,33



CONTROL DATA

 STATE OF SOUTH DAKOTA
 P 6403(10)
 SHEET SHEET SHEETS
 TOTAL SHEETS

 9 6403(10)
 54
 333

Plotting Date: 2/6/2023

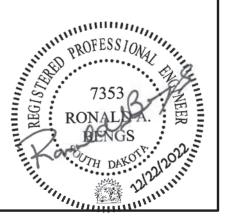
	H	ORIZO	NTAL AND VERTI	CAL CONTR	OL POINTS	
POINT	STATION	OFFSET	DESCRIPTION	NORTHING	EASTING	ELEVATION
				(US survey foot)	(US survey foot)	(US survey foot)
2017188	7+76.34	104.43	CP-SPIKE	639743.499	1050911.252	6165.15
162	32+06.65	-137.33	BM TT 4 RF	642277.262	1050994.568	6237.19
2017161	60+74.17	-201.08	CP *REBAR-YPC	642144.509	1053939.370	6155.71
200	87+68.42	-142.47	CP	643725.710	1055398.367	6157.16
2017159	126+42.07	-88.68	CP *REBAR	646642.318	1057383.027	6044.57
157	168+61.73	35.26	CP *REBAR	650821.555	1057730.429	6050.23
2017156	191+18.84	25.27	CP *REBAR	653074.677	1057888.853	6049.02
2017264	191+73.48	60.48	CP *REBAR	653129.135	1057924.344	6052.25
155	205+59.13	70.38	CP *REBAR	654512.525	1057950.968	6031.32
2017154	214+66.24	16.41	CP *SPIKE	655384.303	1057663.950	5999.91
2017153	224+25.24	36.89	CP *SPIKE	656260.830	1057273.185	5958.22
2017152	230+59.54	-74.92	CP	656861.192	1056993.632	5911.77
2017150	245+37.11	-24.25	CP	658031.619	1057876.937	5832.22
148	263+60.27	-61.94	CP *REBAR	657618.037	1059538.915	5828.65
2017192	280+38.29	27.71	CP	657786.925	1061086.852	5787.15
2017144	288+38.02	86.90	CP	657670.274	1061903.831	5797.58
2017143	296+39.07	83.88	CP	658382.330	1062367.620	5842.41
2017196	300+69.26	-26.48	CP	658782.261	1062150.036	5884.81
2017141	314+18.21	-33.40	CP	659858.831	1062509.091	5981.09
139	332+13.34	-50.68	CP *REBAR	661384.664	1063316.361	6093.99
2017138	337+05.64	-29.97	CP	661718.151	1063703.191	6077.40
2017198	342+54.80	-36.95	CP	662038.597	1064150.708	6041.75
2017199	352+57.62	-40.69	CP	662318.968	1065111.439	5960.89
135	361+95.47	-38.05	CP *REBAR	662507.645	1066012.816	5903.61
2017146	371+78.79	-9.69	CP	657983.004	1060261.598	5806.22
2017263	376+15.66	-63.08	CP	662385.687	1067384.441	5871.57

	HORIZONTAL AND VERTICAL CONTROL POINTS								
POINT	STATION	OFFSET	DESCRIPTION	NORTHING	EASTING	ELEVATION			
				(US survey foot)	(US survey foot)	(US survey foot)			
2017133	381+71.27	92.46	CP	662364.649	1067955.203	5847.97			
2017131	392+10.39	-34.42	BM TT 6 RF	662419.684	1068976.506	5822.25			
2017130	397+90.59	174.61	CP	662227.635	1069596.748	5797.90			
2017129	404+41.34	47.76	CP	662852.671	1069982.433	5779.16			
2017128	415+24.83	20.98	CP	663866.825	1070129.441	5767.75			
2017193	426+67.31	-33.10	CP	664471.130	1071061.487	5749.97			
2017125	448+28.27	40.59	CP	664596.029	1072979.617	5681.67			
2017124	452+20.47	64.13	CP	664556.254	1073396.146	5672.05			
2017123	464+52.84	-13.28	CP	665269.073	1074417.002	5645.23			
2017122	475+62.12	167.62	CP	665795.812	1075475.186	5603.36			
2017201	481+99.67	-22.45	CP	666505.184	1075380.929	5580.32			
2017260	483+56.61	106.17	CP *REBAR	666586.418	1075555.166	5573.51			
2017121	487+51.67	61.02	CP *REBAR	667021.930	1075625.442	5557.28			
2017249	496+92.96	20.34	CP *SPIKE	667885.936	1075218.685	5532.94			
2017002	499+54.34	56.93	CP *REBAR-RPC	668135.042	1075227.179	5530.45			
2017247	504+22.53	29.47	CP *SPIKE	668550.521	1075317.671	5515.85			
2017116	511+13.49	24.25	CP *REBAR	669143.543	1075639.734	5480.70			
2017115	515+27.23	20.95	CP *REBAR	669559.434	1075596.422	5465.20			
2017114	518+61.09	-18.34	CP *REBAR	669891.467	1075568.146	5445.95			
2017243	520+40.43	-80.70	CP *SPIKE	670075.336	1075520.759	5443.55			
2017240	528+74.74	25.83	CP *SPIKE	669989.970	1076106.007	5379.15			
2017239	532+23.86	16.77	CP *SPIKE	670141.068	1076433.580	5353.71			
109	536+24.73	-23.91	CP *REBAR	670444.175	1076705.051	5333.15			
102	South side of	Rochford	BM Q177	670393.160	1079355.848	5279.99			

The coordinates shown on this sheet are based on the South Dakota State Plane Coodinate System. South Zone (4002) (NAD83(2011)(Epoch: 2010.0000) The elevations shown on this sheet are based on NAVD88 (Geoid 12A);

Average Combined SF = 0.999665216

^{*} Point numbers beginning with "2017" are points that were occupied in 2017 with a fast-static session to confirm position. The other points are base control used for the static sessions from a survey in 2011



^{*} Points labeled "CP" only are either a #5 Rebar or 3/8" spike.

HORIZONTAL ALIGNMENT DATA

STATE OF SOUTH P 6403(10) 55 333

Plotting Date: 2/6/2023

			MAINLI	NE		
Type POB	Station 0+00.00				Northing 639183.780	Easting 1050336.120
		TL=834.21		N 39°35'38" E		
PC PI PT	6+34.21 8+51.32 10+53.29	R=650.00	Delta=36°56'28"		639672.492 639839.797 640056.679	1050740.322 1050878.698 1050888.747
r i	10+33.29	TL=1880.99		N 2°39'10" E	040030.073	1030868.747
PC PI	29+34.28 32+81.82	R=350.00	Delta=89°35'43"		641935.657 642282.822	1050975.806 1050991.891
PT	34+81.59	TL=2025.90		S 87°45'07" E	642269.188	1051339.161
PC PI	55+07.49 56+84.07	P-EE0 00	Delta=35°35'54"		642189.716 642182.789	1053363.500 1053539.942
PT	58+49.21	R=550.00	Della-33 33 34		642074.450	1053539.942
r i	30743.21	TL=72.68		S 52°09'12" E	042074.430	1033079.377
PC PI	59+21.89 60+76.28	R=500.00	Delta=34°19'10"		642029.855 641935.129	1053736.772 1053858.687
PT	62+21.39	TL=250.24		S 86°28'22" E	641925.630	1054012.785
PC PI	64+71.63 66+47.91	R=500.00	Delta=38°50'34"		641910.235 641899.389	1054262.549 1054438.503
PT	68+10.59	TL=294.01		N 54°41'04" E	642001.297	1054582.350
PC PI	71+04.60 72+20.85	R=250.00	Delta=49°52'45"		642171.255 642238.460	1054822.253 1054917.116
PT	73+22.24	TL=415.80		N 4°48'19" E	642354.308	1054926.854
PC PI	77+38.04 78+96.20	R=1770.00	Delta=10°12'44"		642768.650 642926.252	1054961.687 1054974.936
PT	80+53.52	TL=117.53		N 15°01'03" E	643079.008	1055015.916
PC PI	81+71.05 84+40.81	R=550.00	Delta=52°15'08"		643192.528 643453.069	1055046.371 1055116.268
PT	86+72.64	TL=713.47		N 67°16'11" E	643557.300	1055365.071
PC PI	93+86.11 96+81.80	R=835.00	Delta=39°00'02"		643832.979 643947.232	1056023.129 1056295.857
PT	99+54.48	TL=525.81		N 28°16'09" E	644207.659	1056435.902
PC PI	104+80.29 106+35.07	R=3200.00	Delta=5°32'17"		644670.759 644807.070	1056684.936 1056758.238
PT	107+89.60	TL=131.03		N 33°48'26" E	644935.672	1056844.352

PC PI	109+20.63 110+55.46	R=905.00	Delta=16°56'51"		645044.550 645156.581	1056917.260 1056992.279	
PT	111+88.32	TL=429.87		N 16°51'35" E	645285.614	1057031.383	
PC PI	116+18.19 117+44.69	R=600.00	Delta=23°48'40"		645697.011 645818.074	1057156.060 1057192.749	
PT	118+67.54	TL=32.03		N 40°40'15" E	645914.020	1057275.191	
PC PI	118+99.57 121+48.83	R=700.00	Delta=39°12'00"		645938.315 646127.369	1057296.066 1057458.511	
PT	123+78.49	TL=387.72		N 1°28'15" E	646376.546	1057464.909	
PC PI	127+66.21 127+73.64	R=10000.00	Delta=0°05'07"		646764.134 646771.569	1057474.861 1057475.052	
PT	127+81.08	TL=904.29		N 1°33'22" E	646779.003	1057475.254	
PC PI	136+85.37 137+20.63	R=10000.00	Delta=0°24'15"		647682.955 647718.205	1057499.810 1057500.768	
PT	137+55.89	TL=229.66		N 1°57′36″ E	647753.447	1057501.974	
PC PI	139+85.55 140+00.01	R=5000.00	Delta=0°19'53"		647982.968 647997.423	1057509.829 1057510.324	
PT	140+14.47	TL=259.80		N 2°17'30" E	648011.875	1057510.902	
PC PI	142+74.28 144+04.85	R=6000.00	Delta=2°29'36"		648271.472 648401.935	1057521.290 1057526.511	
PT	145+35.37	TL=607.19		N 4°47'06" E	648532.047	1057537.402	
PC PI	151+42.57 152+83.75	R=1000.00	Delta=16°04'19"		649137.125 649277.815	1057588.051 1057599.828	
PT	154+23.08	TL=157.33		N 11°17'14" W	649416.265	1057572.195	
PC PI	155+80.41 156+00.13	R=1500.00	Delta=1°30'23"		649570.553 649589.892	1057541.402 1057537.542	
PT	156+19.84	TL=65.30		N 9°46'51" W	649609.325	1057534.192	
PC PI	156+85.15 157+57.00	R=1250.00	Delta=6°34'48"		649673.677 649744.487	1057523.099 1057510.892	
PT	158+28.70	TL=23.52		N 3°12'03" W	649816.230	1057506.880	
PC Pl	158+52.21 159+77.93	R=900.00	Delta=15°54'15"		649839.708 649965.231	1057505.567 1057498.548	
PT	161+02.03	TL=272.44		N 12°42'12" E	650087.872	1057526.194	

The coordinates shown on this sheet are based on the South Dakota State Plane Coodinate System. South Zone (4002) (NAD83(2011)(Epoch: 2010.0000). The elevations shown on this sheet are based on NAVD88 (Geoid 12A); Average Combined SF = 0.999665216

244+47.44

HORIZONTAL ALIGNMENT DATA

STATE OF SHEET SOUTH P 6403(10) 56 333

1057852.174

657937.335

2/6/2023

Plotting Date:

MAINLINE (Continued)

PC PI	163+74.48 164+00.65	R=5000.00	Delta=0°36'00"		650353.648 650379.182	1057586.105 1057591.861
PT	164+26.83	TL=320.77		N 13°18'11" E	650404.654	1057597.884
PC PI	167+47.59 168+94.31	R=3000.00	Delta=5°35'59"		650716.812 650859.595	1057671.693 1057705.454
PT	170+40.80	TL=769.34		N 7°42'12" E	651004.990	1057725.120
PC PI	178+10.13 179+72.01	R=3000.00	Delta=6°10'37"		651767.384 651927.795	1057828.246 1057849.944
PT	181+33.56	TL=153.52		N 1°31'35" E	652089.609	1057854.255
PC PI	182+87.08 183+29.97	R=4000.00	Delta=1°13'43"		652243.076 652285.949	1057858.345 1057859.487
PT	183+72.86	TL=958.87		N 0°17'52" E	652328.836	1057859.710
PC Pl	193+31.73 193+61.43	R=20000.00	Delta=0°10'13"		653287.696 653317.397	1057864.692 1057864.846
PT	193+91.13	TL=873.04		N 0°28'04" E	653347.098	1057865.089
PC Pl	202+64.18 203+29.57	R=5000.00	Delta=1°29'56"		654220.111 654285.508	1057872.218 1057872.752
PT	203+94.97	TL=258.69		N 1°58'00" E	654350.869	1057874.997
PC PI	206+53.66 208+88.82	R=1000.00	Delta=26°27'58"		654609.410 654844.427	1057883.874 1057891.944
PT	211+15.58	TL=710.68		N 24°29'58" W	655058.411	1057794.429
PC PI	218+26.26 218+36.94	R=1000.00	Delta=1°13'27"		655705.105 655714.826	1057499.722 1057495.291
PT	218+47.63	TL=964.39		N 25°43'25" W	655724.451	1057490.654
PC PI	228+12.02 230+47.02	R=300.00	Delta=76°08'48"		656593.269 656804.984	1057072.079 1056970.080
PT	232+10.72	TL=255.24		N 50°25'23" E	656954.709	1057151.214
PC PI	234+65.96 235+42.78	R=835.00	Delta=10°30'44"		657117.328 657166.268	1057347.947 1057407.154
PT	236+19.16	TL=286.37		N 39°54'39" E	657225.189	1057456.439
PC PI	239+05.53 240+90.87	R=835.00	Delta=25°01'48"		657444.845 657587.013	1057640.171 1057759.087
PT	242+70.30	TL=177.13		N 14°52'51" E	657766.142	1057806.685

PI	246+13.31	R=186.00	Delta=83°27'04"		658097.641	1057852.174
PT	247+18.35	TL=45.37		S 81°40'05" E	658073.606	1058058.889
PC	247+63.72				658067.031	1058103.781
PI	249+21.64	R=1004.00	Delta=17°52'42"		658044.147	1058260.040
PT	250+77.00	TL=36.11		S 63°47'23" E	657974.396	1058401.728
PC PI	251+13.11 252+07.28	R=1309.50	Delta=8°13'34"		657958.447 657916.857	1058434.126 1058518.610
PT	253+01.12	TL=213.91		S 55°33'49" E	657863.607	1058596.274
PC	255+15.03				657742.645	1058772.695
PI	256+90.36	R=1504.00	Delta=13°17'54"		657643.499	1058917.296
PT	258+64.11	TL=65.42		S 68°51'43" E	657580.273	1059080.826
		11-05.42		3 00 31 43 E		
PC	259+29.53	5 560 00	D I. 5504710411		657556.681	1059141.848
PI	262+30.73	R=569.00	Delta=55°47'21"		657448.063	1059422.783
PT	264+83.57	TL=559.54		N 55°20'56" E	657619.319	1059670.560
PC	270+43.11				657937.461	1060130.853
PI	272+83.25	R=480.00	Delta=53°09'24"		658073.999	1060328.399
PT	274+88.43	TL=444.64		S 71°29'40" E	657997.780	1060556.121
DC	270+22-09				CE70EC CE1	1000077 774
PC Pl	279+33.08 279+82.06	R=2000.00	Delta=2°48'21"		657856.651 657841.105	1060977.774 1061024.221
PT	280+31.02	TL=235.04		S 68°41'19" E	657823.304	1061069.853
PC	282+66.06				657737.882	1061288.818
PI	283+91.23	R=800.00	Delta=17°47'07"		657692.391	1061405.429
284+96.04 bi	c = 284+74.25 ah 284+92.59				657686.030 657684.692	1061512.070 1061530.363
PI	286+83.07	R=692.000	Delta=30°46'46"		657672.977	1061730.478
PT	288+64.34	TL=1.409		N 62°44'48" E	657760.201	1061889.809
PC	288+65.75				657760.846	1061891.062
PI	295+97.41	R=725.000	Delta=90°31'26"		658095.892	1062541.5
PT	300+11.20	TL=397.468		N 27°46'38" W	658743.239	1062200.521
PC	304+08.67				659094.905	1062015.287
PI	307+31.83	R=350.000	Delta=85°26'00"		659380.825	1061864.684
PT	309+30.55	TI _ 275 726		N E7°20'22" F	659553.715	1062137.705
		TL=275.726		N 57°39'22" E		

The coordinates shown on this sheet are based on the South Dakota State Plane Coodinate System. South Zone (4002) (NAD83(2011)(Epoch: 2010.0000).

The elevations shown on this sheet are based on NAVD88 (Geoid 12A);

Average Combined SF = 0.999665216

STATE OF SOUTH DAKOTA

P 6403(10)

2/6/2023

Plotting Date:

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HORIZONTAL ALIGNMENT DATA

MAINLINE	(Continued)
	(

PC PI	312+06.28 313+75.66	R=835.000	Delta=22°56'00"		659701.229 659791.846	1062370.653 1062513.75
PT	315+40.50	TL=538.582		N 34°43'22" E	659931.059	1062610.228
PC PI	320+79.08 321+36.62	R=835.000	Delta=7°53'05"		660373.728 660421.026	1062917.008 1062949.786
PT	321+93.99	TL=266.374		N 42°36'28" E	660463.38	1062988.743
PC PI	324+60.36 327+20.00	R=650.000	Delta=43°32'53"		660659.433 660850.529	1063169.071 1063344.841
PT	329+54.40	TL=120.431		N 0°56'25" W	661110.134	1063340.58
PC PI	330+74.83 332+55.86	R=350.000	Delta=54°41'51"		661230.55 661411.551	1063338.603 1063335.632
PT	334+08.96	TL=478.259		N 53°45'26" E	661518.575	1063481.633
PC PI	338+87.22 339+27.14	R=2000.000	Delta=2°17'14"		661801.326 661824.93	1063867.358 1063899.558
PT	339+67.06	TL=490.015		N 56°02'40" E	661847.23	1063932.674
PC PI	344+57.07 346+35.19	R=835.000	Delta=24°04'58"		662120.928 662220.414	1064339.127 1064486.868
PCC PI	348+08.04 350+11.68	R=2000.000	Delta=11°37'38"		662250.954 662285.87	1064662.346 1064862.964
PT	352+13.91	TL=188.017		S 88°14'44" E	662279.635	1065066.502
PC PI	354+01.93 355+63.31	R=835.000	Delta=21°52'39"		662273.879 662268.938	1065254.432 1065415.738
PT	357+20.76	TL=298.667		N 69°52'37" E	662324.459	1065567.269
PC PI	360+19.43 362+53.56	R=835.000	Delta=31°19'36"		662427.212 662507.763	1065847.704 1066067.543
PT	364+75.97	TL=888.375		S 78°47'47" E	662462.272	1066297.213
PC PI	373+64.34 374+81.03	R=350.000	Delta=36°52'36"		662289.664 662266.992	1067168.658 1067283.124
PT	375+89.61	TL=170.096		N 64°19'37" E	662317.546	1067388.293
PC PI	377+59.71 378+30.58	R=405.000	Delta=19°51'11"		662391.237 662421.943	1067541.598 1067605.478
PT	379+00.04	TL=713.663		N 84°10'48" E	662429.13	1067675.989



PC PI	386+13.70 387+79.14	R=800.000	Delta=23°22'03"		662501.497 662518.273	1068385.974 1068550.557	
PT	389+39.97	TL=311.681		S 72°27'09" E	662468.395	1068708.294	
PC PI	392+51.65 395+36.18	R=800.000	Delta=39°09'23"		662374.424 662288.641	1069005.471 1069276.755	
PT	397+98.38	TL=46.040		N 68°23'28" E	662393.422	1069541.283	
PC PI	398+44.42 399+78.36	R=550.000	Delta=27°22'21"		662410.376 662459.701	1069584.087 1069708.61	
PCC PI	401+07.18 404+42.55	R=593.000	Delta=58°58'52"		662560.755 662813.794	1069796.512 1070016.62	
PRC PI	407+17.62 410+33.03	R=653.000	Delta=51°33'45"		663132.821 663432.857	1069913.193 1069815.922	
PT	413+05.28	TL=284.536		N 33°36'00" E	663695.568	1069990.467	
PC PI	415+89.82 418+43.17	R=650.000	Delta=42°35'24"		663932.564 664143.592	1070147.927 1070288.133	
PT	420+72.98	TL=32.294		N 76°11'24" E	664204.069	1070534.168	
PC PI	421+05.28 421+95.47	R=835.000	Delta=12°19'49"		664211.778 664233.308	1070565.528 1070653.117	
PT	422+84.97	TL=353.297		N 63°51'35" E	664273.046	1070734.087	
PC PI	426+38.27 428+13.69	R=835.000	Delta=23°43'47"		664428.699 664505.987	1071051.248 1071208.731	
PT	429+84.09	TL=134.183		N 40°07'48" E	664640.115	1071321.798	
PC PI	431+18.27 435+40.73	R=650.000	Delta=66°02'31"		664742.71 665065.711	1071408.282 1071680.564	
PT	438+67.50	TL=308.698		S 73°49'41" E	664948.048	1072086.302	
PC PI	441+76.20 442+62.82	R=2000.000	Delta=4°57'36"		664862.069 664837.943	1072382.785 1072465.977	
PT	443+49.33	TL=467.240		S 68°52'05" E	664806.715	1072546.772	
PC PI	448+16.57 451+85.94	R=635.000	Delta=60°22'19"		664638.267 664505.103	1072982.592 1073327.123	
PT	454+85.66	TL=248.303		N 50°45'36" E	664738.755	1073613.201	
PC PI	457+33.96 458+36.33	R=835.000	Delta=13°58'45"		664895.823 664960.58	1073805.513 1073884.8	
PRC PI	459+37.69 460+88.10	R=4000.000	Delta=4°18'25"		665004.266 665068.453	1073977.383 1074113.412	
PCC PI	462+38.37 463+81.03	R=2000.000	Delta=8°09'36"		665142.674 665213.069	1074244.237 1074368.318	
PT	465+23.21	TL=2.060		N 52°16'21" E	665300.362	1074481.151	

The coordinates shown on this sheet are based on the South Dakota State Plane Coodinate System. South Zone (4002) (NAD83(2011)(Epoch: 2010.0000). The elevations shown on this sheet are based on NAVD88 (Geoid 12A); Average Combined SF = 0.999665216

Delta=3°42'33"

Delta=20°48'26"

Delta=8°44'17"

Delta=14°40'59"

R=2000.000

TL=0.004

R=350.000

TL=206.872

R=309.000

TL=114.021

R=635.000

TL=78.007

	ALIGNMENT DATA	
JURIZUNTAL	ALIGNWENT DATA	

PC

PT

PC

Ы

PT

PC

PT

PC

PT

POE

530+43.74

531+08.50

531+73.22

531+73.22

532+37.48

533+00.33

535+07.20

535+30.81

535+54.32

536+68.35

537+50.16

538+31.07

539+09.08

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1076264.57

1076321.426

1076380.169

1076380.173

1076438.462

1076483.342

1076627.822

1076644.31

1076663.173

1076754.279

1076819.65

1076870.417

1076918.822

Plotting 023

ting Date:	2/6/202

N 65°06'23" E

N 44°17'57" E

N 53°02'14" E

N 38°21'15" E

670072.281

670103.284

670130.543

670130.545

670157.594

670203.585

670351.644

670368.54

670382.735

670451.296

670500.49

670564.647

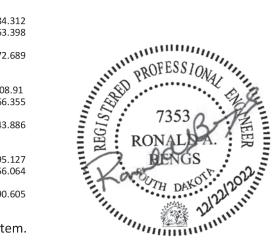
670625.819

MAINLINE (Continued

PC PI	465+25.27 465+98.13	R=900.000	Delta=9°15'27"		665301.623 665346.21	1074482.78 1074540.412
PT	466+70.68	TL=555.450		N 61°31'47" E	665380.945	1074604.465
PC PI	472+26.13 474+63.92	R=600.000	Delta=43°14'15"		665645.729 665759.081	1075092.743 1075301.771
PCC PI	476+78.92 478+47.14	R=650.000	Delta=29°01'15"		665984.85 666144.576	1075376.403 1075429.203
PRC PI	480+08.15 481+87.71	R=450.000	Delta=43°30'28"		666309.863 666486.291	1075397.887 1075364.459
PRC PI	483+49.85 487+24.15	R=650.000	Delta=59°52'11"		666637.264 666951.956	1075461.677 1075664.32
PT	490+29.05	TL=558.551		N 27°05'26" W	667285.184	1075493.867
PC PI	495+87.61 497+61.08	R=650.000	Delta=29°53'08"		667782.456 667936.896	1075239.504 1075160.505
PT	499+26.65	TL=233.558		N 2°47'42" E	668110.162	1075168.964
PC PI	501+60.21 502+72.11	R=300.000	Delta=40°54'38"		668343.442 668455.207	1075180.353 1075185.81
PT	503+74.41	TL=117.632		N 43°42'20" E	668536.099	1075263.126
PC PI	504+92.05 505+20.31	R=300.000	Delta=10°45'57"		668621.135 668641.569	1075344.404 1075363.936
PT	505+48.41	TL=76.389		N 54°28'17" E	668657.996	1075386.941
PC PI	506+24.80 507+26.76	R=264.000	Delta=42°14'00"		668702.387 668761.635	1075449.109 1075532.085
PT	508+19.40	TL=310.363		N 12°14'17" E	668861.276	1075553.697
PC PI	511+29.77 512+00.70	R=350.000	Delta=22°54'54"		669164.586 669233.912	1075619.485 1075634.522
PT	512+69.74	TL=200.073		N 10°40'37" W	669303.621	1075621.38
PC PI	514+69.82 515+82.70	R=835.000	Delta=15°23'53"		669500.231 669611.16	1075584.312 1075563.398
PT	516+94.22	TL=440.076		N 4°43'16" E	669723.66	1075572.689
PC PI	521+34.30 528+32.23	R=175.000	Delta=151°50'51"		670162.242 670857.803	1075608.91 1075666.355
PT	525+98.09	TL=128.860		S 23°25'53" E	670217.427	1075943.886
PC PI	527+26.95 528+80.19	R=140.000	Delta=95°10'17"		670099.193 669958.587	1075995.127 1076056.064
PT	529+59.50	TL=84.246		N 61°23'50" E	670031.949	1076190.605

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Average Combined SF = 0.999665216



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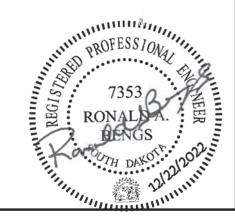
TABLE OF SUPERELEVATION

			TABLE O	F SUI	PERELEVATION
Curve	Station	to	Station		
	0+00				РОВ
	0+00		5+35.61		Normal Crown Section
	5+35.61		6+51.61		Superelevation Transition
1	6+51.61		10+35.89		650' Radius Curve Left
					0.0600'/' Superelevation Rate
					Point of Rotation at Centerline
	10+35.89		11+51.89		Superelevation Transition
	11+51.89		28+35.68		Normal Crown Section
	28+35.68		29+51.68		Superelevation Transition
2	29+51.68		34+64.19		350' Radius Curve Right
					0.0600'/' Superelevation Rate
					Point of Rotation at Centerline
	34+64.19		35+80.19		Superelevation Transition
	35+80.19		54+08.89		Normal Crown Section
	54+08.89		55+24.89		Superelevation Transition
3	55+24.89		57+69.31		550' Radius Curve Right
					0.0600'/' Superelevation Rate
					Point of Rotation at Centerline
	57+69.31		58+85.31	LT	Superelevation Transition
	58+85.31		58+85.79	RT	Normal Crown Section
	58+85.79		60+01.79		Superelevation Transition
4	60+01.79		62+03.99		500' Radius Curve Left
					0.0600'/' Superelevation Rate
					Point of Rotation at Centerline
	62+03.99		63+19.99		Superelevation Transition
	63+19.99		63+73.03		Normal Crown Section
	63+73.03		64+89.03		Superelevation Transition
5	64+89.03		67+93.19		500' Radius Curve Left
					0.0600'/' Superelevation Rate
					Point of Rotation at Centerline
	67+93.19		69+09.19	RT	Superelevation Transition
	69+09.19		70+06.00	RT	Normal Crown Section
	70+06.00		71+22.00		Superelevation Transition
6	71+22.00		73+04.84		250' Radius Curve Left
					0.0600'/' Superelevation Rate
					Point of Rotation at Centerline
	73+04.84		74+20.84	RT	Superelevation Transition
	74+20.84		76+53.29	LT	Normal Crown Section
	76+53.29		77+51.98		Superelevation Transition

7	77+51.98	80+06.58		1770' Radius Curve Right
				0.0481'/' Superelevation Rate
				Point of Rotation at Centerline
	80+06.58	81+05.27	LT	Superelevation Transition
	81+05.27	81+05.45		Normal Crown Section
	81+05.45	82+21.45		Superelevation Transition
8	82+21.45	86+55.24		550' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	86+55.24	87+71.24	LT	Superelevation Transition
	87+71.24	92+87.51	RT	Normal Crown Section
	92+87.51	94+03.51		Superelevation Transition
9	94+03.51	99+37.08		835' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	99+37.08	100+53.08	RT	Superelevation Transition
	100+53.08	104+09.17	LT	Normal Crown Section
	104+09.17	104+90.83		Superelevation Transition
10	104+90.83	107+59.56		3200' Radius Curve Right
				0.0363'/' Superelevation Rate
				Point of Rotation at Centerline
	107+59.56	108+41.22	LT	Superelevation Transition
	108+41.22	108+41.82	RT	Normal Crown Section
	108+41.82	109+57.46		Superelevation Transition
11	109+57.46	111+70.99		905' Radius Curve Left
				0.0597'/' Superelevation Rate
				Point of Rotation at Centerline
	111+70.99	112+86.63	RT	Superelevation Transition
	112+86.63	115+19.59	LT	Normal Crown Section
	115+19.59	116+35.59		Superelevation Transition
12	116+35.59	117+67.14		600' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	117+67.14	118+83.14	LT	Superelevation Transition
	118+83.14	118+83.97	RT	Normal Crown Section
	118+83.97	119+99.97		Superelevation Transition
13	119+99.97	123+61.09		700' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	123+61.09	124+77.09	RT	Superelevation Transition
	124+77.09	127+19.71	LT	Normal Crown Section
	127+19.71	127+70.58		Superelevation Transition

14	127+70.58	127+76.71		10,000' Radius Curve Right
				0.0151'/' Superelevation Rate
				Point of Rotation at Centerline
	127+76.71	128+27.58	LT	Superelevation Transition
	128+27.58	136+38.87	LT	Normal Crown Section
	136+38.87	136+89.74		Superelevation Transition
15	136+89.74	137+51.52		10,000' Radius Curve Right
				0.0151'/' Superelevation Rate
				Point of Rotation at Centerline
	137+51.52	138+02.39	LT	Superelevation Transition
	138+02.39	139+25.33	LT	Normal Crown Section
	139+25.33	139+93.35		Superelevation Transition
16	139+93.35	140+06.67		5000' Radius Curve Right
				0.0269'/' Superelevation Rate
				Point of Rotation at Centerline
	140+06.67	140+74.69		Superelevation Transition
	140+74.69	142+18.22		Normal Crown Section
	142+18.22	142+81.04		Superelevation Transition
17	142+81.04	145+28.61		6,000' Radius Curve Right
				0.0233'/' Superelevation Rate
				Point of Rotation at Centerline
	145+28.61	145+91.44	LT	Superelevation Transition
	145+91.44	150+45.31	RT	Normal Crown Section
	150+45.31	151+59.63		Superelevation Transition
18	151+59.63	153+91.51		1,000' Radius Curve Left
				0.0588'/' Superelevation Rate
				Point of Rotation at Centerline
	153+91.51	155+05.83		Superelevation Transition_
	155+05.83	155+06.06		Normal Crown Section
	155+06.06	155+78.13		Superelevation Transition

Plotting Date:



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19	155+78.13	155+78.13		1,500' Radius Curve Right
				0.0516'/' Superelevation Rate
				Point of Rotation at Centerline
	155+78.13	156+50.19	LT	Superelevation Transition
	156+50.19	156+50.51	LT	Normal Crown Section
	156+50.51	157+44.17		Superelevation Transition
20	157+44.17	157+44.17		1,250' Radius Curve Right
				0.0553'/' Superelevation Rate
				Point of Rotation at Centerline
	157+44.17	158+37.84	LT	Superelevation Transition
	158+37.84	158+37.86	LT	Normal Crown Section
	158+37.86	159+53.55		Superelevation Transition
21	159+53.55	160+84.70		900' Radius Curve Right
				0.0598'/' Superelevation Rate
				Point of Rotation at Centerline
	160+84.70	162+00.38	LT	Superelevation Transition
	162+00.38	163+14.26	LT	Normal Crown Section
	163+14.26	163+82.28		Superelevation Transition
22	163+82.28	164+19.02		5,000' Radius Curve Right
				0.0269'/' Superelevation Rate
				Point of Rotation at Centerline
	164+19.02	164+87.04	LT	Superelevation Transition
	164+87.04	166+74.97	RT	Normal Crown Section
	166+74.97	167+58.50		Superelevation Transition
23	167+58.50	170+29.89		3,000' Radius Curve Left
				0.0376'/' Superelevation Rate
				Point of Rotation at Centerline
	170+29.89	171+13.42	RT	Superelevation Transition
	171+13.42	177+37.51	RT	Normal Crown Section
	177+37.51	178+21.04		Superelevation Transition
24	178+21.04	181+22.66		3000' Radius Curve Left
				0.0376'/' Superelevation Rate
				Point of Rotation at Centerline
	181+22.66	182+06.19	RT	Superelevation Transition
	182+06.19	182+21.43	RT	Normal Crown Section
	182+21.43	182+96.25		Superelevation Transition
25	182+96.25	183+63.69		4000' Radius Curve Left
				0.0316'/' Superelevation Rate
				Point of Rotation at Centerline
	183+63.69	184+38.51	RT	Superelevation Transition
	184+38.51	192+93.51	LT	Normal Crown Section
	192+93.51	193+34.04		Superelevation Transition

26	193+34.04	193+88.83		20000' Radius Curve Right
				0.0790'/' Superelevation Rate
				Point of Rotation at Centerline
	193+88.83	194+29.36	LT	Superelevation Transition
	194+29.36	202+03.96	LT	Normal Crown Section
	202+03.96	202+71.98		Superelevation Transition
27	202+71.98	203+87.16		5,000' Radius Curve Right
				0.0269'/' Superelevation Rate
				Point of Rotation at Centerline
	203+87.16	204+55.18	LT	Superelevation Transition
	204+55.18	205+56.40	RT	Normal Crown Section
	205+56.40	206+70.72		Superelevation Transition
28	206+70.72	210+98.52		1000' Radius Curve Left
				0.0588'/' Superelevation Rate
				Point of Rotation at Centerline
	210+98.52	212+12.84	RT	Superelevation Transition
	212+12.84	217+29.00	RT	Normal Crown Section
	217+29.00	218+36.94		Superelevation Transition
29	218+37.00	218+37.00		1000' Radius Curve Left
				0.0588'/' Superelevation Rate
				Point of Rotation at Centerline
	218+37.00	219+44.88	RT	Superelevation Transition
	219+44.88	227+13.42	LT	Normal Crown Section
	227+13.42	228+29.42		Superelevation Transition
30	228+29.42	231+93.32		300' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	231+93.32	233+09.32	LT	Superelevation Transition
	233+09.32	233+67.36	RT	Normal Crown Section
	233+67.36	234+83.36		Superelevation Transition
31	234+83.36	236+01.76		835' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	236+01.76	237+17.76	RT	Superelevation Transition
	237+17.76	238+06.93	RT	Normal Crown Section
	238+06.93	239+22.93		Superelevation Transition
32	239+22.93	242+42.40		835' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	242+42.40	243+58.40	RT	Superelevation Transition
	243+58.40	243+59.34	LT	Normal Crown Section
	243+59.34	244+75.34		Superelevation Transition

33	244+75.34	246+25.45		186' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	246+25.45	247+41.45	LT	Superelevation Transition
	247+41.45	247+42.01	LT	Normal Crown Section
	247+42.01	248+56.27		Superelevation Transition
34	248+56.27	249+82.95		1,004' Radius Curve Right
				0.0588'/' Superelevation Rate
				Point of Rotation at Centerline
	249+82.95	250+97.21	LT	Superelevation Transition
	250+97.21	250+98.02	LT	Normal Crown Section
	250+98.02	252+05.89		Superelevation Transition
35	252+05.89	252+85.35		1,309' Radius Curve Right
				0.0544'/' Superelevation Rate
				Point of Rotation at Centerline
	252+85.35	253+93.22	LT	Superelevation Transition
	253+93.22	254+26.24	RT	Normal Crown Section
	254+26.24	255+29.98		Superelevation Transition
36	255+29.98	257+88.16		1,504' Radius Curve Left
				0.0515'/' Superelevation Rate
				Point of Rotation at Centerline
	257+88.16	258+91.89	RT	Superelevation Transition
	258+91.89	258+91.93	RT	Normal Crown Section
	258+91.93	260+07.93		Superelevation Transition
37	260+07.93	264+66.17		569' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	264+66.17	265+82.17	RT	Superelevation Transition
	265+82.17	269+44.51	LT	Normal Crown Section
	269+44.51	270+60.51		Superelevation Transition

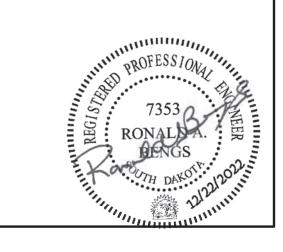


TABLE OF SUPERELEVATION (Continued)

STATE OF SOUTH DAKOTA P 6403(10) SHEET TOTAL SHEETS

61 333

Plotting Date: 2/6/2023

38	270+60.51	274+71.03		480' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	274+71.03	275+87.03	LT	Superelevation Transition
	275+87.03	278+51.32	LT	Normal Crown Section
	278+51.32	279+46.27		Superelevation Transition
39	279+46.27	280+17.83		2000' Radius Curve Right
				0.0455'/' Superelevation Rate
				Point of Rotation at Centerline
	280+17.83	281+12.77	LT	Superelevation Transition
	281+12.77	281+68.63	RT	Normal Crown Section
	281+68.63	282+83.16		Superelevation Transition
40	282+83.16	284+92.59		800' Radius Curve Left
				0.0590'/' Superelevation Rate
				Point of Rotation at Centerline
	Equation 284+	96.04 Back = 2	84+74.	
PCC	284+92.59			
41	284+92.59	288+86.13		692' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	288+86.13	288+64.92		Superelevation Transition
	288+64.92	288+65.15		Normal Crown Section
	288+65.15	289+04.94		Superelevation Transition
42	289+04.94	300+15.59		725' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	300+15.59	301+31.59	RT	Superelevation Transition
	301+31.59	303+31.86	LT	Normal Crown Section
	303+31.86	304+47+86		Superelevation Transition
43	304+47+86	309+34.94		350' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	309+34.94	310+50.94	LT	Superelevation Transition
	310+50.94	311+29.47	RT	Normal Crown Section
	311+29.47	312+45.47		Superelevation Transition
44	312+45.47	315+44.89		835' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	315+44.89	316+60.89	RT	Superelevation Transition
	316+60.89	320+02.27	LT	Normal Crown Section
	320+02.27	321+18.27		Superelevation Transition
45	321+18.27	321+98.38		835' Radius Curve Right

				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	321+98.38	323+14.38	LT	Superelevation Transition
	323+14.38	323+83.55	RT	Normal Crown Section
	323+83.55	324+99.55		Superelevation Transition
46	324+99.55	329+20.29		650' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	329+20.29	330+36.29	RT	Superelevation Transition
	330+36.29	330+36.29	RT	Normal Crown Section
	330+36.29	331+52.52		Superelevation Transition
47	331+52.52	334+13.35		350' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	334+13.35	335+29.35	LT	Superelevation Transition
	335+29.35	338+27.26	LT	Normal Crown Section
	338+27.26	339+22.20		Superelevation Transition
48	339+22.20	339+75.66		2,000' Radius Curve Right
				0.0455'/' Superelevation Rate
				Point of Rotation at Centerline
	339+75.66	340+70.60	LT	Superelevation Transition
	340+70.60	343+80.26	LT	Normal Crown Section
	343+80.26	344+96.26		Superelevation Transition
49	344+96.26	348+08.04		835' Radius Curve Right
				0.0600'/' Superelevation Rate
PCC	348+08.04			
50	348+08.04	352+35.70		2000' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	352+35.70	352+95.66		Superelevation Transition
	352+95.66	353+03.33		Normal Crown Section
	353+03.33	354+41.12		Superelevation Transition
51	354+41.12	357+25.15		835' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	357+25.15	358+19.36		Superelevation Transition
	358+19.36	359+20.83		Normal Crown Section
	359+20.83	360+58.62		Superelevation Transition
52	360+58.62	364+80.36		835' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline

	365+38.36	372+87.53	RT	Normal Crown Section
	372+87.53	374+03.53		Superelevation Transition
53	374+03.53	375+80.00		350' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	375+80.00	376+96.00	RT	Superelevation Transition
	376+96.00	376+96.90	LT	Normal Crown Section
	376+96.90	378+12.90		Superelevation Transition
54	378+12.90	379+04.43		405' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	379+04.43	380+20.43	LT	Superelevation Transition
	380+20.43	385+36.89	LT	Normal Crown Section
	385+36.89	386+52.89		Superelevation Transition
55	386+52.89	389+44.36		800' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	389+44.36	390+60.36	LT	Superelevation Transition
	390+60.36	391+74.84	RT	Normal Crown Section
	391+74.84	392+90.84		Superelevation Transition
56	392+90.84	397+26.77		800' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	397+26.77	398+42.77	RT	Superelevation Transition



TABLE OF SUPERELEVATION (Continued)

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	P 6403(10)	62	333

lotting Date: 2/6/2023

	398+42.77	398+43.65	RT	Normal Crown Section
	398+43.65	399+59.60		Superelevation Transition
57	399+59.60	401+07.18		550' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
PCC	401+07.18			
58	401+07.18	406+23.63		593' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	406+23.63	407+17.62	RT	Superelevation Transition
PRC	407+17.62			
	407+17.62	408+53.81		Superelevation Transition
59	408+53.81	413+09.67		653' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	413+09.67	414+25.67	LT	Superelevation Transition
	414+25.67	415+13.01	LT	Normal Crown Section
	415+13.01	416+29.01		Superelevation Transition
60	416+29.01	419+94.87		650' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	419+94.87	421+10.87	LT	Superelevation Transition
	421+10.87	421+10.97	RT	Normal Crown Section
	421+10.97	422+26.97		Superelevation Transition
61	422+26.97	422+89.36		835' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	422+89.36	424+05.36	RT	Superelevation Transition
	424+05.36	425+63.45	RT	Normal Crown Section
	425+63.45	426+76.96		Superelevation Transition
62	426+76.96	429+57.98		835' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	429+57.98	430+71.49	RT	Superelevation Transition
	430+71.49	430+72.46	LT	Normal Crown Section
	430+72.46	431+88.46		Superelevation Transition
63	431+88.46	438+71.89		650' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	438+71.89	439+87.89	LT	Superelevation Transition
	439+87.89	441+16.23	LT	Normal Crown Section
	155167.65	1 11 110.23		Normal Crown Section

	441+16.23	442+11.17		Superelevation Transition
64	442+11.17	443+57.93		2,000' Radius Curve Right
				0.0455'/' Superelevation Rate
				Point of Rotation at Centerline
	443+57.93	444+52.87	LT	Superelevation Transition
	444+52.87	447+39.79	RT	Normal Crown Section
	447+39.79	448+55.75		Superelevation Transition
65	448+55.75	454+90.06		635' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	454+90.06	456+06.02	RT	Superelevation Transition
	456+06.02	456+57.15	LT	Normal Crown Section
	456+57.15	457+73.15		Superelevation Transition
66	457+73.15	458+59.58		835' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	458+59.58	459+75.58	LT	Superelevation Transition
	459+75.58	459+76.32	RT	Normal Crown Section
	459+76.32	459+37.69		Superelevation Transition
PRC	459+37.69			
	459+37.69	460+51.14		Superelevation Transition
67	460+51.14	462+38.37		4,000' Radius Curve Left
				0.0316'/' Superelevation Rate
				Point of Rotation at Centerline
PCC	462+38.37			
	462+38.37	462+70.22		Superelevation Transition
68	462+70.22	464+42.31		2,000' Radius Curve Left
				0.0455'/' Superelevation Rate
				Point of Rotation at Centerline
	464+42.31	465+37.25	RT	Superelevation Transition
	465+37.25	465+38.21	LT	Normal Crown Section
	465+38.21	466+53.89		Superelevation Transition
69	466+53.89	466+75.14		900' Radius Curve Right
				0.0598'/' Superelevation Rate
				Point of Rotation at Centerline
	466+75.14	467+90.82	LT	Superelevation Transition
	467+90.82	471+49.32	RT	Normal Crown Section
	471+49.32	472+65.32		Superelevation Transition
70	472+65.32	476+78.92		600' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
PCC	476+78.92			

71	476+78.92	479+13.54		650' Radius Curve Left
		1,0 1,000		0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	479+13.54	480+08.15	RT	Superelevation Transition
				·
PRC	480+08.15			
	480+08.15	480+30.34	LT	Normal Crown Section
	480+30.34	481+46.34		Superelevation Transition
72	481+46.34	482+55.24		450' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	482+55.24	483+49.85		Superelevation Transition
PRC	483+49.85			
	483+49.85	483+71.24	LT	Superelevation Transition
	483+71.24	483+72.04	RT	Normal Crown Section
	483+72.04	484+88.04		Superelevation Transition
73	484+88.04	490+33.44		650' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	490+33.44	491+49.44	RT	Superelevation Transition
	491+49.44	495+10.80	LT	Normal Crown Section
	495+10.80	496+26.80		Superelevation Transition
74	496+26.80	499+31.04		650' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	499+31.04	500+47.04	LT	Superelevation Transition
	500+47.04	500+83.40	LT	Normal Crown Section



STATE OF PROJECT SHEET TOTAL SHEETS SOUTH PAKOTA P 6403(10) 63 333

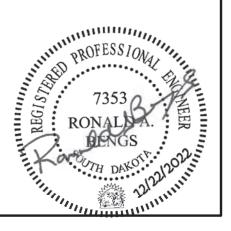
2/6/2023

Plotting Date:

TABLE OF SUPERELEVATION (Continued)

	500+83.40	501+99.40		Superelevation Transition
75	501+99.40	503+38.80		300' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	503+38.80	504+54.80	LT	Superelevation Transition
	504+54.80	504+55.24	LT	Normal Crown Section
	504+55.24	505+31.77		Superelevation Transition
76	505+31.77	505+31.77		300' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	505+31.77	506+08.30	LT	Superelevation Transition
	506+08.30	506+08.49	RT	Normal Crown Section
	506+08.49	507+24.49		Superelevation Transition
77	507+24.49	508+23.79		264' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	508+23.79	509+39.79	RT	Superelevation Transition
	509+39.79	510+52.98	RT	Normal Crown Section
	510+52.98	511+68.95		Superelevation Transition
78	511+68.95	512+74.14		350' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	512+74.14	513+90.11	RT	Superelevation Transition
	513+90.11	513+93.01	LT	Normal Crown Section
	513+93.01	515+09.01		Superelevation Transition
79	515+09.01	516+98.61		835' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	516+98.61	518+14.61	LT	Superelevation Transition
	518+14.61	520+57.49	LT	Normal Crown Section
	520+57.49	521+73.49		Superelevation Transition
80	521+73.49	525+67.98		175' Radius Curve Right
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	525+67.98	526+83.98	LT	Superelevation Transition
	526+83.98	526+84.64	RT	Normal Crown Section
	526+84.64	528+00.64		Superelevation Transition
81	528+00.64	529+15.39		140' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	529+15.39	530+31.39	RT	Superelevation Transition
	530+31.39	530+32.28	LT	Normal Crown Section

	530+32.28	531+09.27		Superelevation Transition
82	531+09.27	531+09.27		2,000' Radius Curve Right
				0.0455'/' Superelevation Rate
				Point of Rotation at Centerline
	531+09.27	531+86.26	LT	Superelevation Transition
	531+86.26	531+86.91	RT	Normal Crown Section
	531+86.91	533+02.91		Superelevation Transition
83	533+02.91	533+04.72		350' Radius Curve Left
				0.0600'/' Superelevation Rate
				Point of Rotation at Centerline
	533+04.72	543+20.72	RT	Superelevation Transition
	543+20.72	534+30.81		Normal Crown Section
		534+30.81		POE



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Irrigation Ditch

Lawn Sprinkler

Manhole Gas

Manhole Electric

Manhole Miscellaneous

Manhole Sanitary Sewer

Microwave Radio Tower

Miscellaneous Property Corner

Overhang Or Encroachment

Pedestrian Push Button Pole

Manhole Storm Sewer

Manhole Telephone

Manhole Water

Merry-Go-Round

Miscellaneous Line

Miscellaneous Post

Overhead Utility Line

Pipe With End Section

Pipe Without End Section

Power And Telephone Pole

Power Pole And Transformer

Pipe With Headwall

Playground Slide

Playground Swing

Power Meter

Propane Tank

Property Pipe

Property Stone

Railroad Profile

Railroad Signs

Railroad Switch

Railroad Track

Rebar

Riprap

Railroad Trestle

Rebar With Cap

Reference Mark

Retaining Wall

River Edge

Rockpiles

Satellite Dish

Regulatory Sign One Post

Regulatory Sign Two Post

Rock And Wire Baskets

Public Telephone

Power Pole

Power And Light Pole

Power Tower Structure

Property Pipe With Cap

Railroad Crossing Signal

Railroad Milepost Marker

Railroad ROW Marker

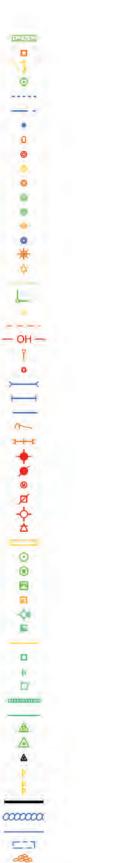
Parking Meter

Lake Edge

Mailbox

Highway ROW Marker

Interstate Close Gate



Witness Corner

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Septic Tank	-
Shrub Tree	30
Sidewalk	
Sign Face	_
Sign Post	10
Slough Or Marsh	allia allia
Spring	2
Stream Gauge	Ø
Street Marker	_
Subsurface Utility Exploration Test Hole	•
Telephone Fiber Optics	- T/F -
Telephone Junction Box	0
Telephone Pole	0
Television Cable Jct Box	-8
Television Tower	4
Test Wells/Bore Holes	۱
Traffic Signal	#
Trash Barrel	•
Tree Belt	-
Tree Coniferous	*
Tree Deciduous	8
Tree Stumps	A
Triangulation Station	A
Underground Electric Line	- P -
Underground Gas Line	- G -
Underground High Pressure Gas Line	- HG -
Underground Sanitary Sewer	- 5 -
Underground Storm Sewer	= 5 =
Underground Tank	
Underground Telephone Line	-T-
Underground Television Cable	- TV -
Underground Water Line	- w -
Warning Sign One Post	b
Warning Sign Two Post	B
Water Fountain	1
Water Hydrant	CO.
Water Meter	0
Water Tower	A
Water Valve	0
Water Well	0
Weir Rock	
Windmill	8
Wingwall	
Afficana Canada	art.

State and National Line	
County Line	
Section Line	
Quarter Line	
Sixteenth Line	
Property Line	
Construction Line	
ROW Line	
New ROW Line	
Cut and Fill Limits	
Control of Access	••••
New Control of Access	······································
Proposed ROW (After Property Disposal)	
(Aller Froperty Disposar)	
Drainage Arrow	_
Remove Concrete Pavement	77
Remove Concrete Driveway Paveme	ent WWW
Remove Asphalt Concrete Pavemen	333
Remove Concrete Sidewalk	
Remove Concrete Median Pavement	
Remove Concrete Curb and/or Gutte	r
Detectable Warning	
Pedestrian Push Button Pole	ر م
and 30" x 48" Clear Space with 1.5% slope	

STATE OF

SOUTH

Plotting Date:

SHEET

64

333

P 6403(10)

2/6/2023

Anchor

Antenna

Approach **Assumed Corner**

Azimuth Marker

Bearing Tree

Bench Mark

Box Culvert

Bulk Tank

Cemetery

Centerline

Cistern

Cattle Guard

Clothes Line

Concrete Symbol

Control Point

Creek Edge

Curb/Gutter

Deck Edge

Ditch Block

Drop Inlet

Curb

Commercial Sign Double Face

Commercial Sign One Post

Commercial Sign Overhead

Commercial Sign Two Post

Dam Grade/Dike/Levee

Doorway Threshold

Drainage Profile

Edge Of Asphalt

Edge Of Gravel

Edge Of Other

Edge Of Concrete

Edge Of Shoulder

Fence Barbwire

Fence Chainlink

Fence Miscellaneous

Fence Electric

Fence Rock

Fence Snow

Fence Wood

Fence Woven

Fire Hydrant

Flower Bed

Gas Valve Or Meter

Guide Sign One Post

Guide Sign Two Post

Gas Pump Island

Flag Pole

Grain Bin

Guardrail

Gutter

Guy Pole

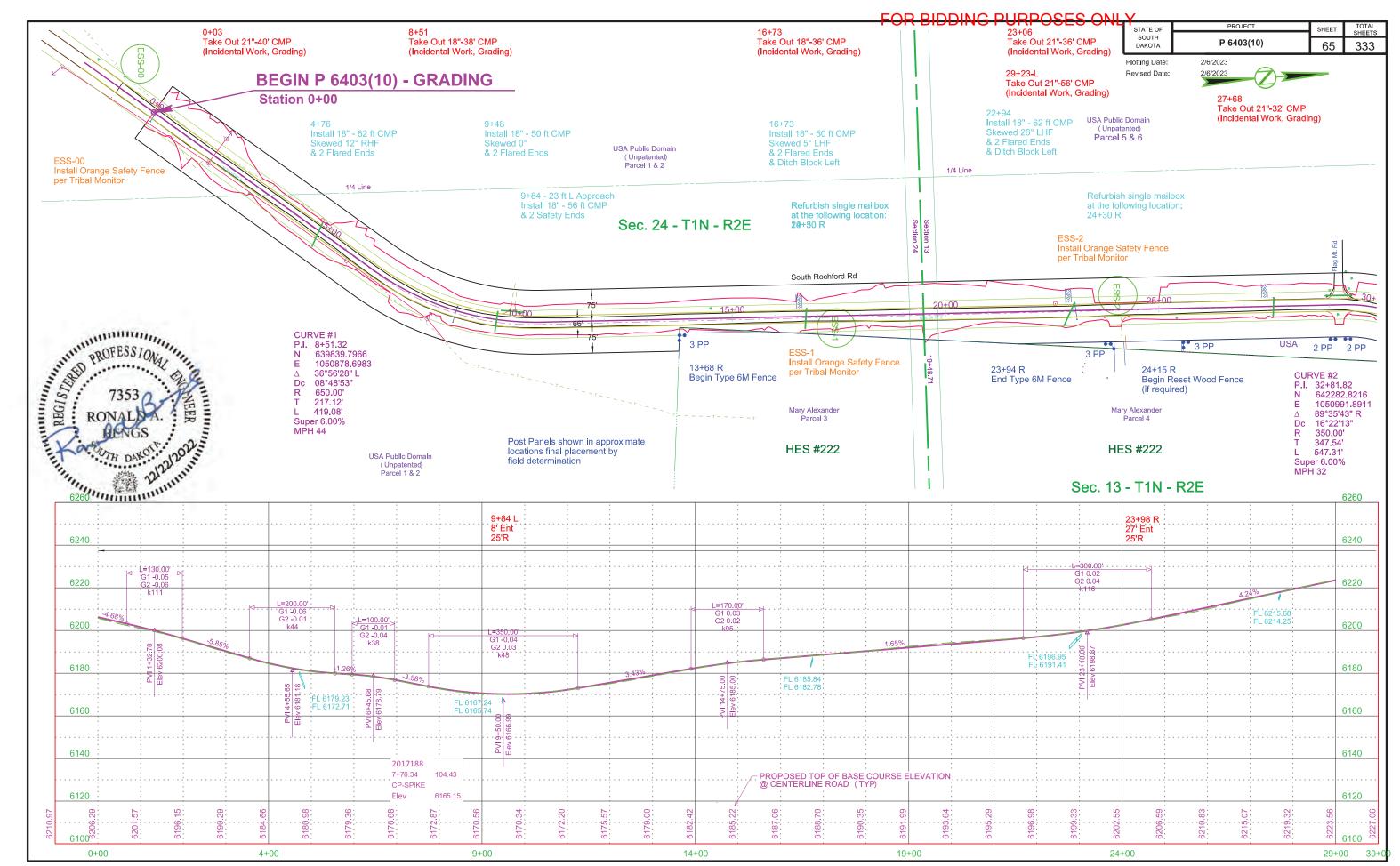
Haystack

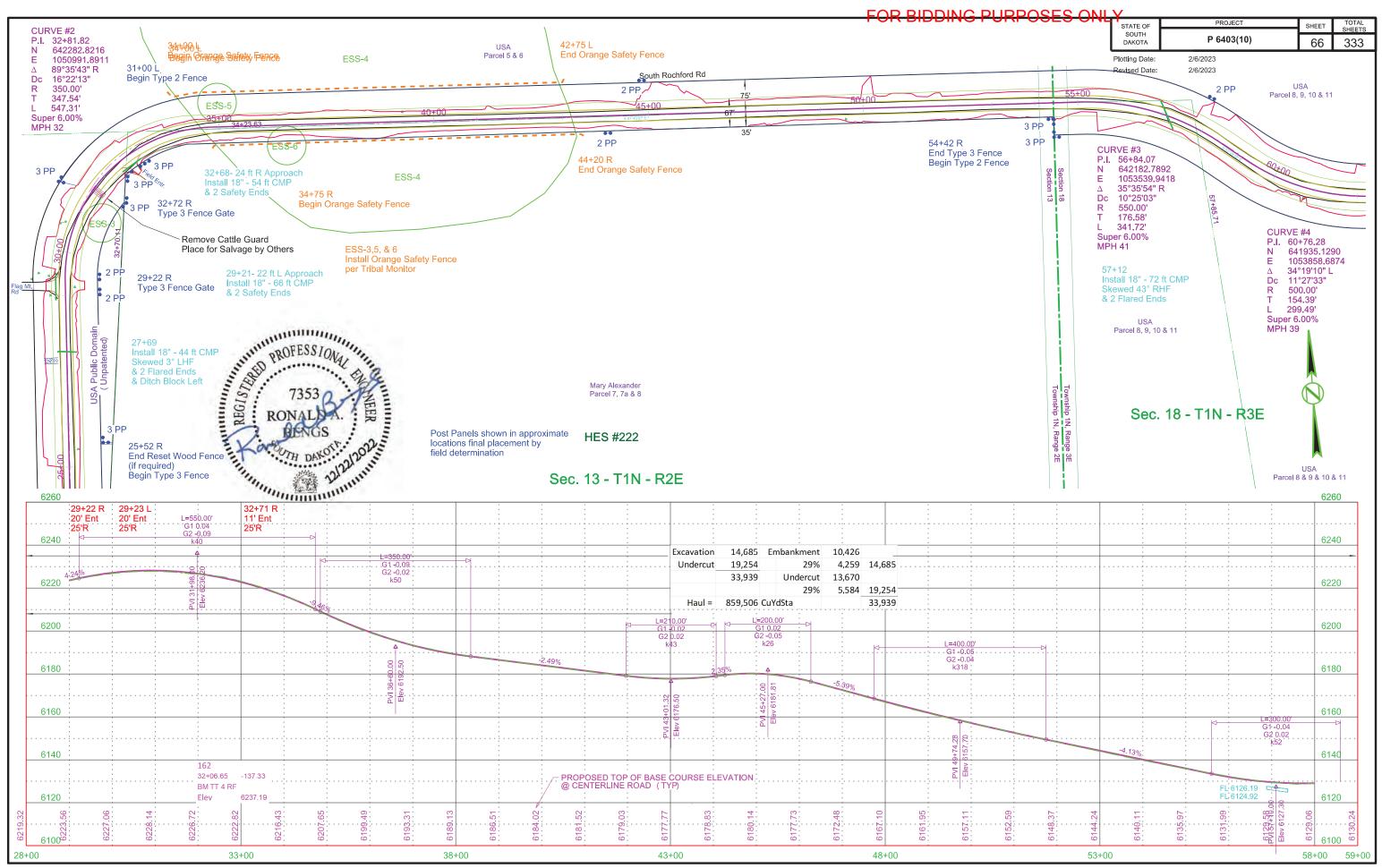
Electric Transformer/Power Junction Box

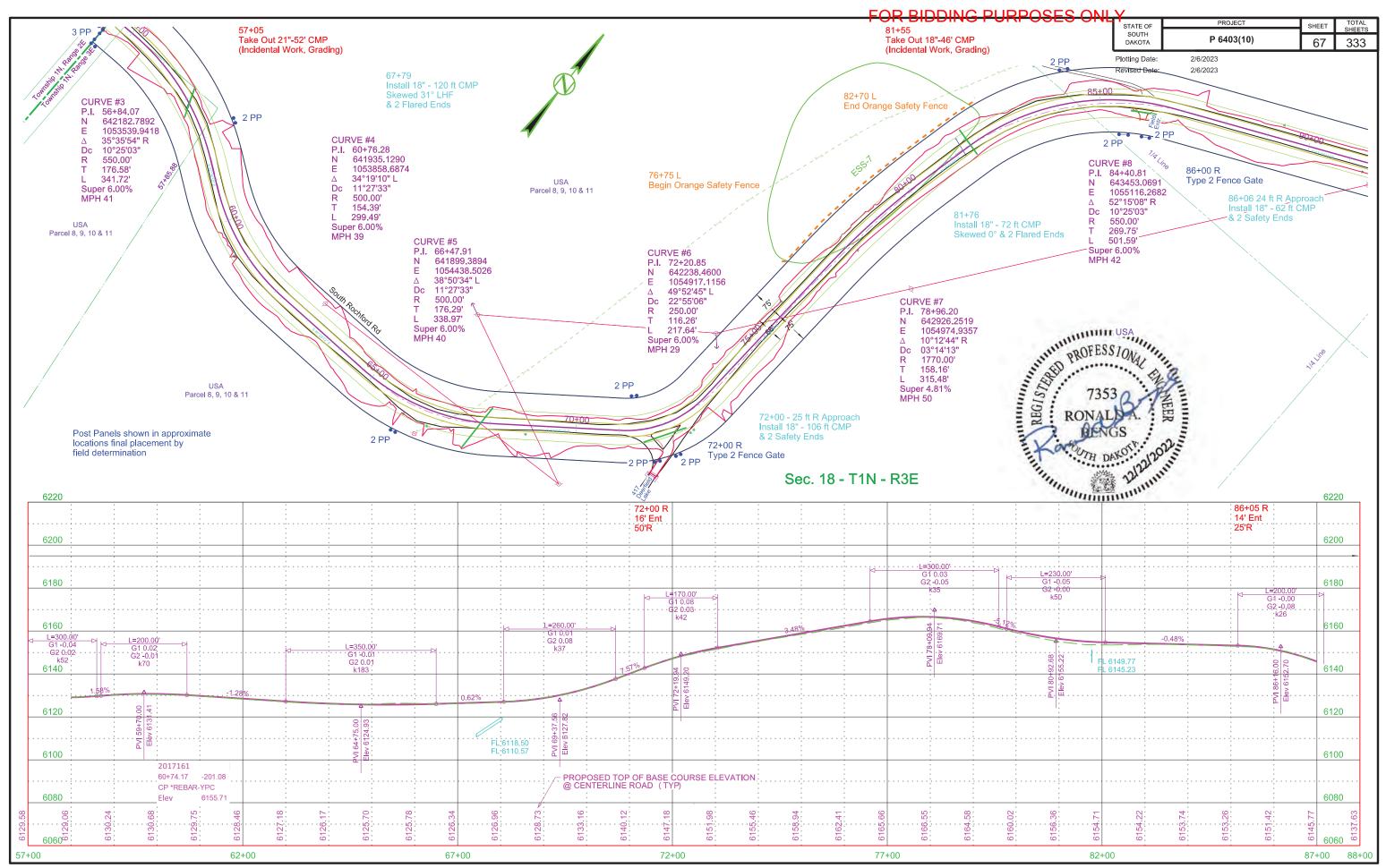
Bridge

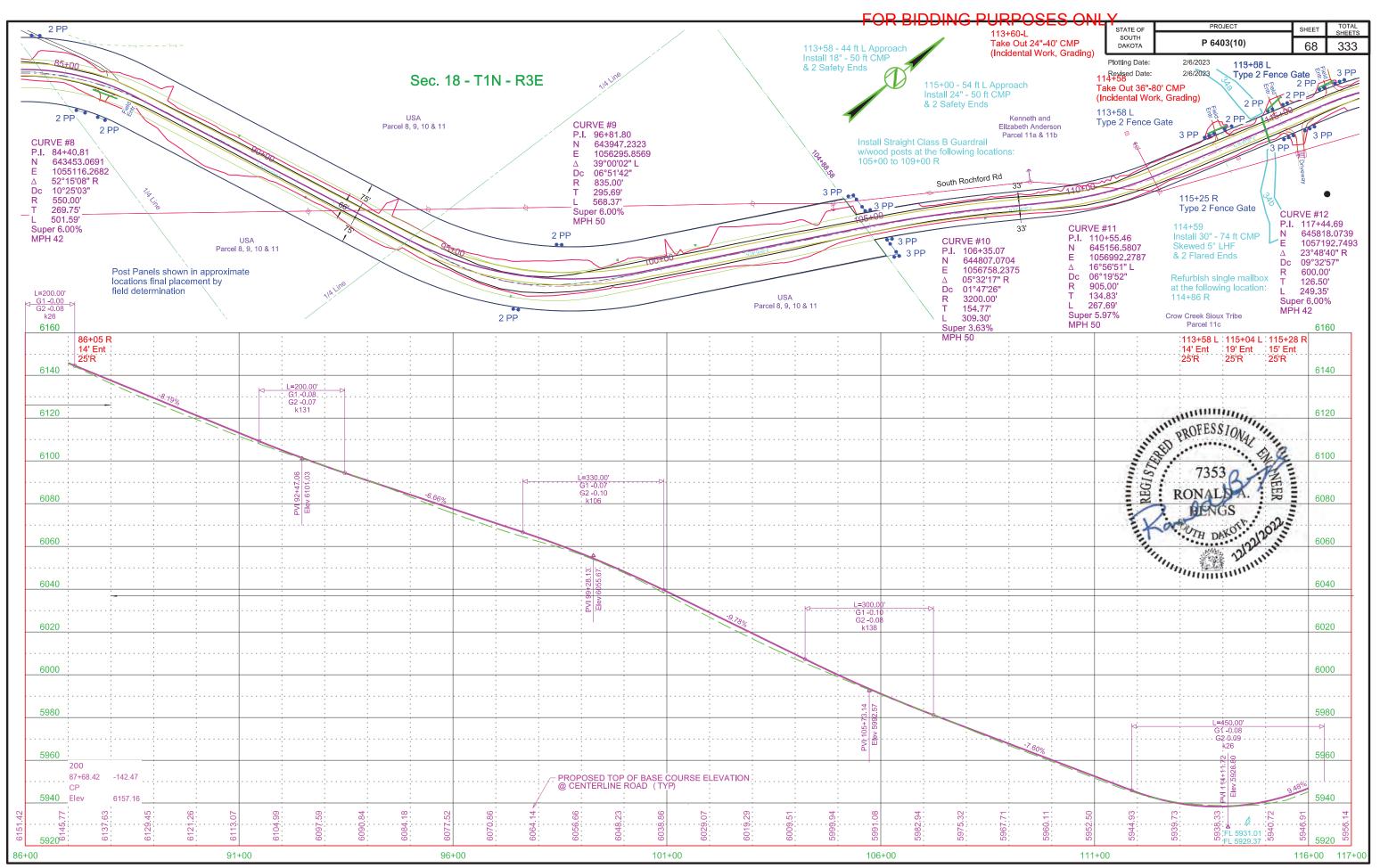
Brush Buildings

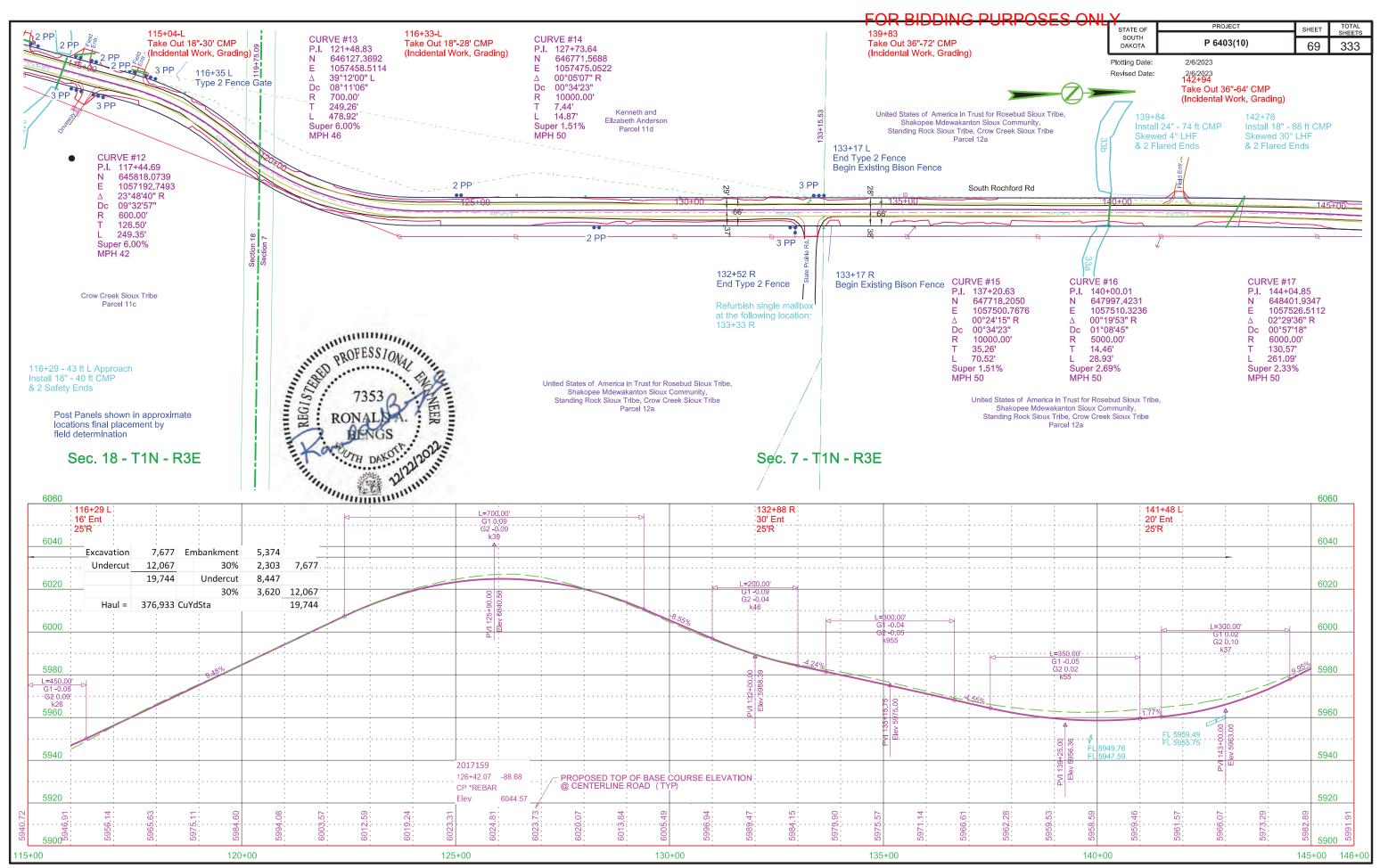
BBQ Grill/ Fireplace

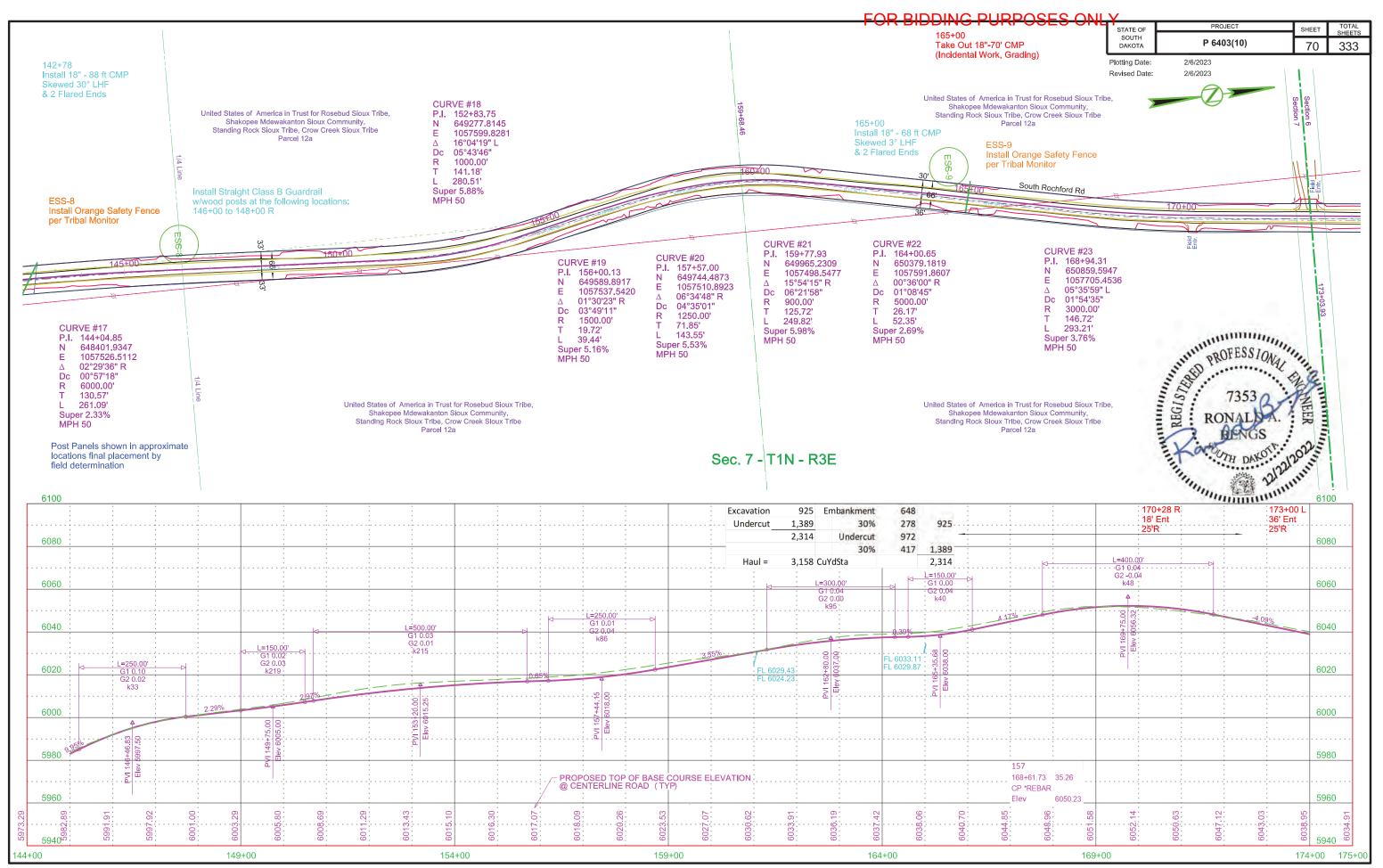


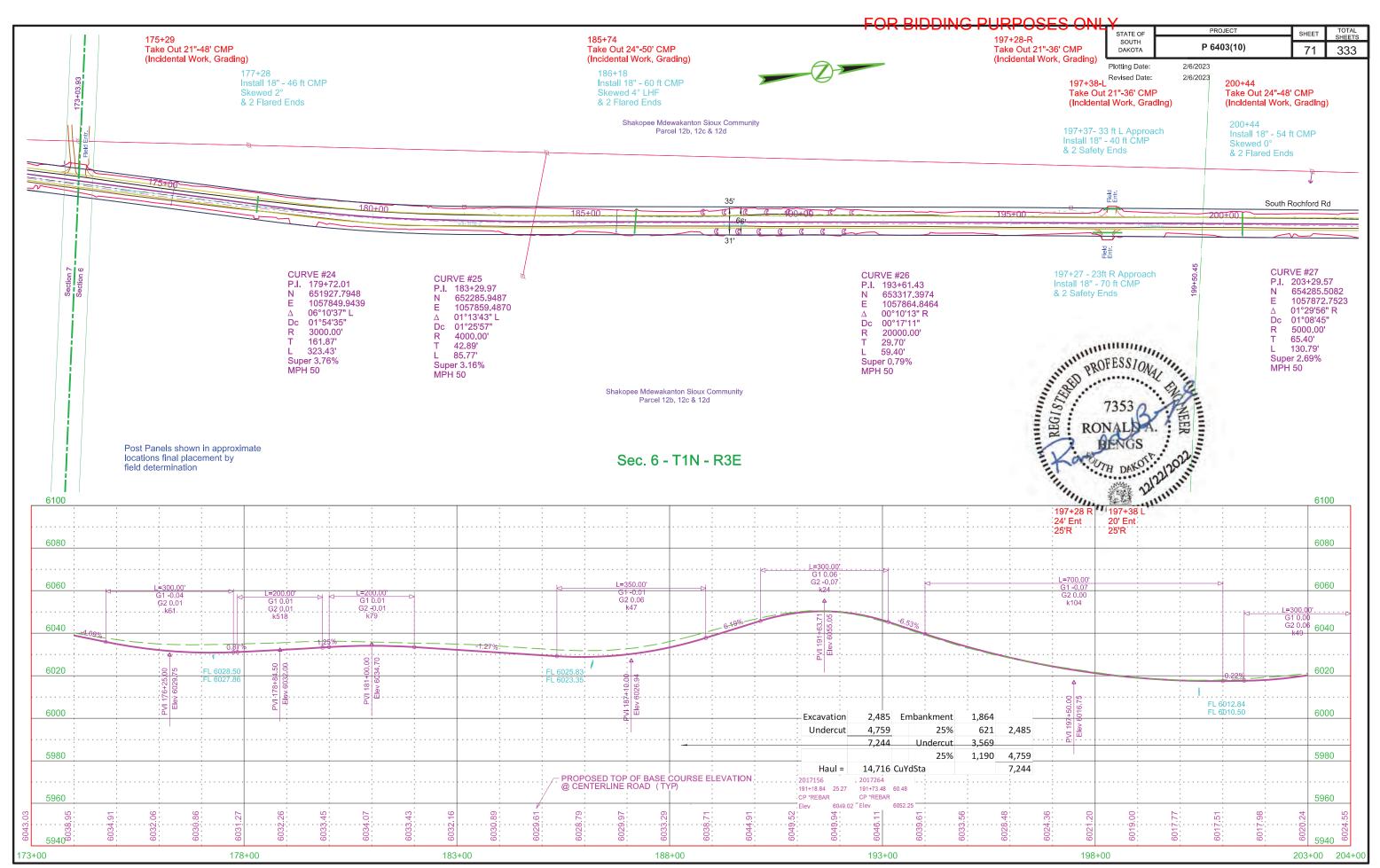


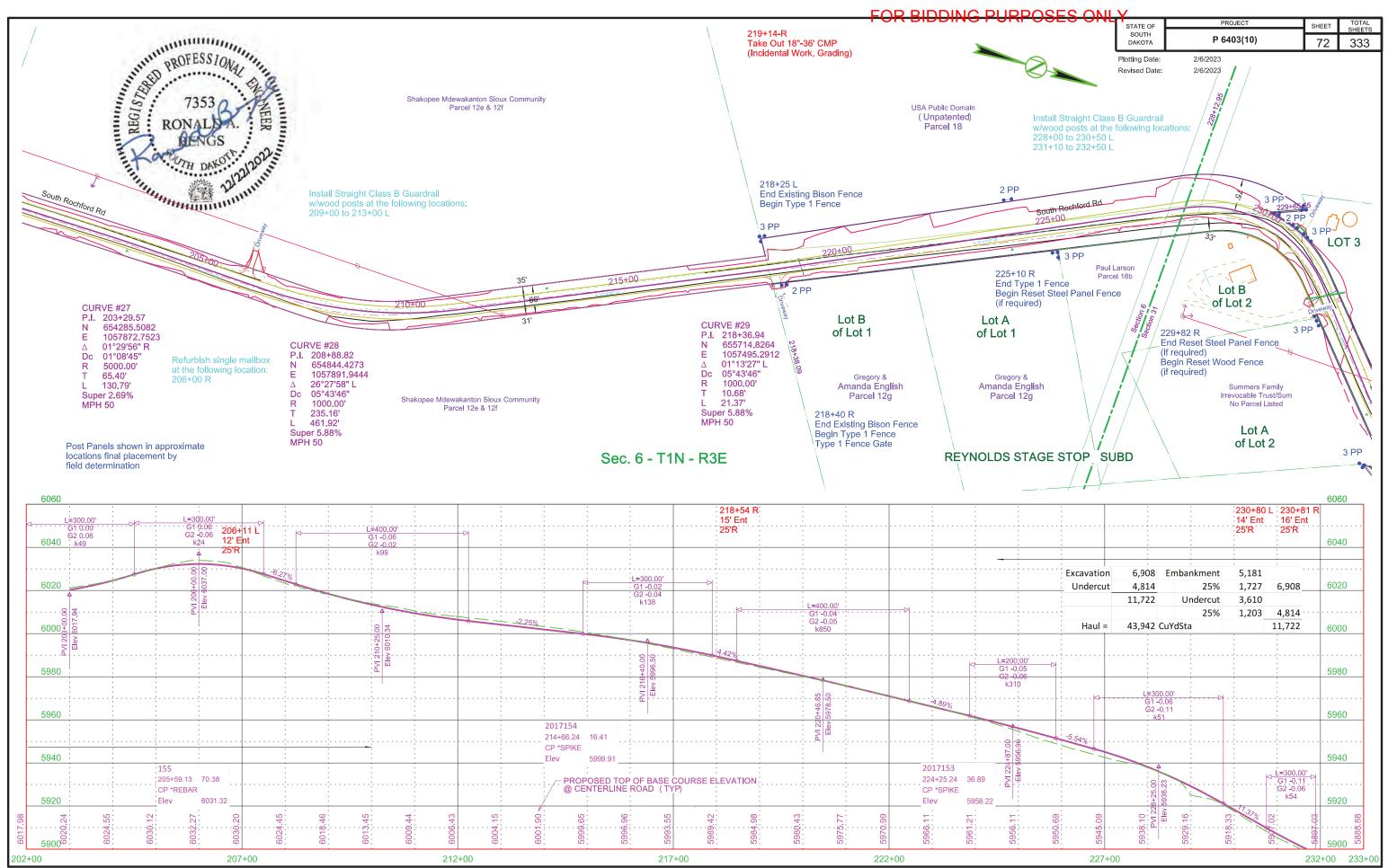


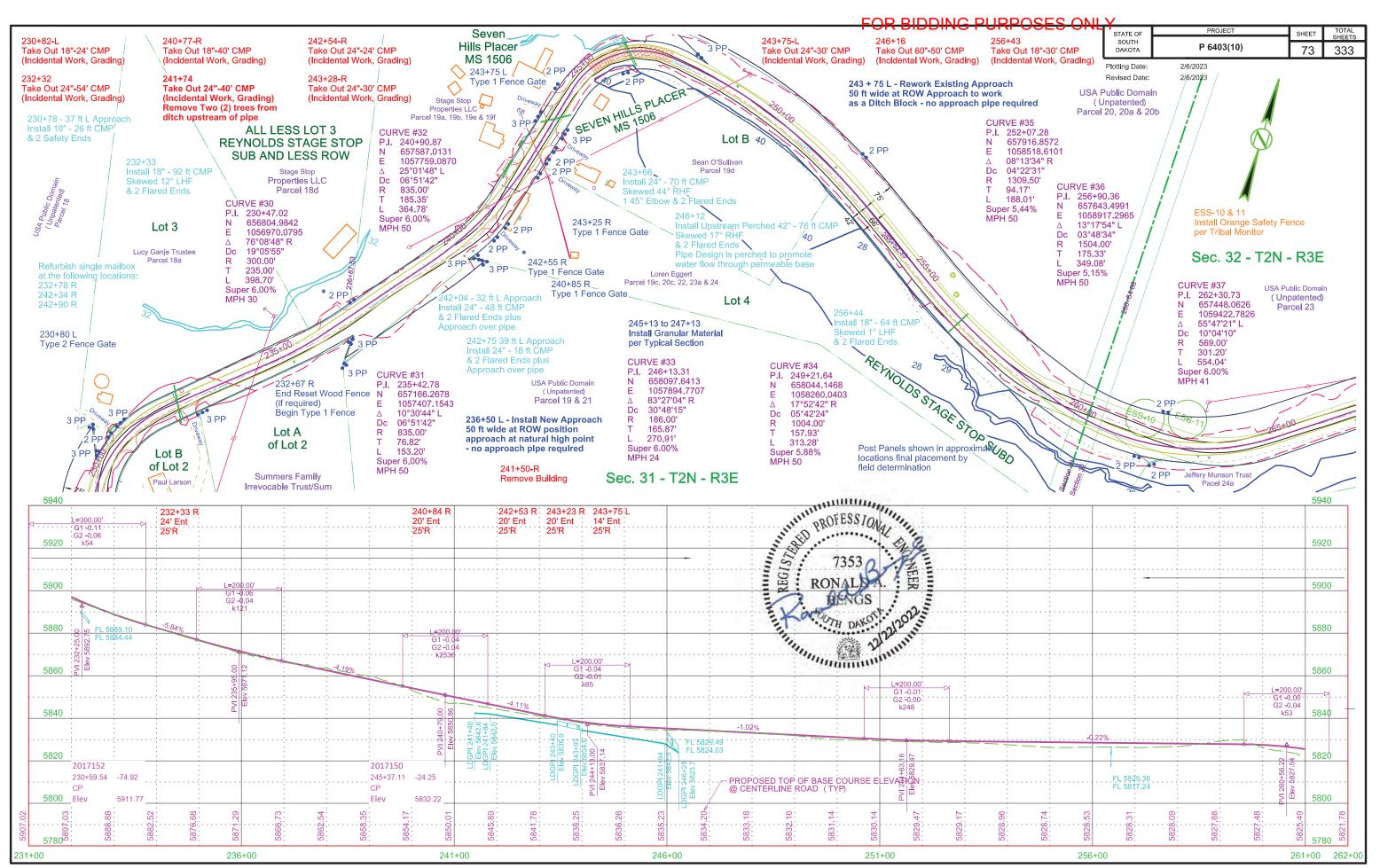


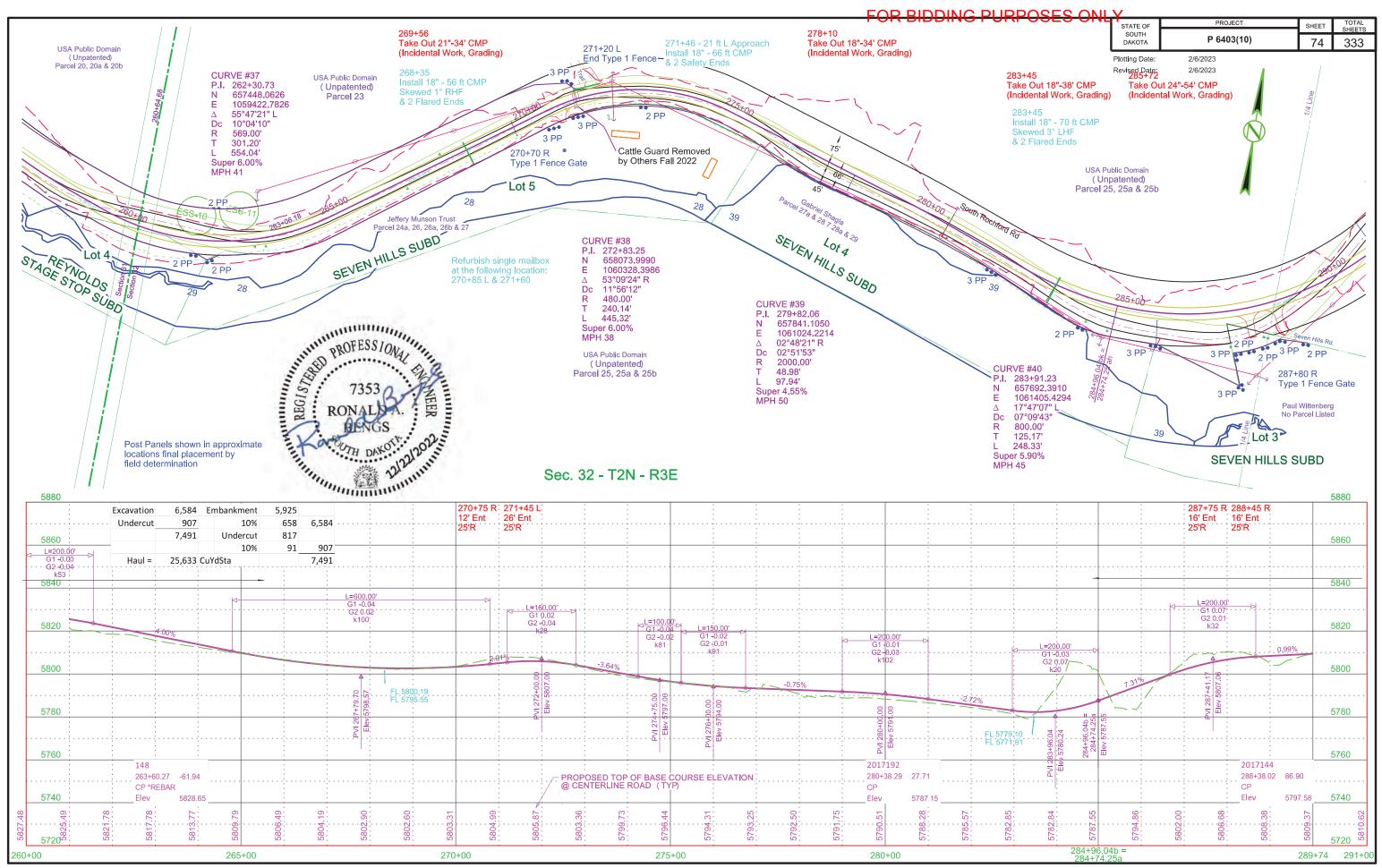


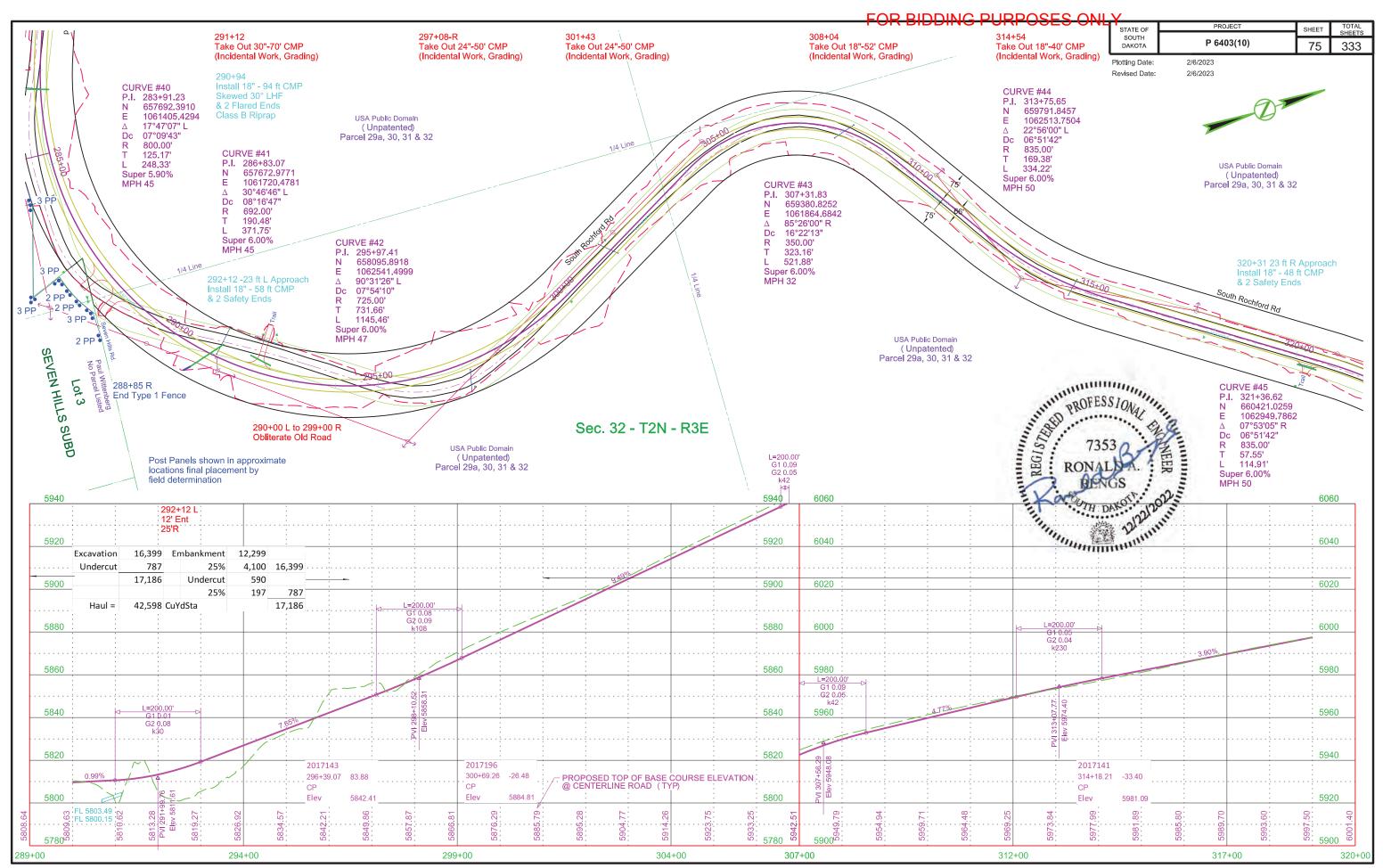


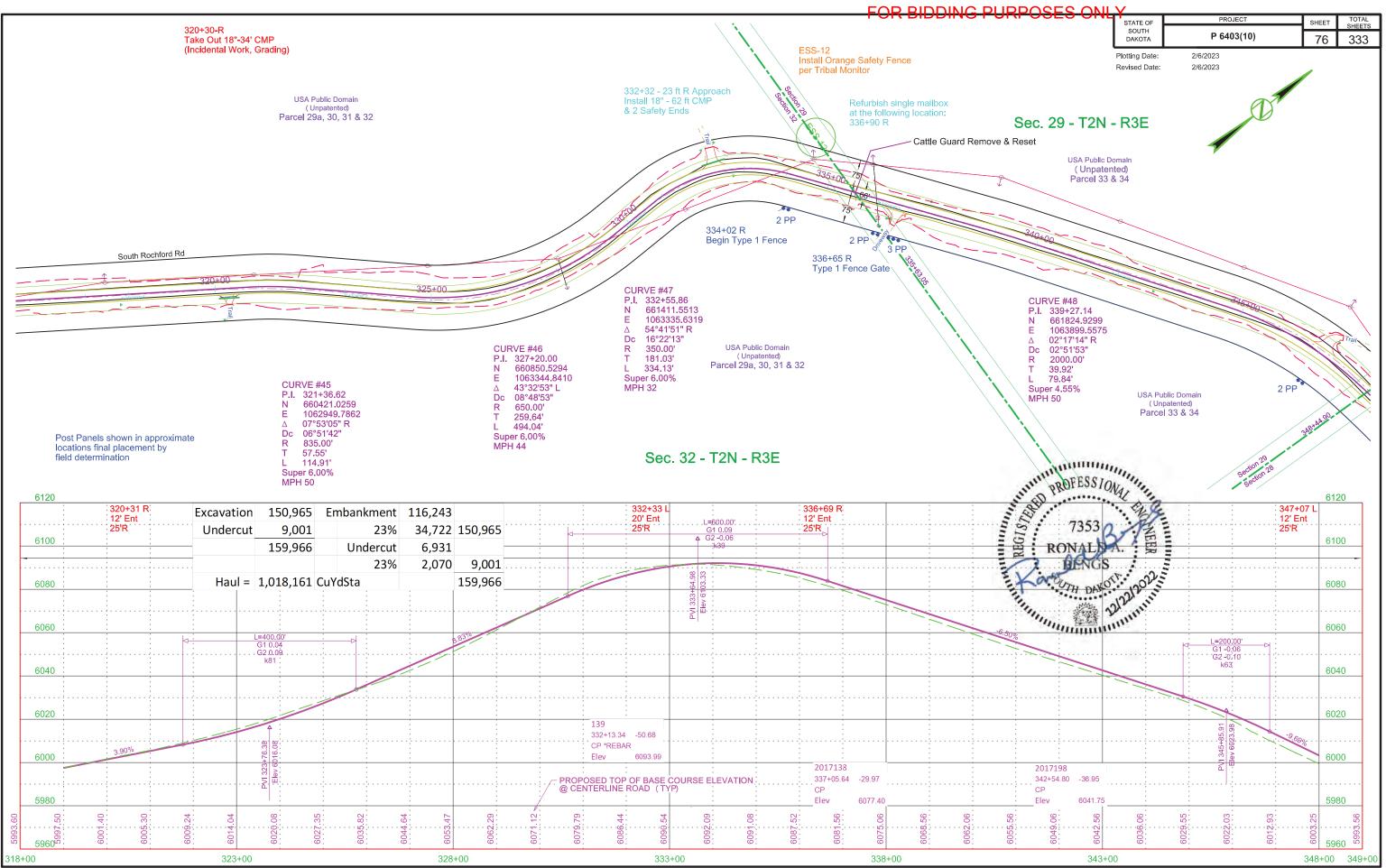


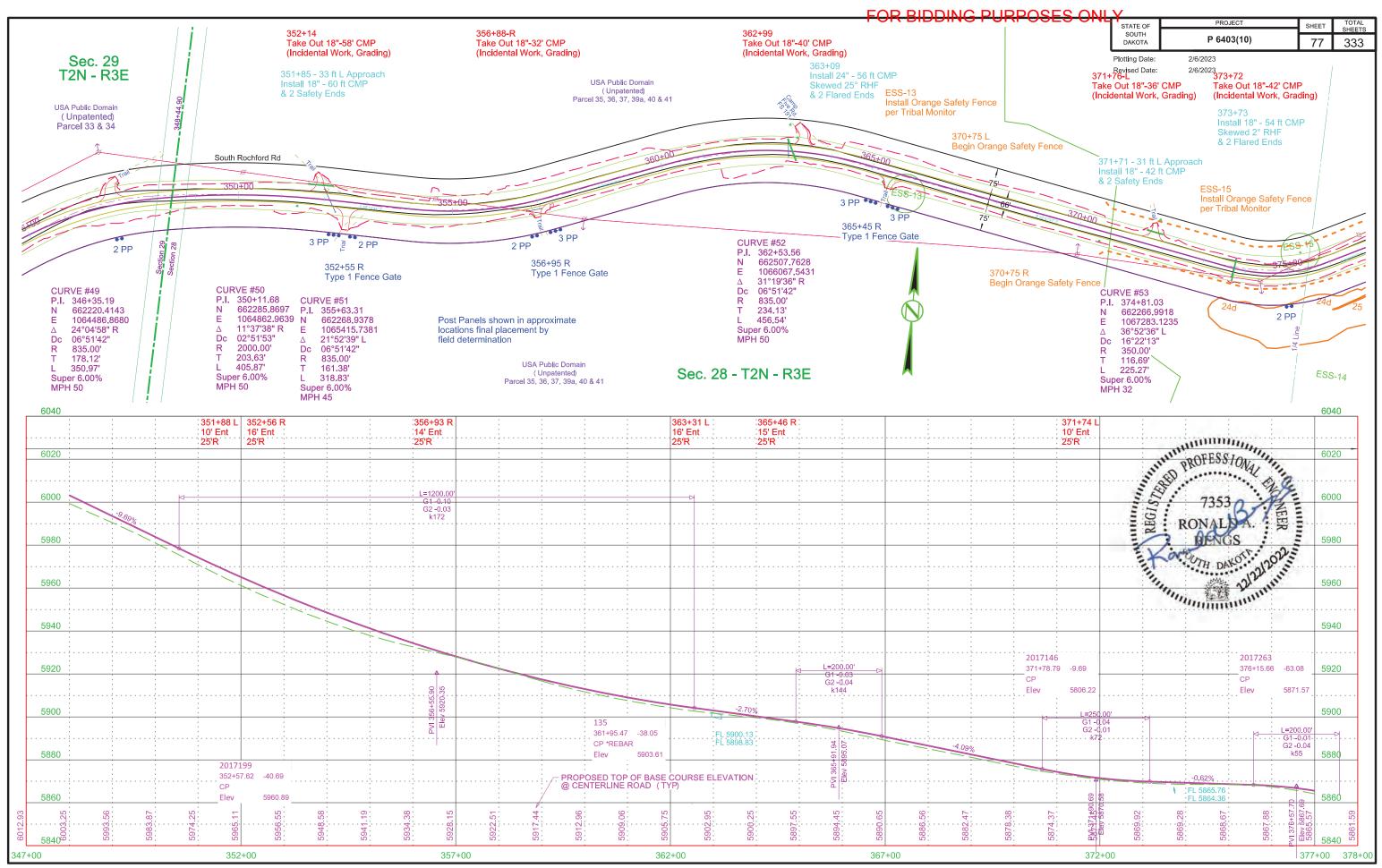


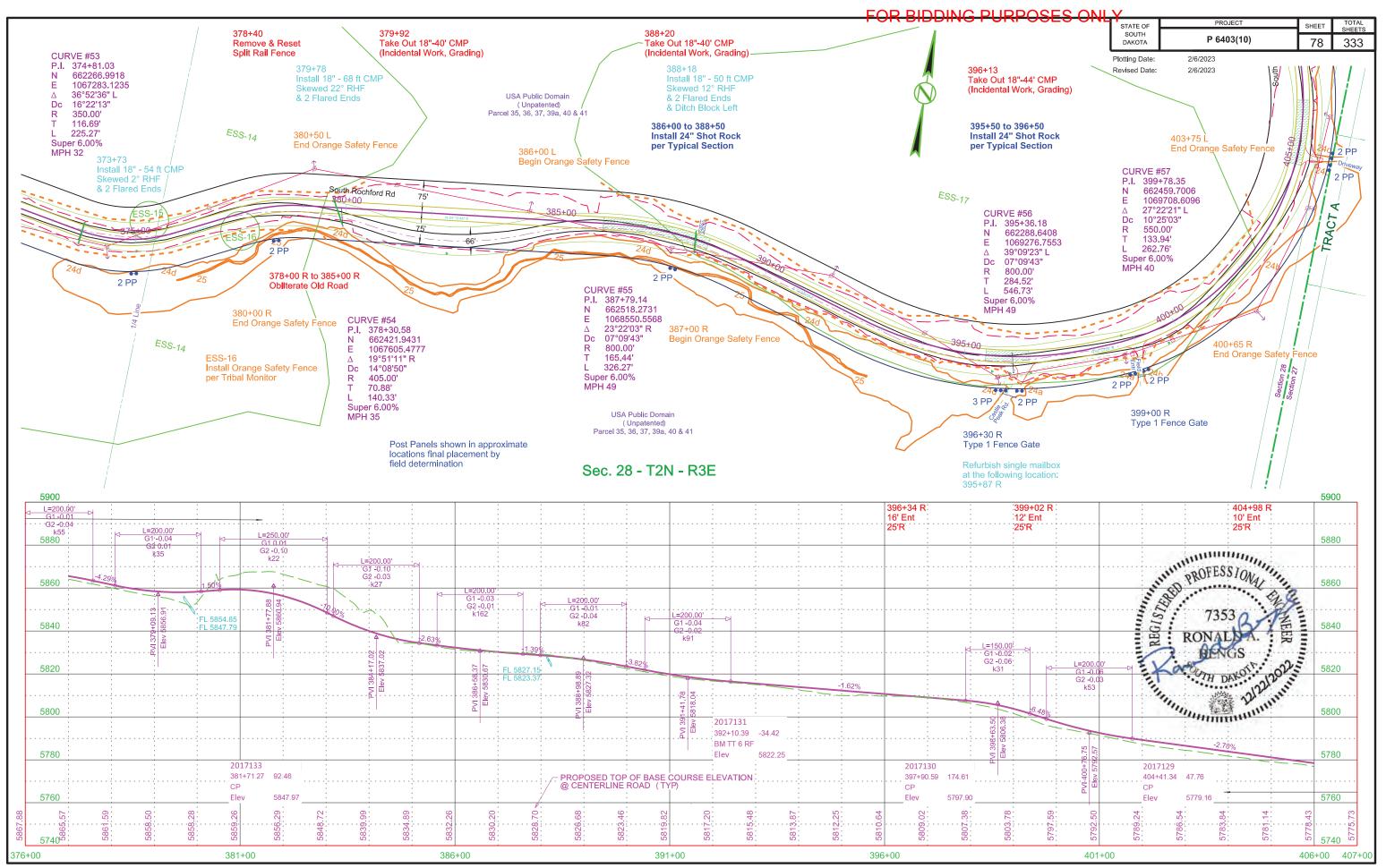


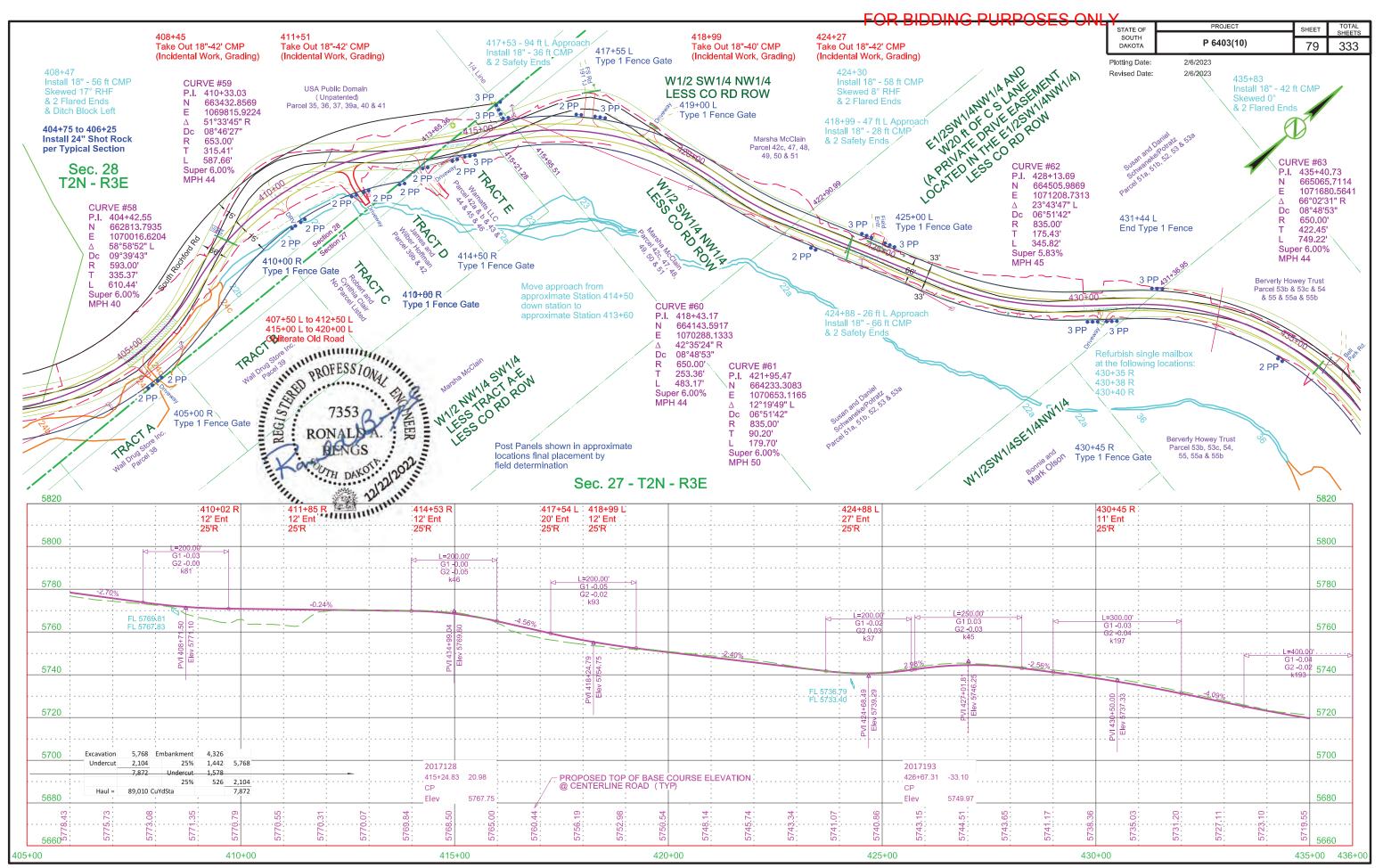


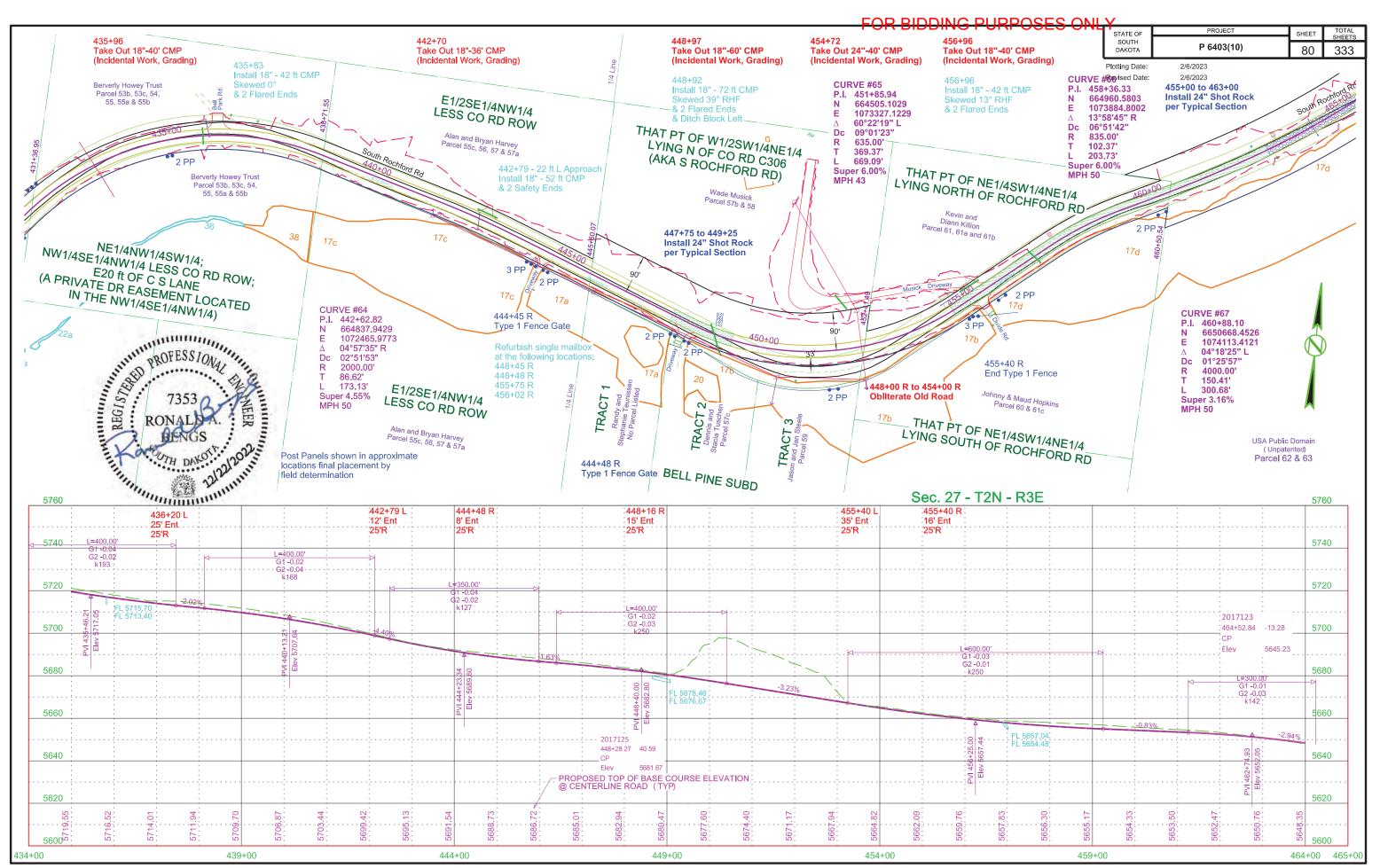


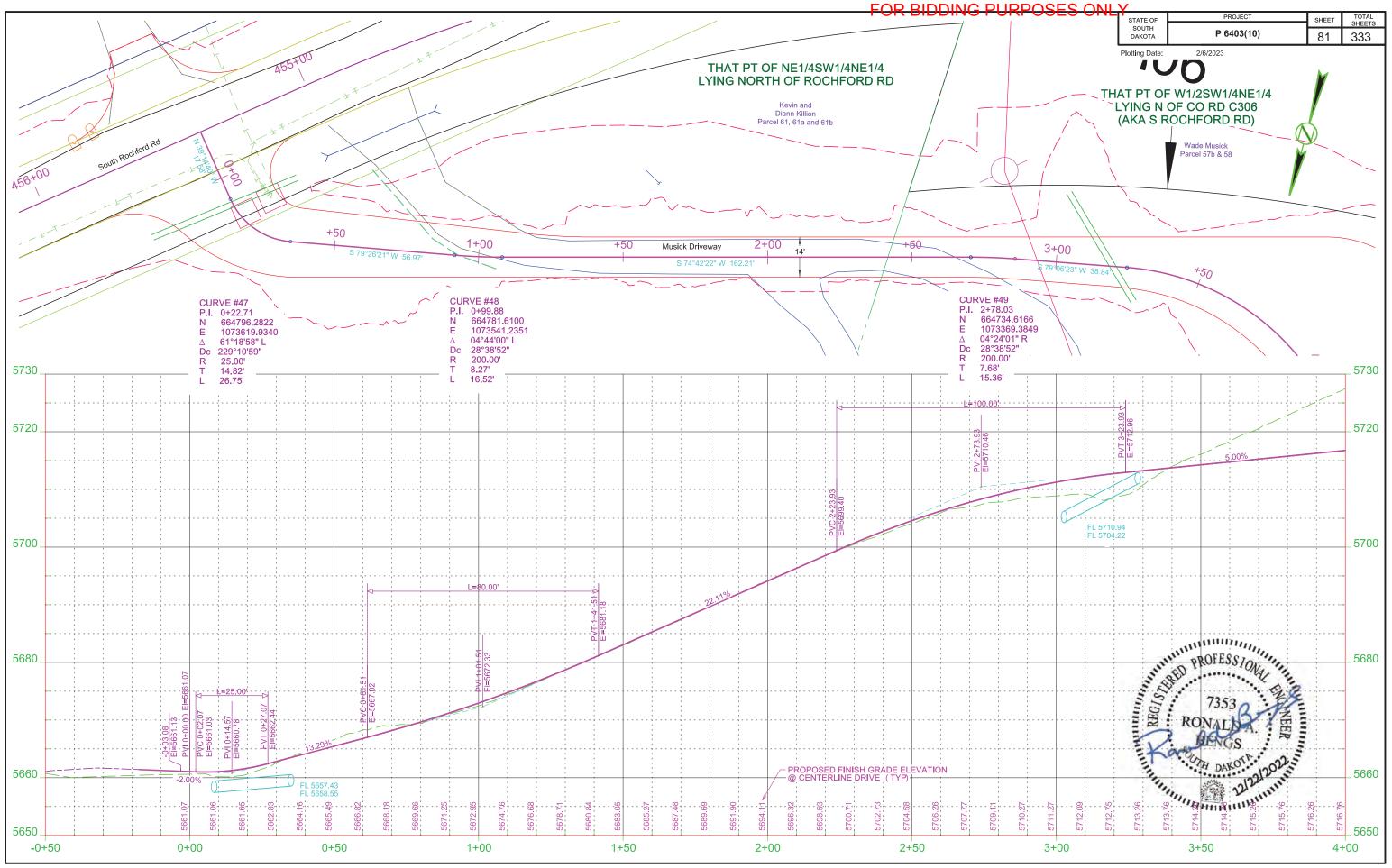


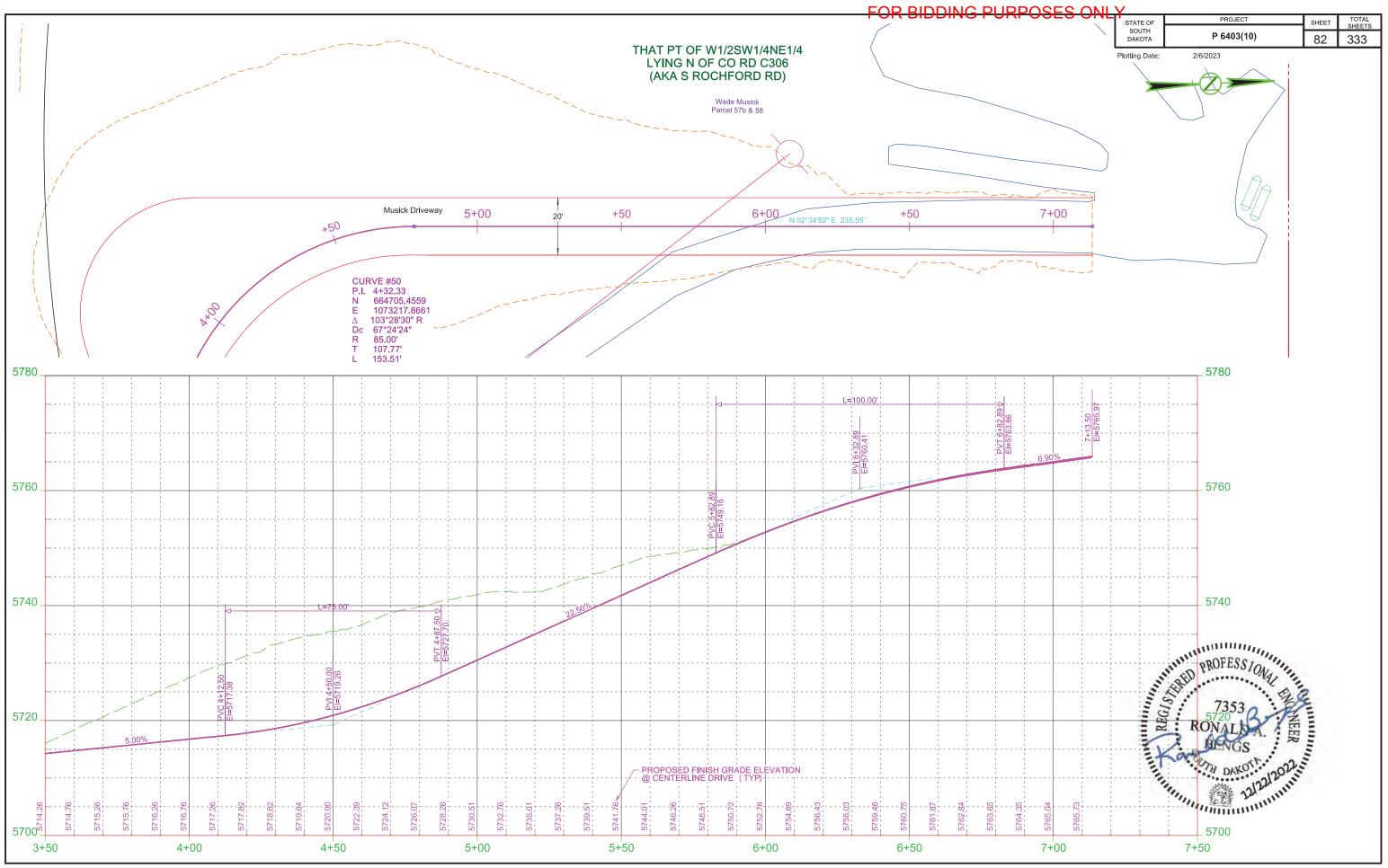


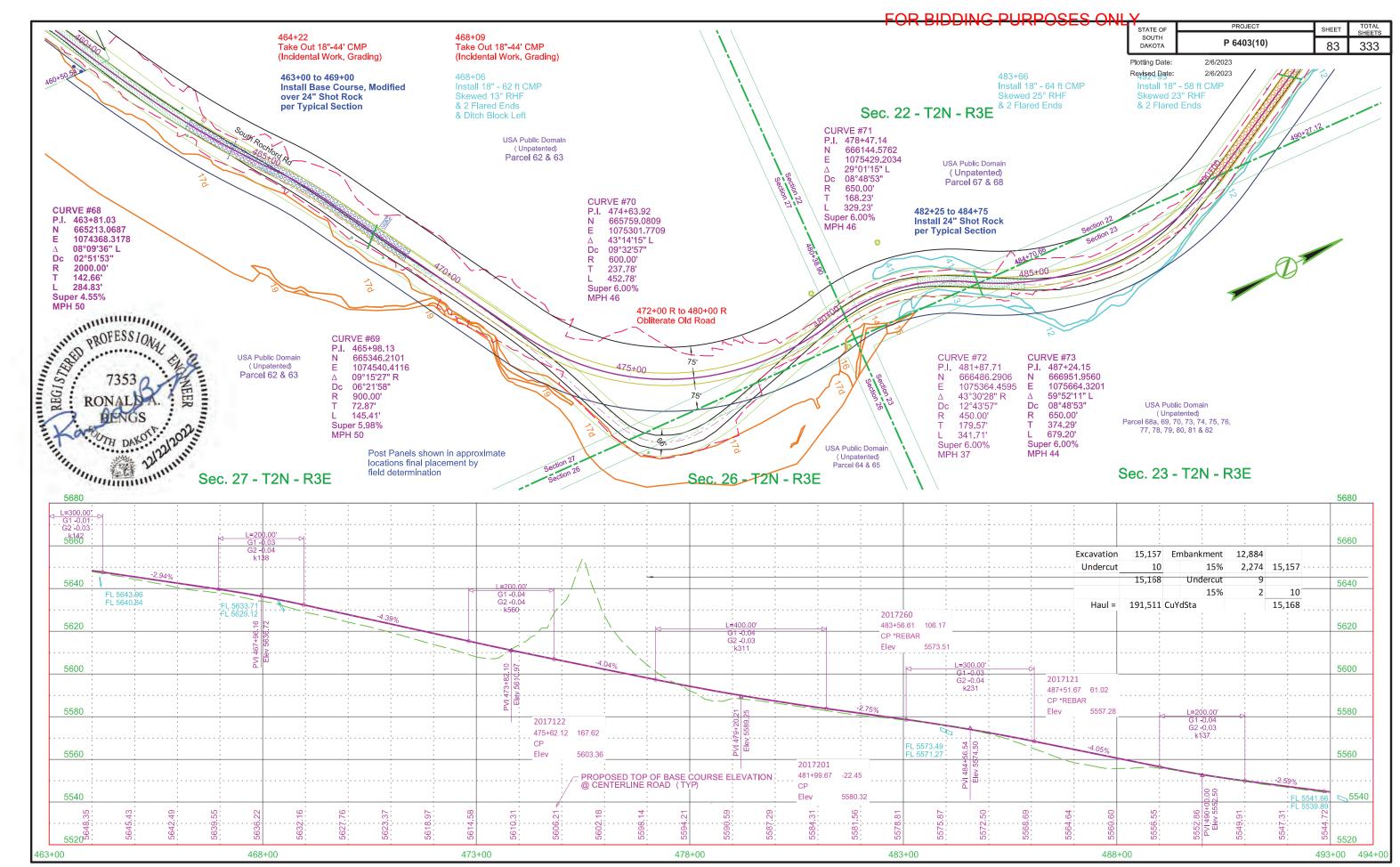


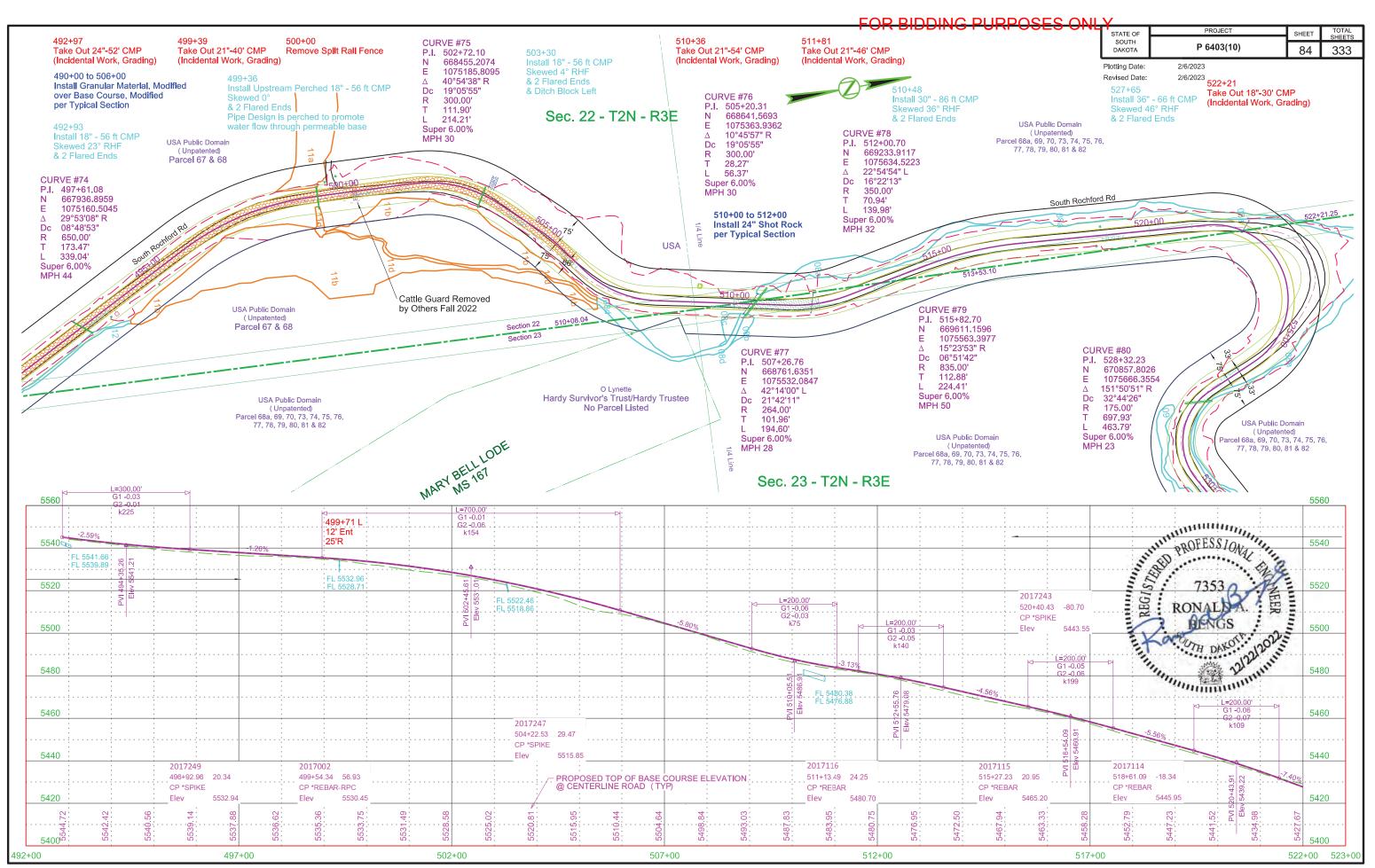


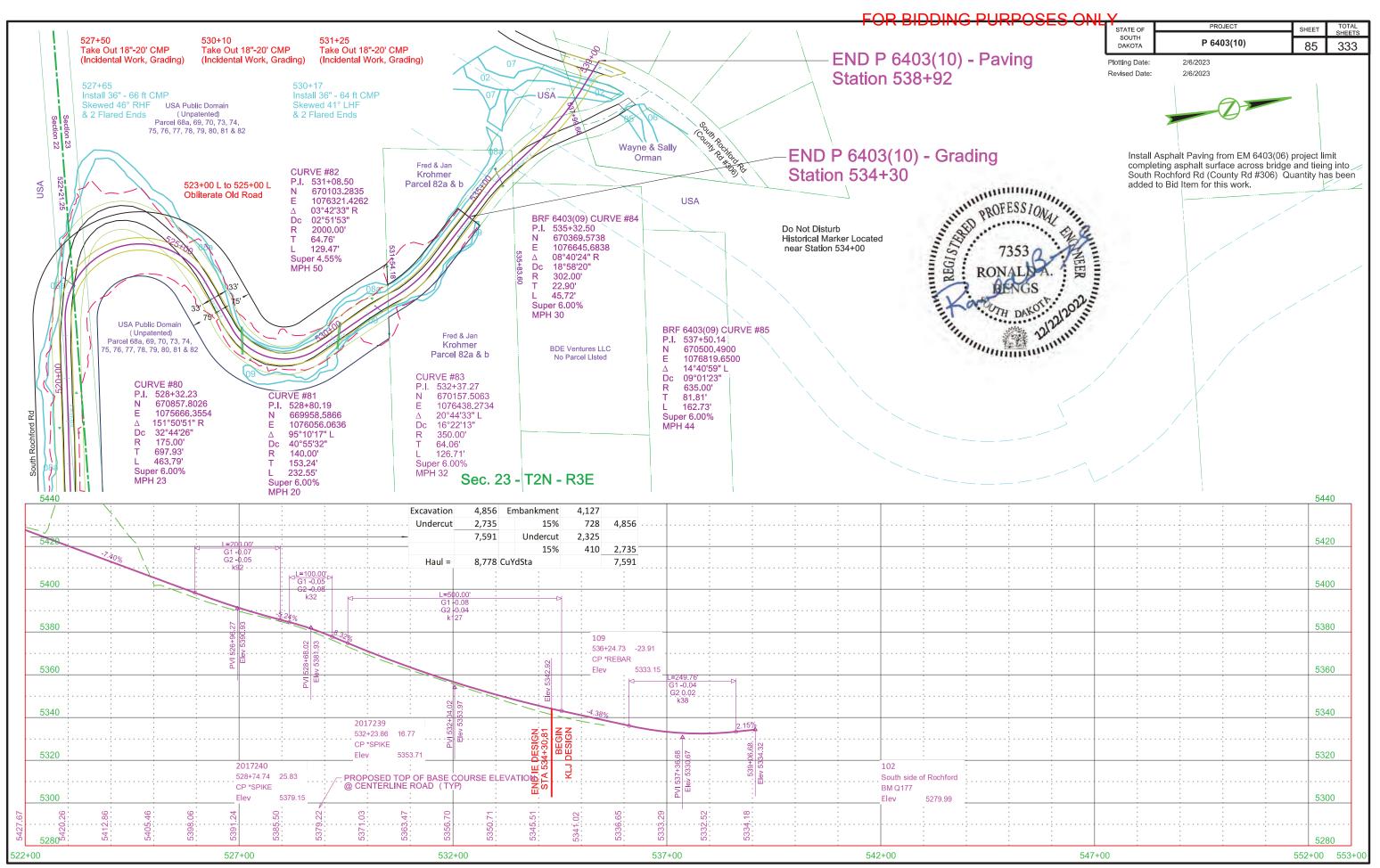












RIGHT OF WAY PLAN TABLE

STATE OF SOUTH DAKOTA P 6403(10) 333 86

Plotting Date: 2/6/2023

Parcel	Station to Station	Side	Type	Area	Owner	Description	
1	Plat			6.24 Ac 271927 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of NW1/4 & NE1/4 S24, T1N, R2E]
2	Plat			Combined with Parcel 1	USA PUBLIC DOMAIN (UNPATENTED)	A portion of NW1/4 & NE1/4 S24, T1N, R2E	
3	Plat			0.03 Ac 1303 Sq Ft	MARY ALEXANDER	Located in HES #222 in S24, T1N, R2E	
4	Plat			0.09 Ac 3899 Sq Ft	MARY ALEXANDER	Located in HES #222 in S24, T1N, R2E	
5	Plat			8.26 Ac 359780 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 S13, T1N, R2E	
6	Plat			Combined with Parcel 5	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 S13, T1N, R2E	
7	Plat			0.02 Ac 1057 Sq Ft	MARY ALEXANDER	Located in HES #222 in S13, T1N, R2E	
7a	32+38.76 to 32+97.50	R	Temp	0.02 Ac 632 Sq Ft	MARY ALEXANDER	HES #222 - SEC 13 T1N R2E]
8	Plat			17 Sq Ft	MARY ALEXANDER	Located in HES #222 in S13, T1N, R2E]
9	Plat			17.37 Ac 756812 Sq Ft	USA	A portion of HES #222, SW1/4, NW1/4, and NE 1/4 S18, T1N, R3E]
10	Plat			Combined with Parcel 9	USA	A portion of HES #222, SW1/4, NW1/4, and NE 1/4 S18, T1N, R3E]
11	Plat			Combined with Parcel 9	USA	A portion of HES #222, SW1/4, NW1/4, and NE 1/4 S18, T1N, R3E]
11a	71+68.82 to 72+02.60	R	Temp	0.03 Ac 1447 Sq Ft	USA	A portion of HES #222, SW1/4, NW1/4, and NE 1/4 S18, T1N, R3E]
11b	113+07.98 to 113+84.08	L	Temp	0.05 Ac 1940 Sq Ft	KENNETH & ELIZABETH ANDERSON	NW1/4NE1/4 LYING W OF CO. RD LESS RTY; N1/2NW1/4]
11c	114+58.64 to 115+50.76	L	Temp	0.05 Ac 2311 Sq Ft	KENNETH & ELIZABETH ANDERSON	NW1/4NE1/4 LYING W OF CO. RD LESS RTY; N1/2NW1/4	1
11d	114+98.37 to 115+58.98	R	Temp	0.04 Ac 1700 Sq Ft	CROW CREEK SIOUX TRIBE	N1/2NE1/4 LYING EAST CO RD	THILIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII
11e	115+91.97 to 116+56.99	L	Temp	0.03 Ac 1386 Sq Ft	KENNETH & ELIZABETH ANDERSON	NW1/4NE1/4 LYING W OF CO. RD LESS RTY; N1/2NW1/4	OFESS 10NAL MILL
11f	140+99.57 to 141+78.31	L	Temp	0.02 Ac 1049 Sq Ft	United States of America in Trust for Rosebud Sioux Tribe, Shakopee Mdewakanton Sioux Community, Standing Rock Sioux Tribe, Crow Creek Sioux Tribe	N1/2NE1/4 LYING EAST CO RD NW1/4NE1/4 LYING W OF CO. RD LESS RTY; N1/2NW1/4 E1/2SE1/4NW1/4 INCLUDING LOT A OF SE1/4NW1/4; SW1/4NE1/4 INCLUDING LOT A; NW1/4SE1/4 A portion of HES #222, SW1/4, NW1/4, and NE 1/4 S18, T1N, R3E	7353 NALDA.
12	Plat			Combined with Parcel 9	USA	A portion of HES #222, SW1/4, NW1/4, and NE 1/4 S18, T1N, R3E	NALDA. Z
12a	172+45.96 to 173+02.64	L	Temp	0.01 Ac 589 Sq Ft	United States of America in Trust for Rosebud Sioux Tribe, Shakopee Mdewakanton Sioux Community, Standing Rock Sioux Tribe, Crow Creek Sioux Tribe	N1/2NE1/4, NE1/4NW1/4, LOT 1-3, LESS ROW SEC 7 T1N R3E	TH DAKOTA 202
12b	173+02.64 to 173+47.28	L	Temp	0.01 Ac 488 Sq Ft	SHAKOPEE MDEWAKANTON SIOUX COMMUNITY	LOT 6-7; NE1/4SW1/4; SE1/4SW1/4; SE1/4 LESS ROW	22/11/11/1
12c	197+05.16 to 197+69.49	R	Temp	0.02 Ac 654 Sq Ft	SHAKOPEE MDEWAKANTON SIOUX COMMUNITY	LOT 6-7; NE1/4SW1/4; SE1/4SW1/4; SE1/4 LESS ROW	
12d	197+19.35 to 197+63.74	L	Temp	0.01 Ac 355 Sq Ft	SHAKOPEE MDEWAKANTON SIOUX COMMUNITY	LOT 6-7; NE1/4SW1/4; SE1/4SW1/4; SE1/4 LESS ROW	
12e	205+88.91 to 206+49.10	L	Temp	0.04 Ac 1897 Sq Ft	SHAKOPEE MDEWAKANTON SIOUX COMMUNITY	HES #236 LESS LOT A LESS ROW SEC 6 T1N R3E	
12f	218+27.34 to 218+43.31	R	Temp	0.004 Ac 185 Sq Ft	SHAKOPEE MDEWAKANTON SIOUX COMMUNITY	HES #236 LESS LOT A LESS ROW SEC 6 T1N R3E]

STATE OF SOUTH DAKOTA P 6403(10) 87

Plotting Date:

Parcel	Station to Station	Side	Туре	Area	Owner	Description							
12g	218+43.31 to 218+76.69	R	Temp	0.01 Ac 589 Sq Ft	GREGORY & AMANDA ENGLISH	LOT B OF LOT 1 REYNOLDS STAGE STOP SUBD							
18	Plat			1.47 Ac 64108 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of NE1/4 SEC 6 T1N R3E							
18a	230+05.41 to 235+89.92	L	Temp	0.17 Ac 7264 Sq Ft	LUCY GANJE TRUSTEE	LOT 3 REYNOLDS STAGE STOP SUBD SEC 31 T2N R3E							
18b	232+15.07 to 232+73.20	R	Temp	0.02 Ac 963 Sq Ft	PAUL LARSON	LOT B OF LOT 2 REYNOLDS STAGE STOP SUBD SEC 31 T2N R3E							
18c	232+94.20 to 236+69.18	R	Temp	0.12 Ac 5249 Sq Ft	SUMMERS FAMILY IRREVOCABLE TRUST/SUM	LOT A OF LOT 2 SEC 31 T2N R3E							
18d	235+89.92 to 237+15.40	L	Temp	0.04 Ac 1646 Sq Ft	STAGE STOP PROPERTIES LLC	ALL LESS LOT 3 REYNOLDS STAGE STOP SUB AND LESS ROW SEVEN HILLS LODE MS 2077 SEC 31 T2N R3E							
18e	236+43.03 to 236+69.95	R	Temp	0.01 Ac 595 SqFt	SUMMERS FAMILY IRREVOCABLE TRUST/SUM	LOT A OF LOT 2 SEC 31 T2N R3E							
19	Plat			3.93 Ac 170871 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 SEC 31 T2N R3E							
19a	237+15.40 to 243+98.96	L	Temp	0.13 Ac 5455 Sq Ft	STAGE STOP PROPERTIES LLC	ALL LESS LOTS A AND B, LESS LOT 1-5 OF SEVEN HILLS SUB, LESS REYNOLDS STAGE STOP SUB AND LESS ROW (ALSO IN SEC 30 AND 32 T2N R3E AND SEC 25 AND 36 T2N R2E) SEVEN HILLS PLACER MS 1506							
19ab	237+15.40 to 243+98.96	L	Drainage Easement	0.06 Ac 2567 Sq Ft	STAGE STOP PROPERTIES LLC	ALL LESS LOTS A AND B, LESS LOT 1-5 OF SEVEN HILLS SUB, LESS REYNOLDS STAGE STOP SUB AND LESS ROW (ALSO IN SEC 30 AND 32 T2N R3E AND SEC 25 AND 36 T2N R2E) SEVEN HILLS PLACER MS 1506							
19b	237+03.91 to 238+25.87	R	Temp	0.04 Ac 1636 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	ALL LESS PT MS #1506, LESS PT MS #2077 SEC 31 T2N R3E							
19c	242+23.53 to 242.57.21	R	Temp	0.01 Ac 272 Sq Ft	LOREN EGGERT	LOT 4 REYNOLDS STAGE STOP SUBD SEC 31 T2N R3E							
19e	244+44.52 to 247+96.55	L	Temp	0.14 Ac 5950 Sq Ft	STAGE STOP PROPERTIES LLC	ALL LESS LOTS A AND B, LESS LOT 1-5 OF SEVEN HILLS SUB, LESS REYNOLDS STAGE STOP SUB AND LESS ROW (ALSO IN SEC 30 AND 32 T2N R3E AND SEC 25 AND 36 T2N R2E) SEVEN HILLS PLACER MS 1506							
20	Plat			Combined with Parcel 19	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 SEC 31 T2N R3E							
20a	249+96.33 to 257+95.70	L	Temp	0.22 Ac 9532 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	ALL LESS PT MS #1506, LESS PT MS #2077 SEC 31 T2N R3E							
20c	258+70.35 to 261+81.78	R	Temp	0.13 Ac 5740 Sq Ft	LOREN EGGERT	LOT 4 REYNOLDS STAGE STOP SUBD SEC 31 T2N R3E							
21	Plat			Combined with Parcel 19	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 SEC 31 T2N R3E							
22	Plat			0.11 Ac 4584 Sq Ft	LOREN EGGERT	Lot 4 of Reynolds Stage Stop Subdivision, in the SE1/4 of Section 31 and in the SW1/4 of SW1/4 of Section 32, Township 2 North, Range 3 East							
23	Plat			22.7 Ac 987917 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4, SE1/4, NW1/4, NE1/4 SEC 32 T2N R3E							
23a	261+31.78 to 264+62.14	R	Temp	0.13 Ac 5746 Sq Ft	LOREN EGGERT	LOT 4 REYNOLDS STAGE STOP SUBD SEC 32 T2N R3E							
24	Plat			Combined with Parcel 22	LOREN EGGERT	Lot 4 of Reynolds Stage Stop Subdivision, in the SE1/4 of Section 31 and in the SW1/4 of SW1/4 of Section 32, Township 2 North, Range 3 East							
24a	261+89.18 to 264+62.02	R	Temp	25 Sq Ft	JEFFERY MUNSON TRUST	North, Range 3 East LOT 5 SEVEN HILLS SUBD S32 T2N R3E A portion of SW1/4, SE1/4, NW1/4, NE1/4 SEC 32 T2N R3E ALL LESS PT MS #1506, LESS PT MS #1808 SEC 32 T2N R3E							
25	Plat			Combined with Parcel 23	USA PUBLIC DOMAIN (UNPATENTED)	LOT 5 SEVEN HILLS SUBD S32 T2N R3E A portion of SW1/4, SE1/4, NW1/4, NE1/4 SEC 32 T2N R3E ALL LESS PT MS #1506, LESS PT MS #1808 SEC 32 T2N R3E							
25a	276+07.45 to 279+33.56	L	Temp	0.47 Ac 20432 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	ALL LESS PT MS #1506, LESS PT MS #1808 SEC 32 T2N R3E							
		1	1	0.00.40		 							

ALL LESS PT MS #1506, LESS PT MS #1808 SEC 32 T2N R3E

USA PUBLIC DOMAIN (UNPATENTED)

281+88.46 to 284+16.67

0.09 Ac

4109 Sq Ft

25b

STATE OF SOUTH DAKOTA P 6403(10) 88

Plotting Date:

Parcel	Station to Station	Side	Type	Area	Owner	Description
26	Plat			0.01 Ac 517 Sq Ft	JEFFERY MUNSON TRUST	LOT 5 SEVEN HILLS SUBD S32 T2N R3E
26a	260+25 to 262+00	R	Temp	0.06 Ac 2757 Sq Ft	JEFFERY MUNSON TRUST	LOT 5 SEVEN HILLS SUBD S32 T2N R3E
26b	262+00 to 263+75	R	Temp	0.056 Ac 2420 Sq Ft	JEFFERY MUNSON TRUST	LOT 5 SEVEN HILLS SUBD S32 T2N R3E
27a	275+58.19 to 276+66.19	R	Temp	0.03 Ac 1161 Sq Ft	GABRIEL SHAGLA	LOT 4 SEVEN HILLS SUBD S32 T2N R3E
28	Plat			0.01 Ac 461 SqFt	GABRIEL SHAGLA	LOT 4 SEVEN HILLS SUBD S32 T2N R3E
29a	291+94.11 to 292+31.56	L	Temp	0.03 Ac 1126 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	ALL LESS PT MS #1506, LESS PT MS #1808 S32 T2N R3E
29b	296+46.08 to 297+42.05	L	Temp	0.02 Ac 1028 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	ALL LESS PT MS #1506, LESS PT MS #1808 S32 T2N R3E
30	Plat			Combined with Parcel 23	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4, SE1/4, NW1/4, NE1/4 SEC 32 T2N R3E
30a	299+35.99 to 300+85.62	R	Temp	0.07 Ac 2949 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4, SE1/4, NW1/4, NE1/4 SEC 32 T2N R3E
31	Plat			Combined with Parcel 23	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4, SE1/4, NW1/4, NE1/4 SEC 32 T2N R3E
32	Plat			Combined with Parcel 23	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4, SE1/4, NW1/4, NE1/4 SEC 32 T2N R3E
33	Plat			4.41 Ac 192276 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 S29 T2N R3E
34	Plat			Combined with Paracel 33	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 S29 T2N R3E
35	Plat			22.90 Ac 997615 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4, SE1/4, NE1/4, S28 T2N R3E
36	Plat			Combined with Parcel 35	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4, SE1/4, NE1/4, S28 T2N R3E
37	Plat			Combined with Parcel 35	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4, SE1/4, NE1/4, S28 T2N R3E
39b	411+60.04 to 412+11.25	R	Temp	0.02 Ac 720 Sq Ft	JAMES HOFFMAN	TRACT D OF NW1/4SW1/4 LESS CO RD ROW ROW S27 T2N R3E
41	Plat			Combined with Parcel 35	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4, SE1/4, NE1/4, S28 T2N R3E
42a	414+28.79 to 415+31.22	R	Temp	0.10Ac 4,205 Sq Ft	WAMATTS LLC	TRACT E OF NW1/4SW1/4 LESS CO RD ROW SEC 27 T2N R3E
45	Plat			136 Sq Ft	WAMATTS LLC	NW1/4 SW1/4 SEC 27, T2N, R3E
46	Plat			104 Sq Ft	WAMATTS LLC	NW1/4 SW1/4 SEC 27, T2N, R3E
49	Plat			.03 Ac 1101 Sq Ft	MARSHA MCCLAIN	W1/2 SW1/4 NW1/4 S27, T2N, R3E
50a	420+30.22 to 423+19.15	L	Temp	0.11 Ac 4998 Sq Ft	MARSHA MCCLAIN	W1/2 SW1/4 NW1/4 S27, T2N, R3E W1/2SW1/4NW1/4 LESS CO RD ROW S27 T2N R3E E1/2SW1/4NW1/4 AND W20 ft OF C S LANE (A PRIVATE DRIVE EASEMENT LOCATED IN THE E1/2SW1/4NW1 CO RD ROW S27 T2N R3E
50b	423+19.15 to 423+34.21	L	Temp	164 Sq Ft	SUSAN SCHWANEKE/POTRATZ/FALSEY	E1/2SW1/4NW1/4 AND W20 ft OF C S LANE (A PRIVATE DRIVE EASEMENT LOCATED IN THE E1/2SW1/4NW1 CO RD ROW S27 T2N R3E

W1/2 SW1/4 NW1/4 S27, T2N, R3E

MARSHA MCCLAIN

0.37 Ac

16082 Sq Ft

STATE OF SOUTH DAKOTA P 6403(10) 89 333

Plotting Date: 2/6/20

Parcel	Station to Station	Side	Туре	Area	Owner	Description
51a	423+95.74 to 425+22.32	R	Temp	0.05 Ac 2001 Sq Ft	SUSAN SCHWANEKE/POTRATZ/FALSEY	E1/2SW1/4NW1/4 AND W20 ft OF C S LANE (A PRIVATE DRIVE EASEMENT LOCATED IN THE E1/2SW1/4NW1/4) LESS CO RD ROW S27 T2N R3E
51b	424+59.82 to 425+03.35	L	Temp	0.02 Ac 683 Sq Ft	SUSAN SCHWANEKE/POTRATZ/FALSEY	E1/2SW1/4NW1/4 AND W20 ft OF C S LANE (A PRIVATE DRIVE EASEMENT LOCATED IN THE E1/2SW1/4NW1/4) LESS CO RD ROW S27 T2N R3E
51c	425+84.77 to 431+87.48	L	Temp	0.32 Ac 13845 Sq Ft	SUSAN SCHWANEKE/POTRATZ/FALSEY	E1/2SW1/4NW1/4 AND W20 ft OF C S LANE (A PRIVATE DRIVE EASEMENT LOCATED IN THE E1/2SW1/4NW1/4) LESS CO RD ROW S27 T2N R3E
53	Plat				SUSAN SCHWANEKE/POTRATZ/FALSEY	E1/2 SW1/4 NW1/4 S27, T2N, R3E
53a	430+29.25 to 430+66.26	R	Temp	0.01 Ac 256 Sq Ft	SUSAN SCHWANEKE/POTRATZ/FALSEY	E1/2SW1/4NW1/4 AND W20 ft OF C S LANE (A PRIVATE DRIVE EASEMENT LOCATED IN THE E1/2SW1/4NW1/4) LESS CO RD ROW S27 T2N R3E
53b	430+86.07 to 432+03.59	R	Temp	0.02 Ac 843 Sq Ft	BEVERLY HOWEY TRUST	NE1/4NW1/4SW1/4; NW1/4SE1/4NW1/4 LESS CO RD ROW; E20 ft OF C S LANE (A PRIVATE DR EASEMENT LOCATED IN THE NW1/4SE1/4NW1/4) S27 T2N R3E
53c	431+87.48 to 433+29.17	L	Temp	0.02 Ac 950 Sq Ft	BEVERLY HOWEY TRUST	NE1/4NW1/4SW1/4; NW1/4SE1/4NW1/4 LESS CO RD ROW; E20 ft OF C S LANE (A PRIVATE DR EASEMENT LOCATED IN THE NW1/4SE1/4NW1/4) S27 T2N R3E
54	Plat			0.18 Ac 7993 Sq Ft	BEVERLY HOWEY TRUST	NW1/4 SE1/4 NW1/4 S27, T2N, R3E
55a	435+83.72 to 436+38.06	L	Temp	0.02 Ac 843 Sq Ft	BEVERLY HOWEY TRUST	NE1/4NW1/4SW1/4; NW1/4SE1/4NW1/4 LESS CO RD ROW; E20 ft OF C S LANE (A PRIVATE DR EASEMENT LOCATED IN THE NW1/4SE1/4NW1/4) S27 T2N R3E
55b	437+32.50 to 438+52.13	L	Temp	0.10Ac 4141 Sq Ft	BEVERLY HOWEY TRUST	NE1/4NW1/4SW1/4; NW1/4SE1/4NW1/4 LESS CO RD ROW; E20 ft OF C S LANE (A PRIVATE DR EASEMENT LOCATED IN THE NW1/4SE1/4NW1/4) S27 T2N R3E
55c	438+52.13 to 445+31.77	L	Temp	0.66 Ac 28579 Sq Ft	ALAN HARVEY/BRYAN HARVEY/JUDITH BELCHER	E1/2 SE1/4 NW1/4 LESS CO RD ROW S27, T2N, R3E
56	Plat			0.05 Ac 1960 Sq Ft	ALAN HARVEY/BRYAN HARVEY/JUDITH BELCHER	E1/2 SE1/4 NW1/4 S27, T2N, R3E
57	Plat			37 Sq Ft	ALAN HARVEY/BRYAN HARVEY/JUDITH BELCHER	E1/2 SE1/4 NW1/4 S27, T2N, R3E
57a	Musick Drive Alignment			0.86 Ac 37648 SqFt	WADE MUSICK	W1/2 SW1/4 NE1/4 S27, T2N, R3E
58	Plat			1.24 Ac 54131 SqFt	WADE MUSICK	W1/2 SW1/4 NE1/4 S27, T2N, R3E
60	Plat			0.05 Ac 2139 Sq Ft	JOHNNY & MAUD HOPKINS/KIMBERLY GRIFFITH	NE1/4 SW1/4 NE1/4 S27, T2N, R3E
61	Plat			0.06 Ac 2464 Sq Ft	KEVIN & DIANN KILLION	NE1/4 SW1/4 NE1/4 S27, T2N, R3E
61a & b	452+75.71 to 453+59.67	L	Temp	0.394Ac 17179 Sq Ft	KEVIN & DIANN KILLION	THAT PT OF NE1/4SW1/4NE1/4 LYING NORTH OF ROCHFORD RD S27 T2N R3E
61c	455+18.85 to 455+53.92	R	Temp	0.01 Ac 359 Sq Ft	JOHNNY & MAUD HOPKINS/KIMBERLY GRIFFITH	THAT PT OF NE1/4SW1/4NE1/4 LYING SOUTH OF ROCHFORD RD S27 T2N R3E
62	Plat			6.67 Ac 290613 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of NE1/4 S27, T2N, R3E
62a	468+35.26 to 469+81.41	L	Temp	0.02 Ac 916 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	E1/2NE1/4; NW1/4NE1/4; N1/2NW1/4; E1/2SW1/4; SW1/4SW1/4; SE1/4 LESS PT MS #1620 LESS N1/2NE1/4NW1/4 S27 T2N R3E
62b	470+58.70 to 476+04.47	L	Temp	0.23 Ac 9889 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	E1/2NE1/4; NW1/4NE1/4; N1/2NW1/4; E1/2SW1/4; SW1/4SW1/4; SE1/4 LESS PT MS #1620 LESS N1/2NE1/4NW1/4 S27 T2N R3E
63	Plat			Combined with Parcel 62	USA PUBLIC DOMAIN (UNPATENTED)	A portion of NE1/4 S27, T2N, R3E
64	Plat			0.25 Ac 10751 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of NE1/4 S27, T2N, R3E A portion of NW1/4 S26, T2N, R3E
						<u> </u>

USA PUBLIC DOMAIN (UNPATENTED)

475+04.12 to 477+27.97

0.07 Ac

3031 Sq Ft

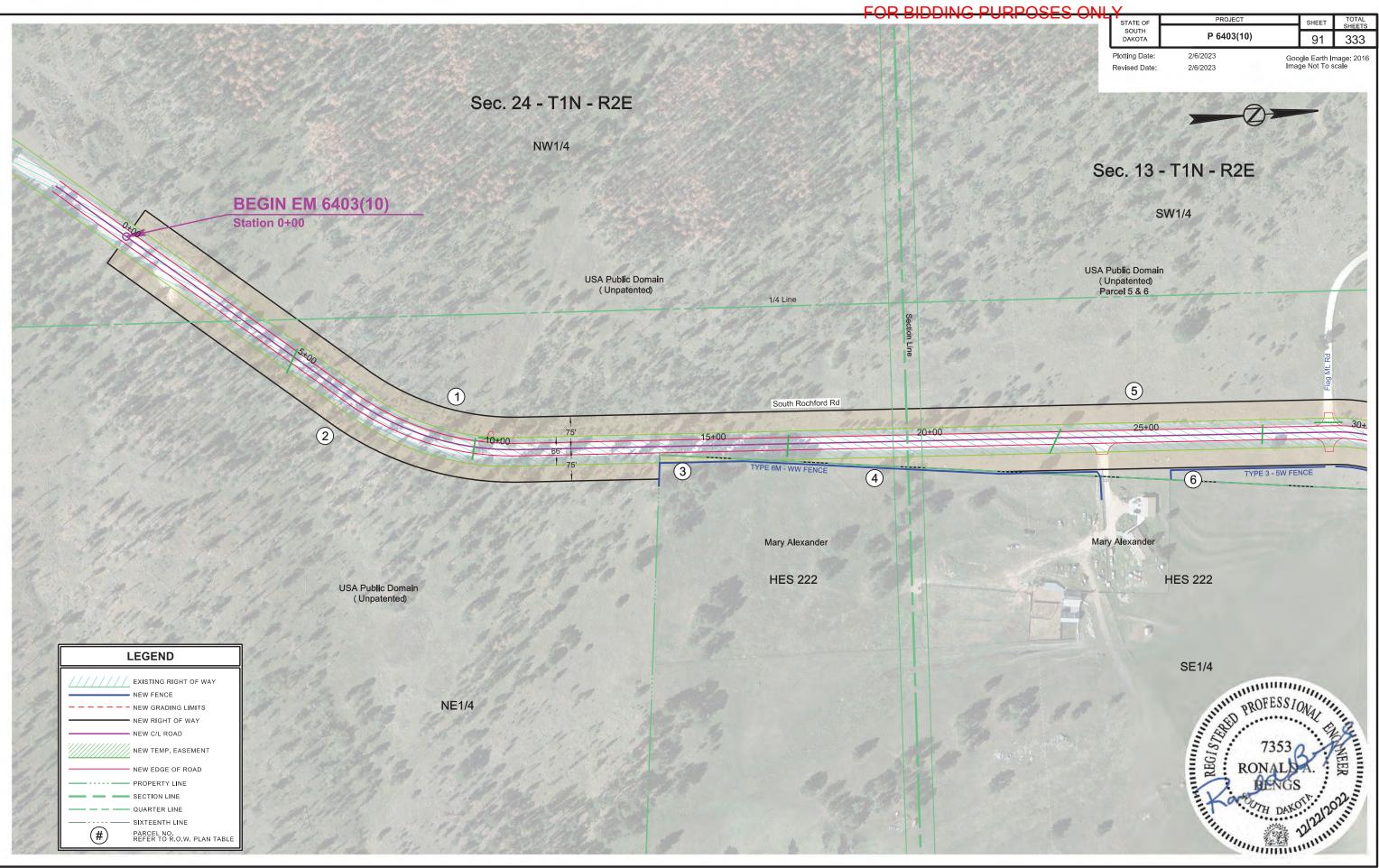
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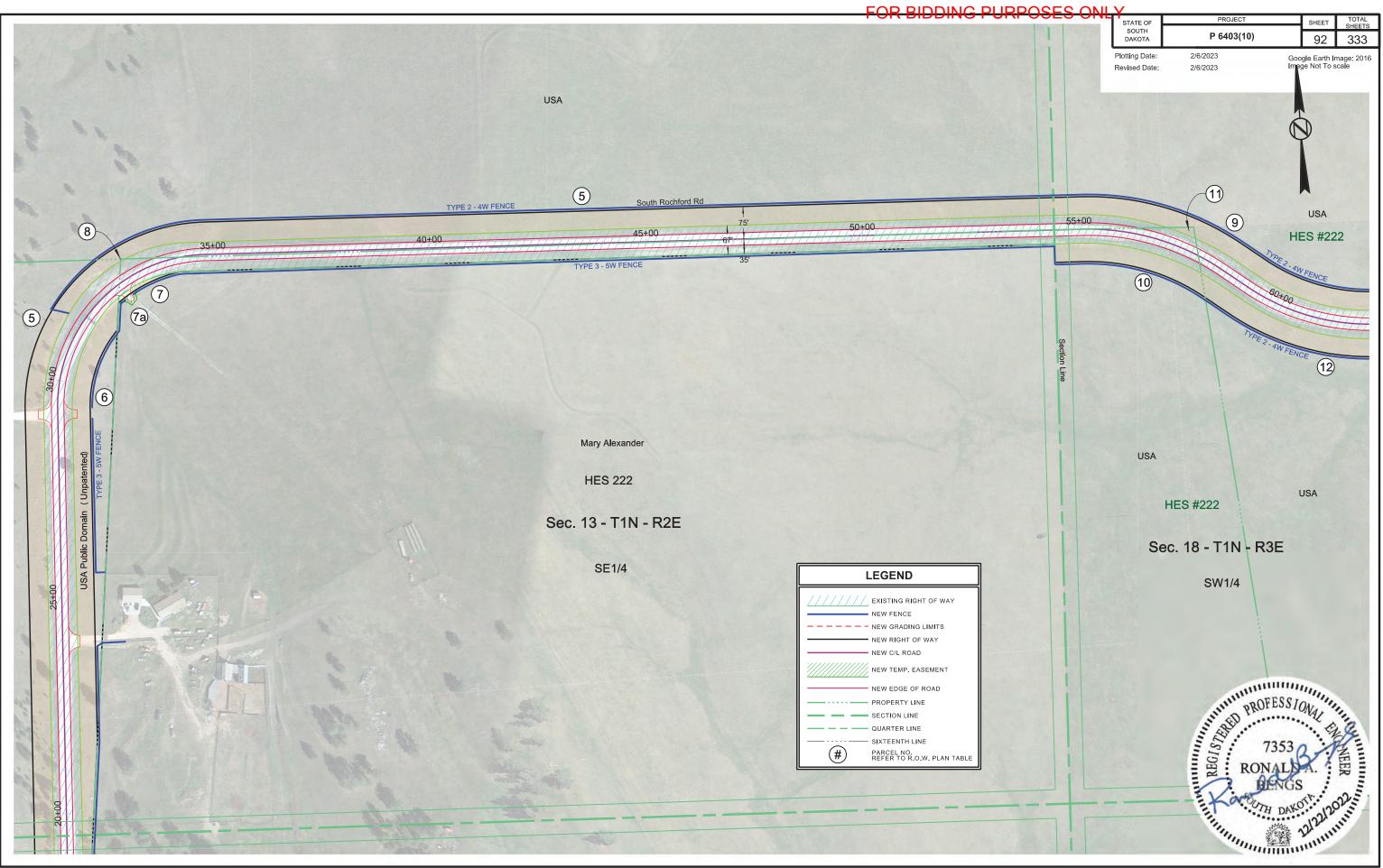
STATE OF SOUTH DAKOTA P 6403(10) 90 333

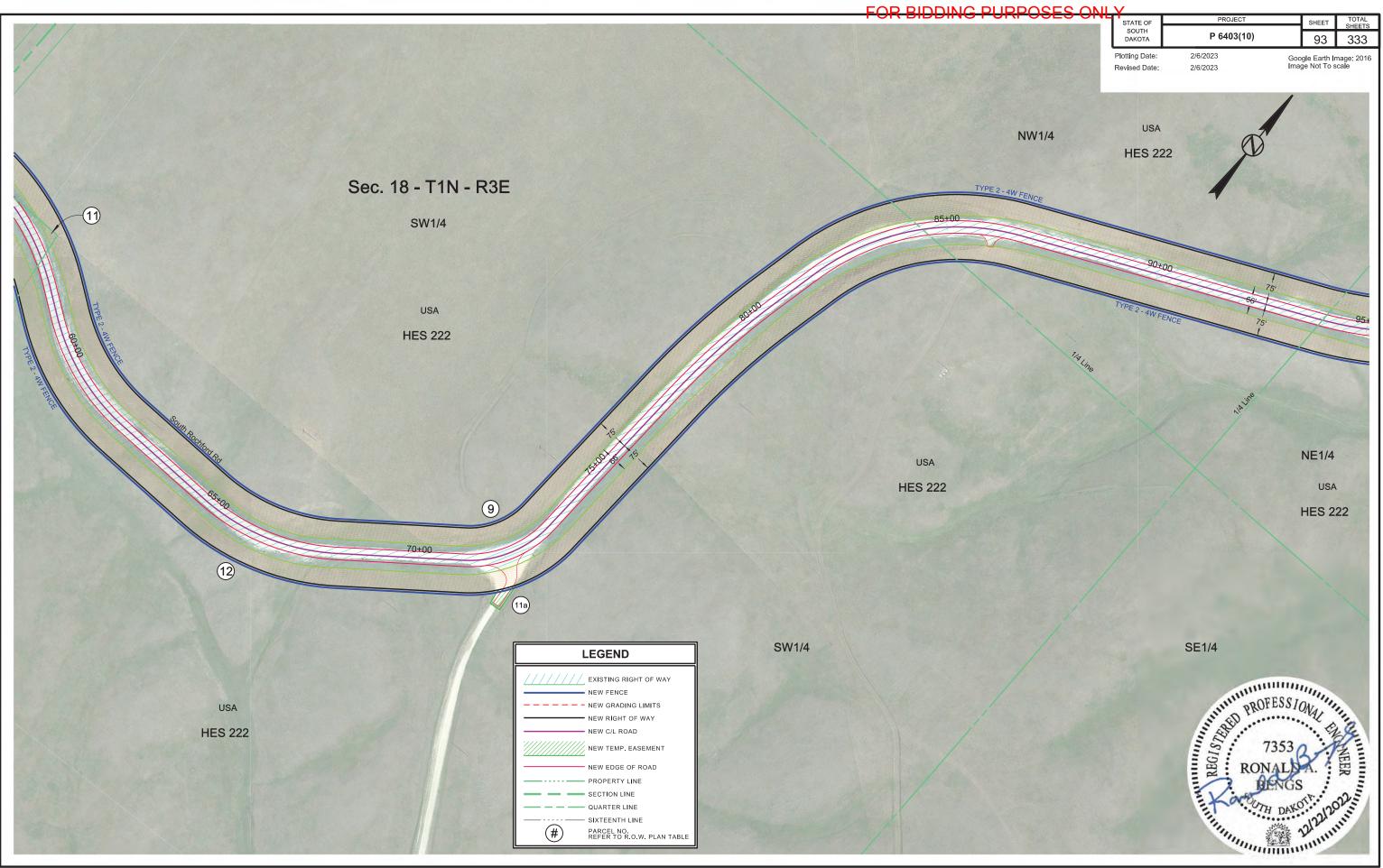
Plotting Date: 2/6/2023

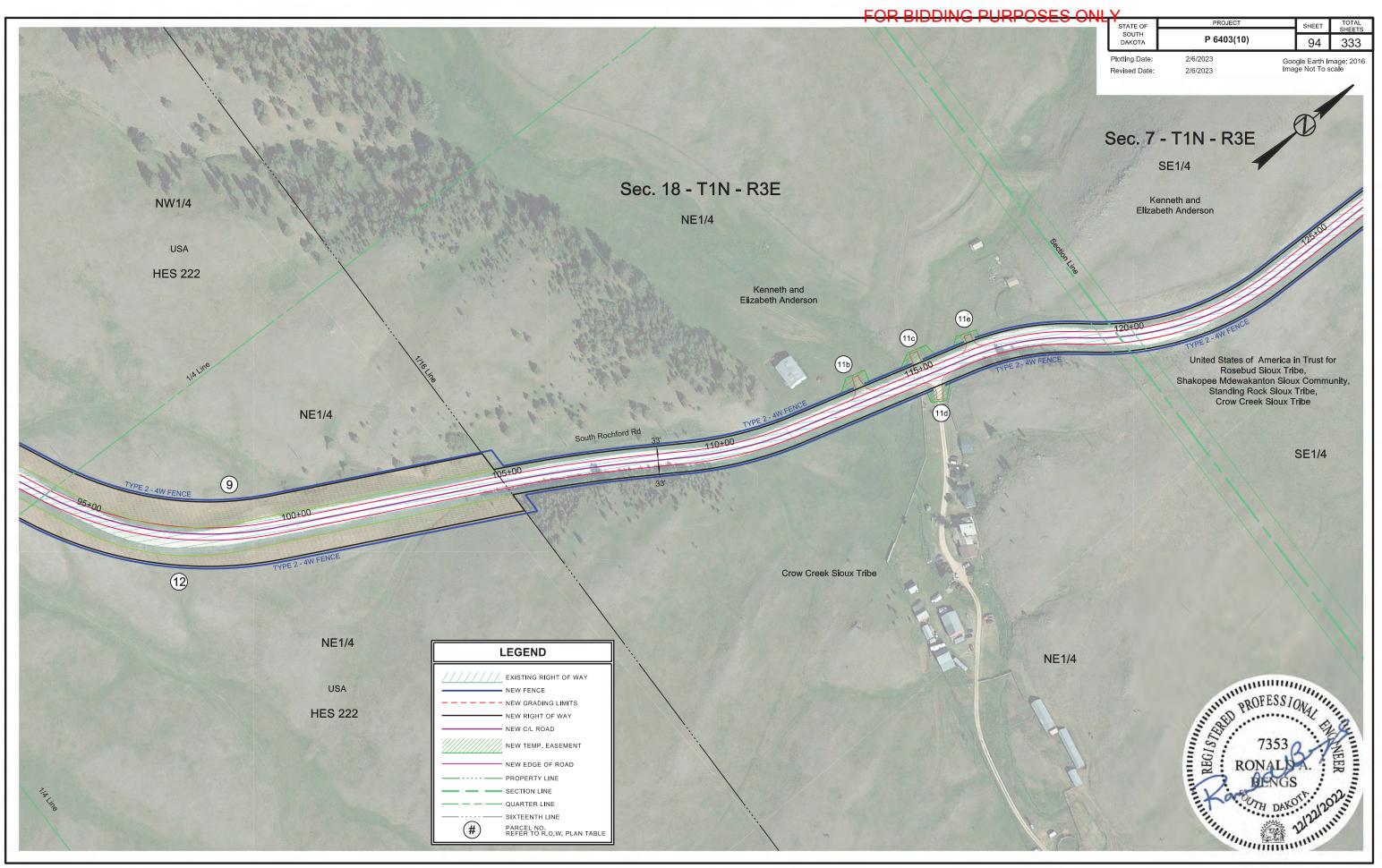
Parcel	Station to Station	Side	Туре	Area	Owner	Description
65	Plat			Combined with Parcel 64	USA PUBLIC DOMAIN (UNPATENTED)	A portion of NW1/4 S26, T2N, R3E
66	Plat			Combined with Parcel 62	USA PUBLIC DOMAIN (UNPATENTED)	A portion of NE1/4 S27, T2N, R3E
67	Plat			10.46 Ac 455551 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 S22, T2N, R3E
68	Plat			Combined with Parcel 67	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 S22, T2N, R3E
69	Plat			7.12 Ac 310214 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4 S23, T2N, R3E
70	Plat			Combined with Parcel 69	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SW1/4 S23, T2N, R3E
71	Plat			Combined with Parcel 67	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 NE1/4 S22, T2N, R3E
71a	505+18.18 to 507+41.95	L	Temp	0.16 Ac 6924 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	ALL LESS S1/2SE1/4SW1/4 S22 T2N R3E
71b	508+08.22 to 508+59.39	L	Temp	0.01 Ac 223 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	ALL LESS S1/2SE1/4SW1/4 S22 T2N R3E
71c	522+66.16 to 523+05.1	L	Temp	0.01 Ac 517 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	ALL LESS S1/2SE1/4SW1/4 S22 T2N R3E
72	Plat			Combined with Paracel 67	USA PUBLIC DOMAIN (UNPATENTED)	A portion of SE1/4 NE1/4 S22, T2N, R3E
77a	522+17.75 to 524+25.25	R	Temp	0.02 Ac 898 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	BLL ON SE1/4NE1/4 S23 T2N R3E
78a	523+05.10 to 524+56.92	L	Temp	0.21 Ac 9045 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	BLL ON SE1/4NE1/4 S23 T2N R3E
82a	530+70.71 to 531+10.73	R	Temp	0.01 Ac 241 Sq Ft	USA PUBLIC DOMAIN (UNPATENTED)	BLL ON SE1/4NE1/4 S23 T2N R3E
82b	531+10.73 to 535+56.15	R	Temp	0.19 Ac 8097 Sq Ft	Fred & Jan Kromer	KERN LODE MS 974 S23 T2N R3E
82c	531+74.80 to 536+07.05	L	Temp	0.21 Ac 9177 Sq Ft	Fred & Jan Kromer	KERN LODE MS 974 S23 T2N R3E

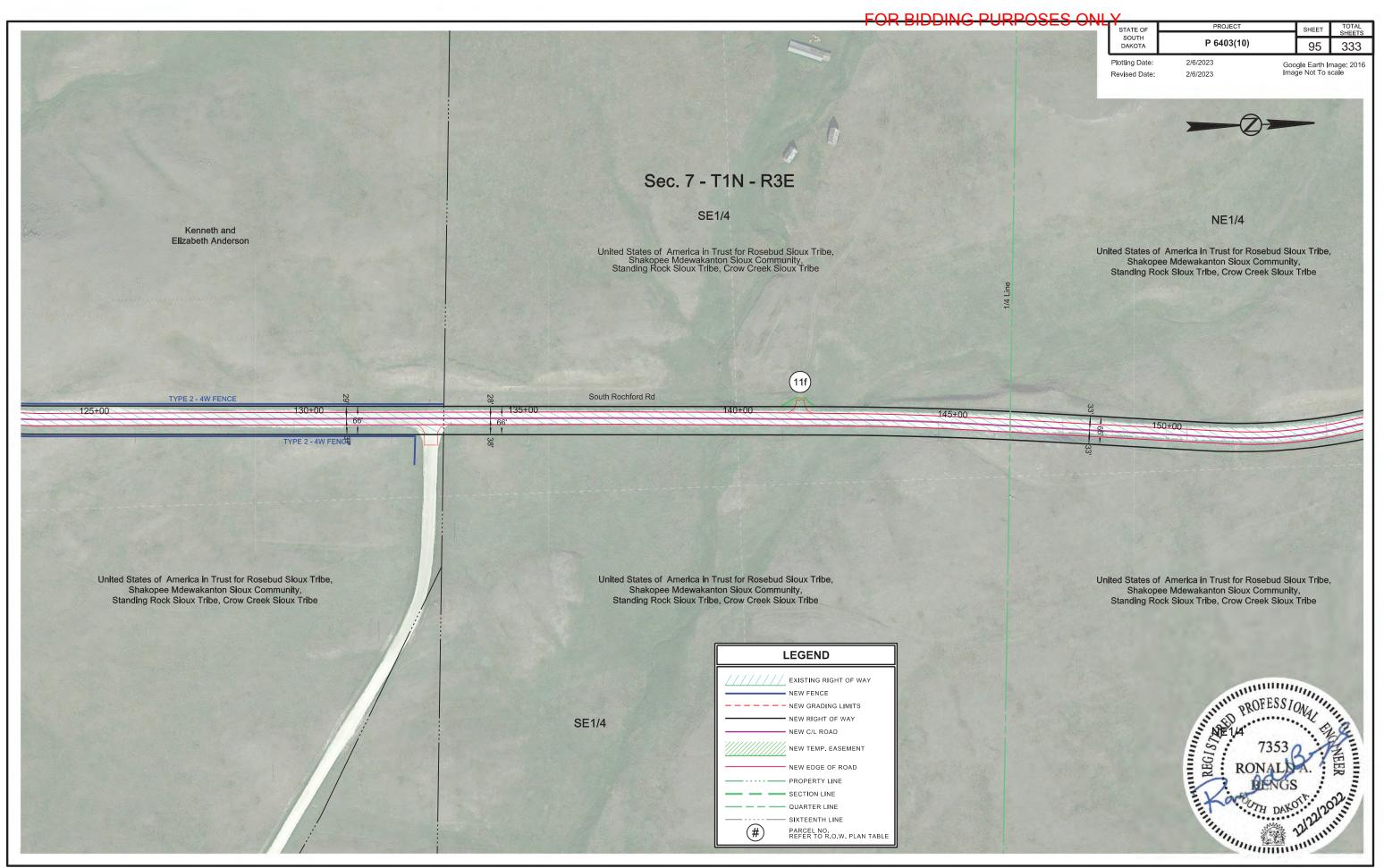


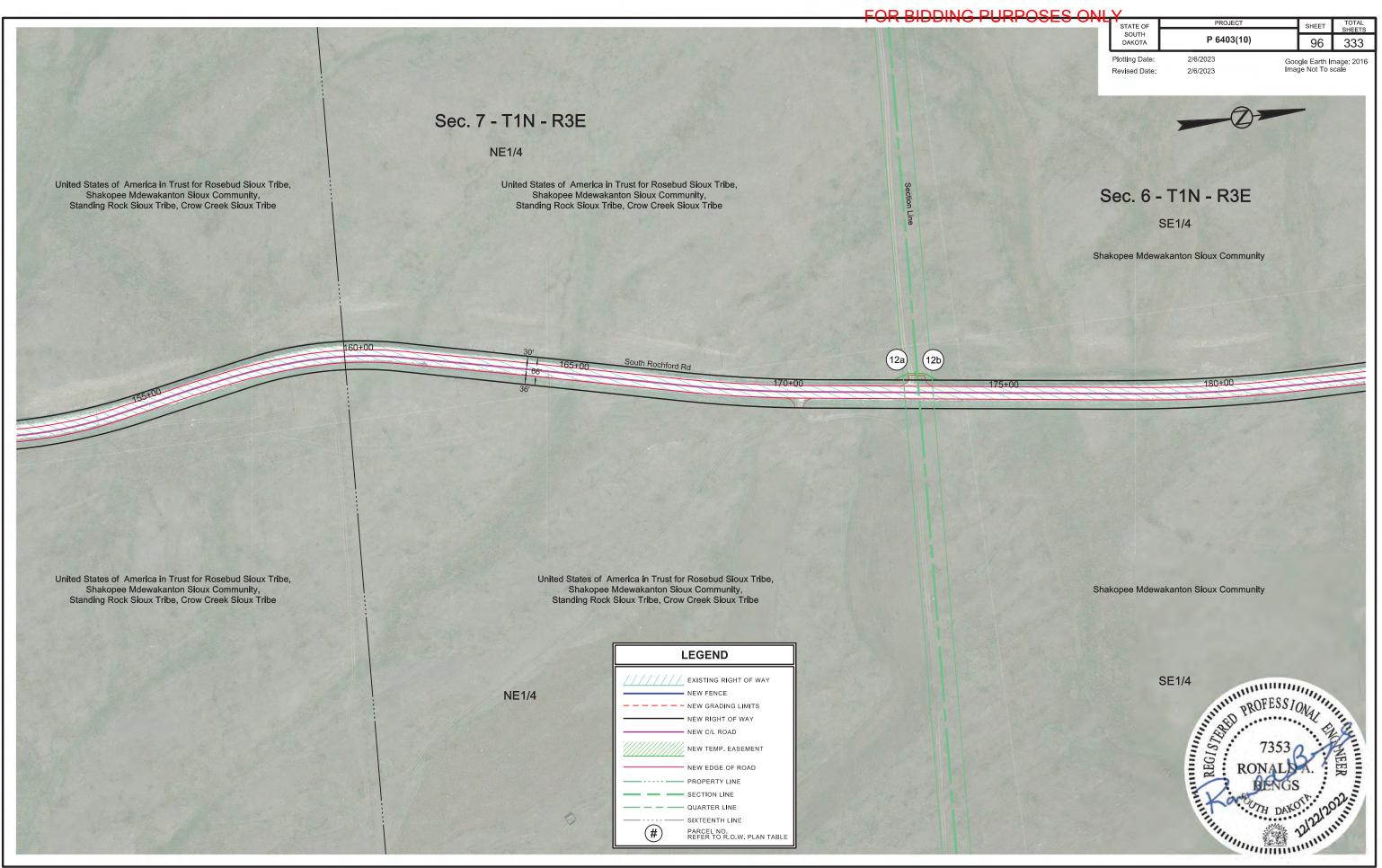


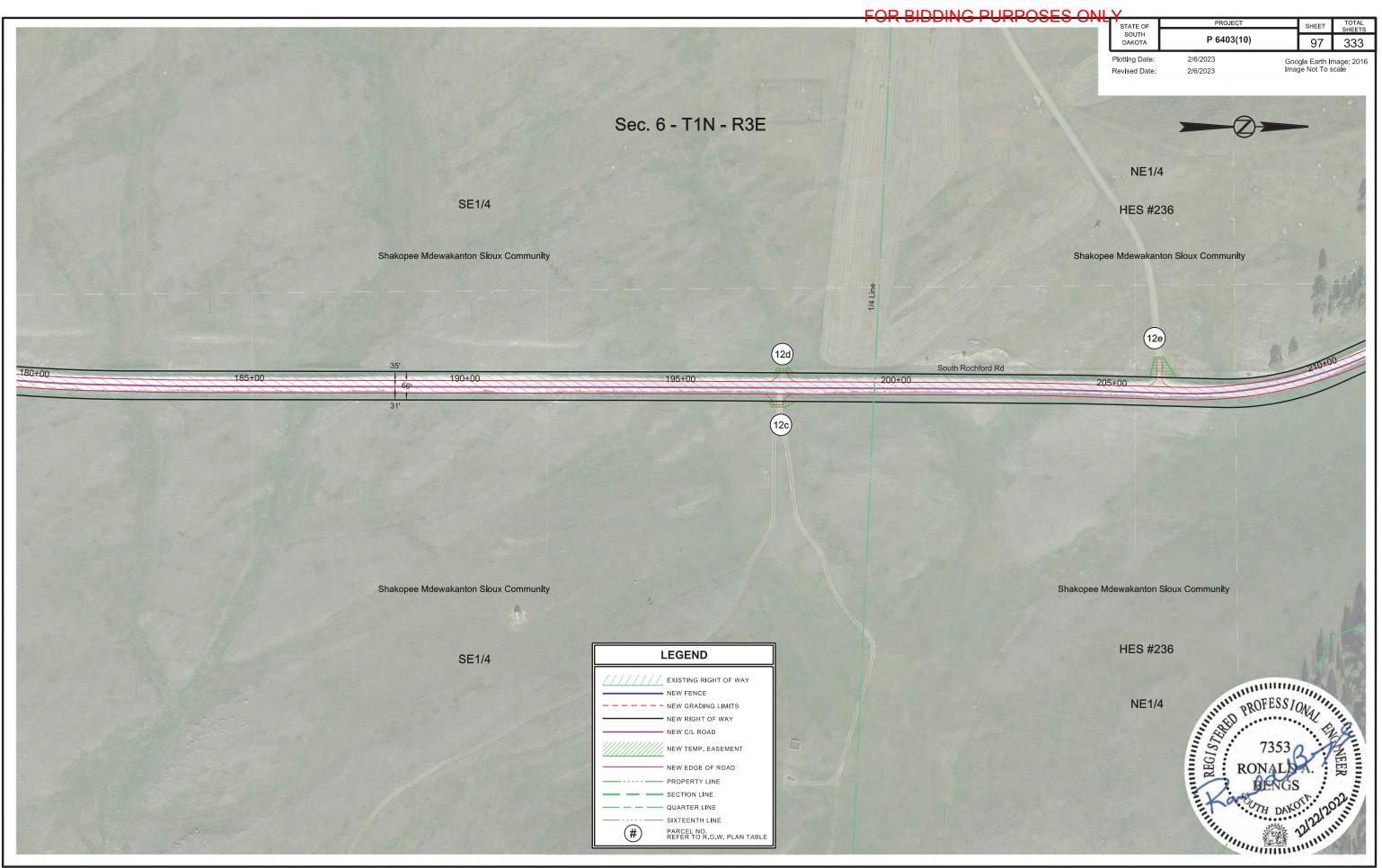


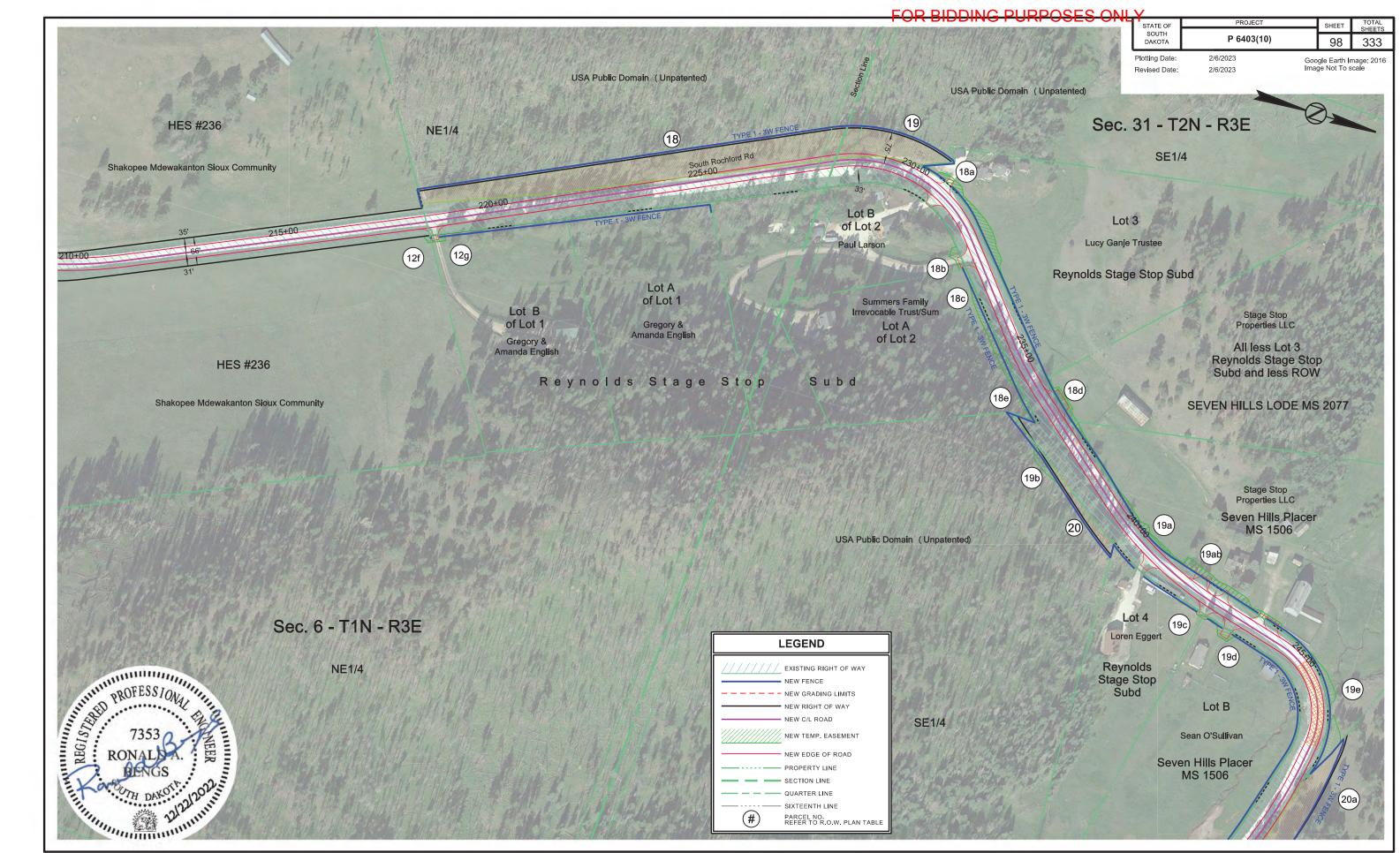


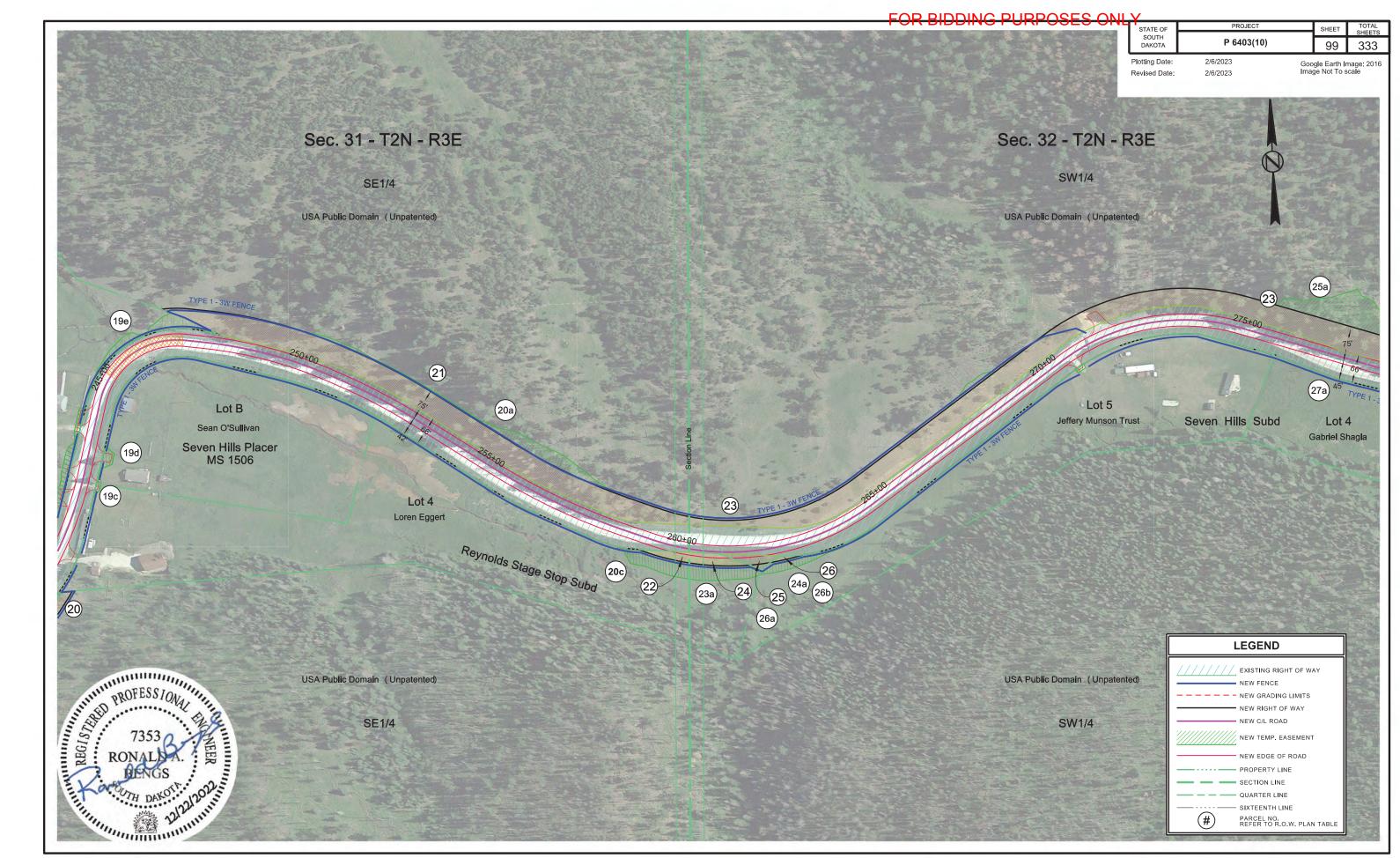


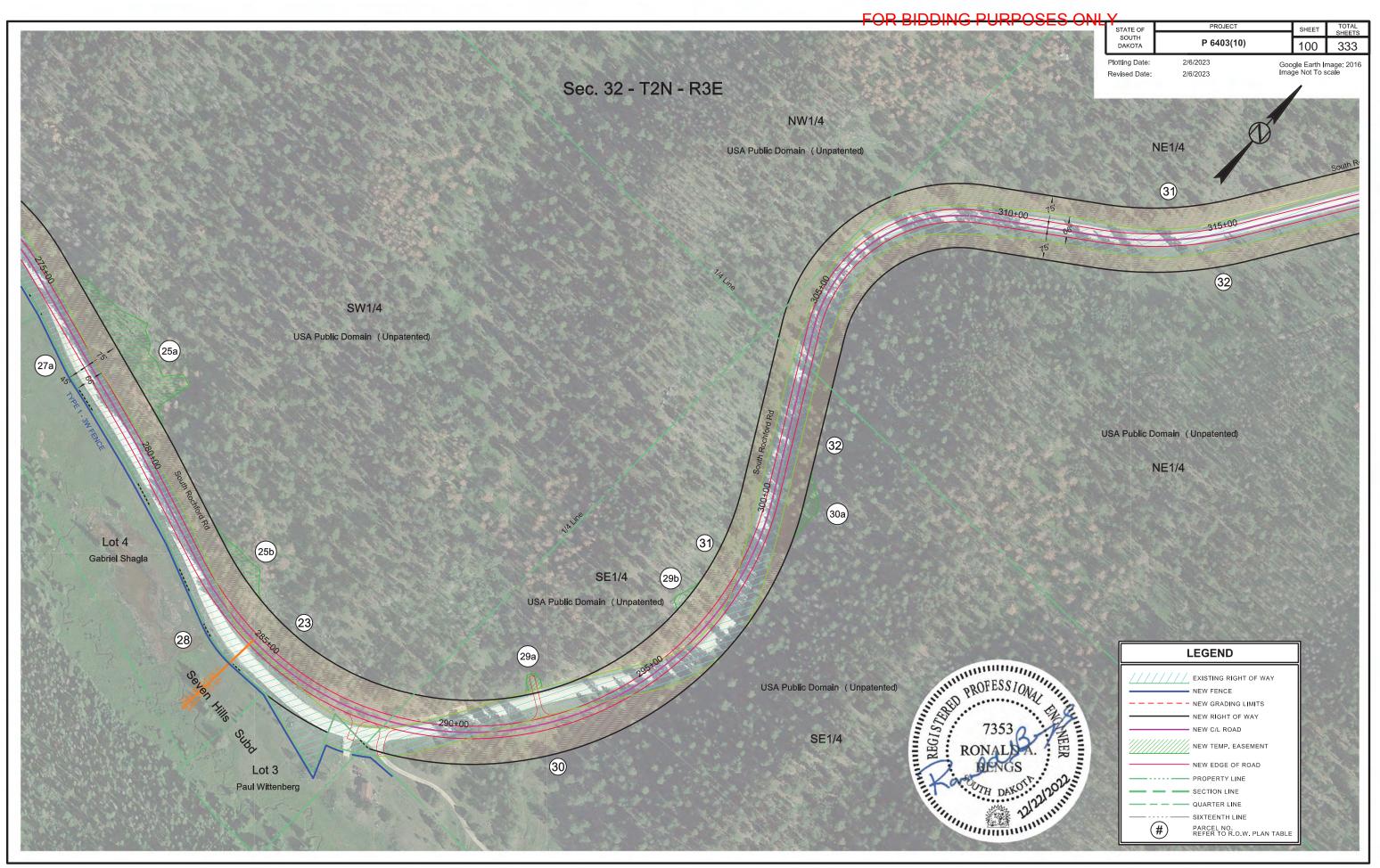


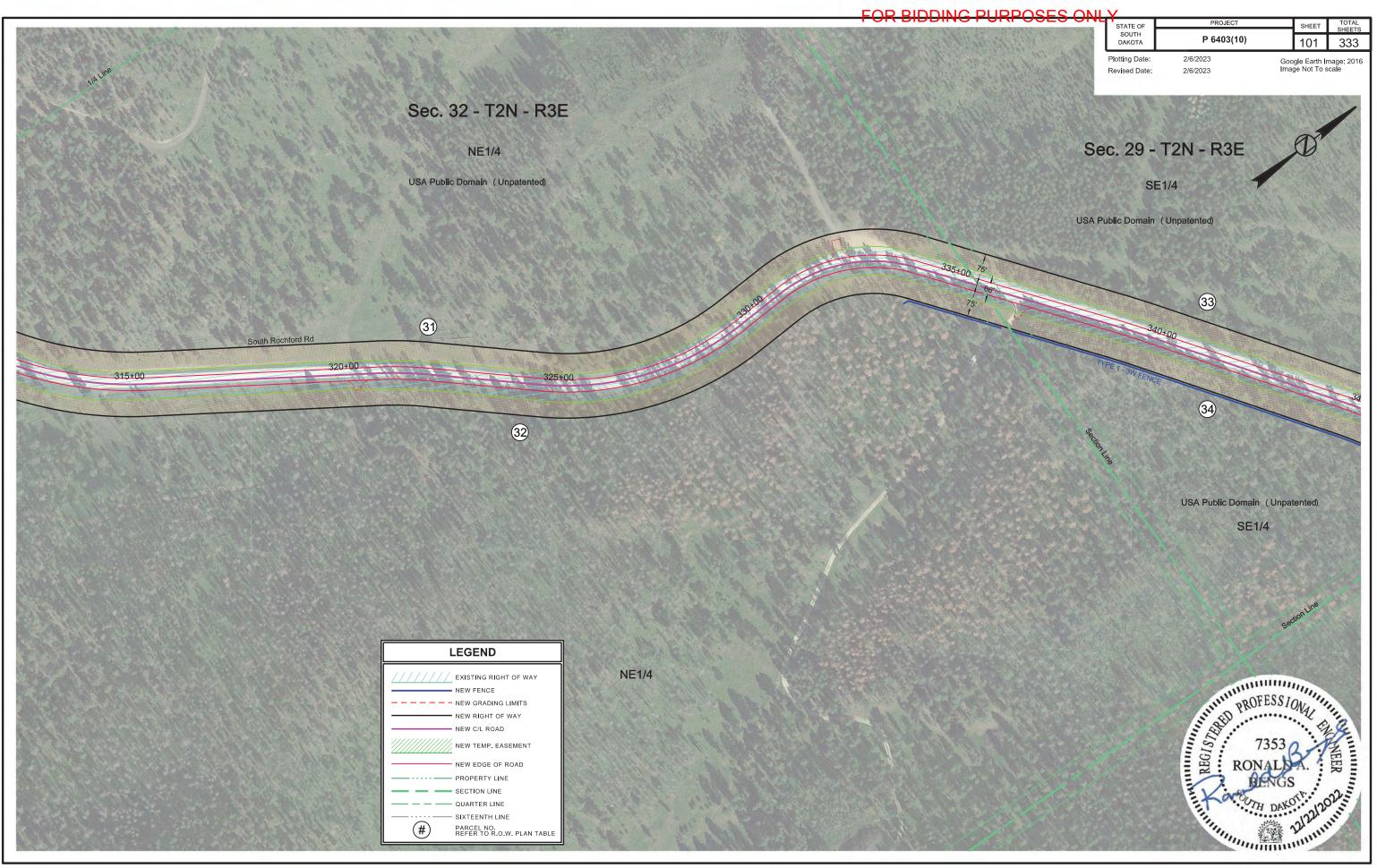


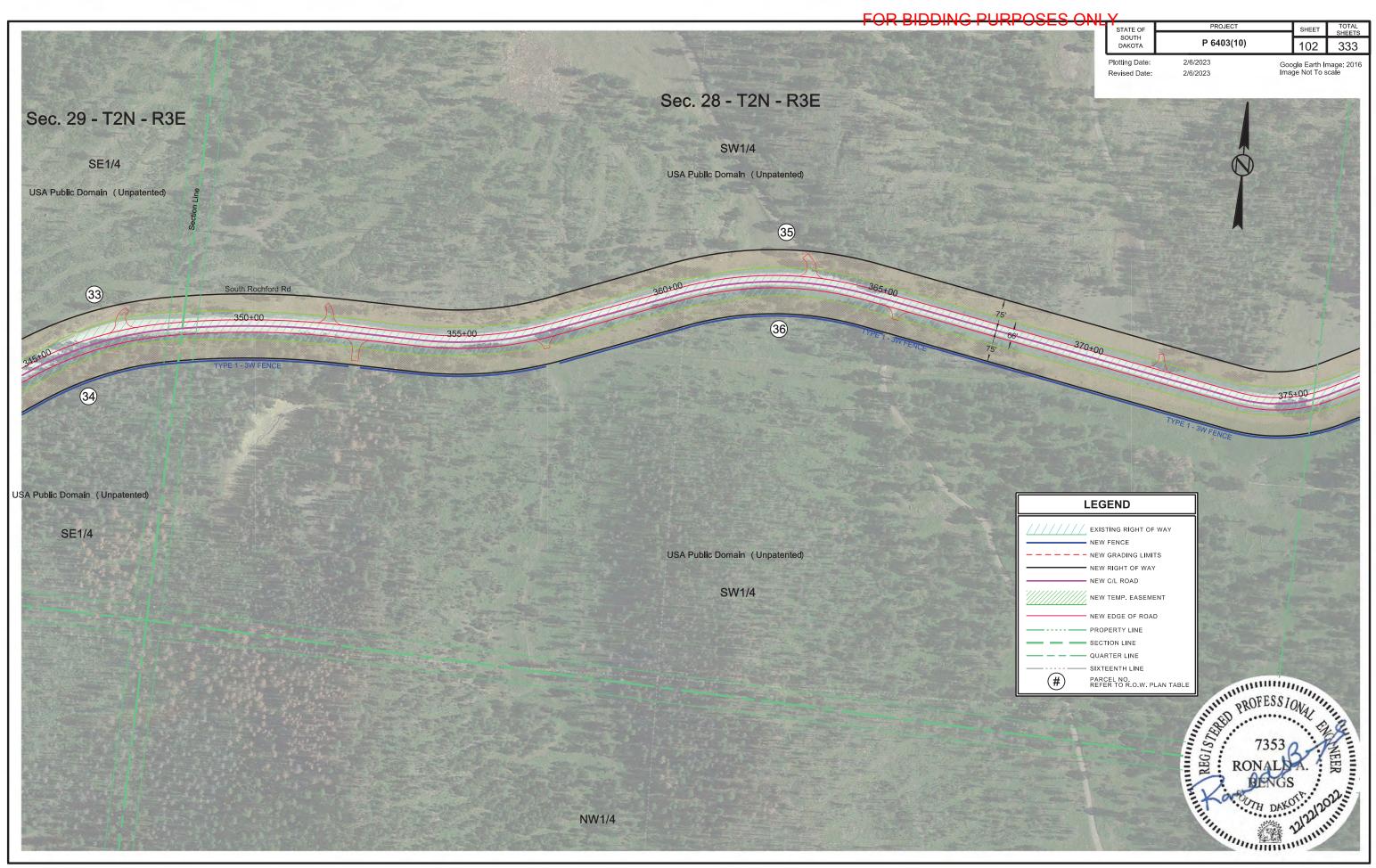


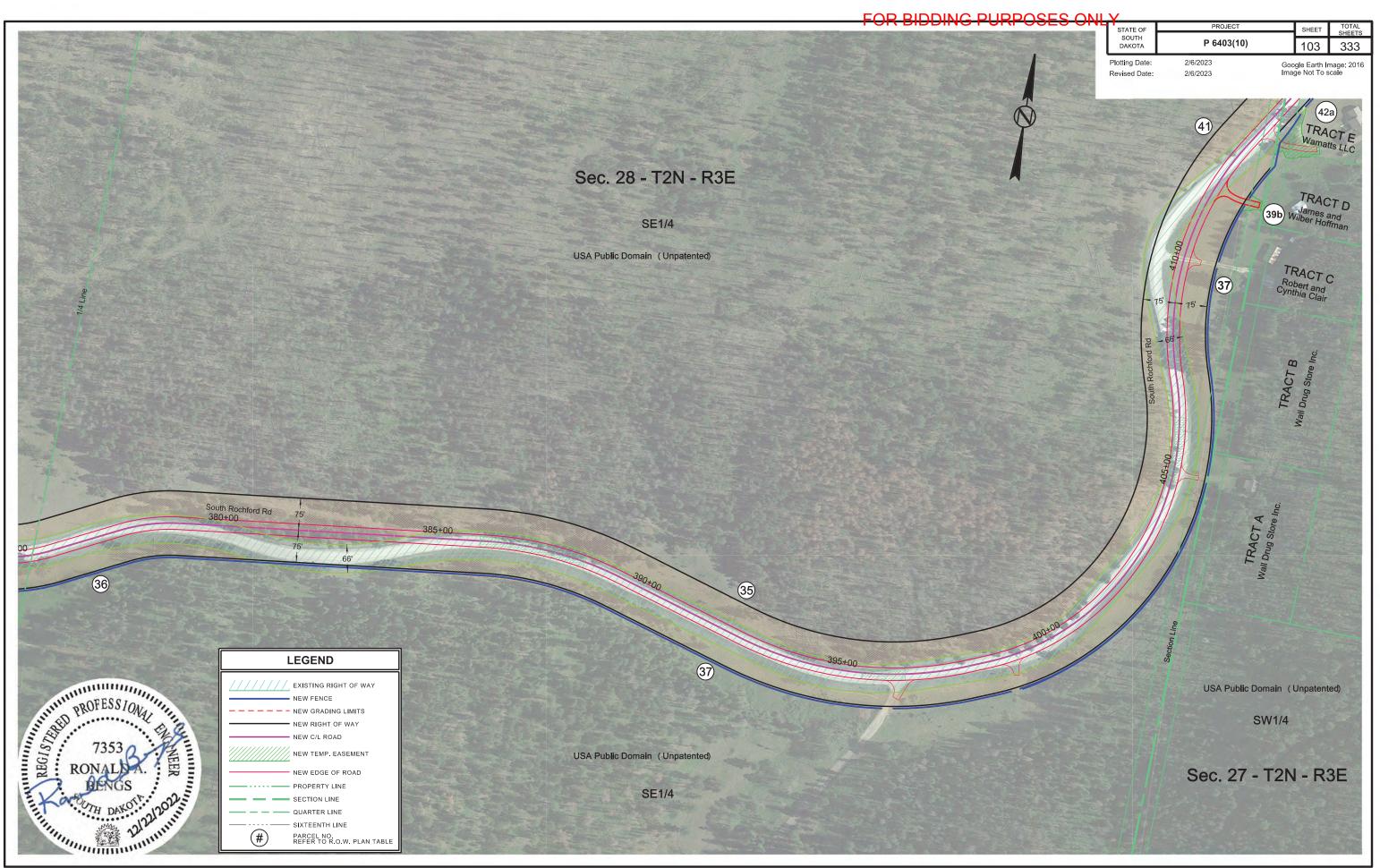


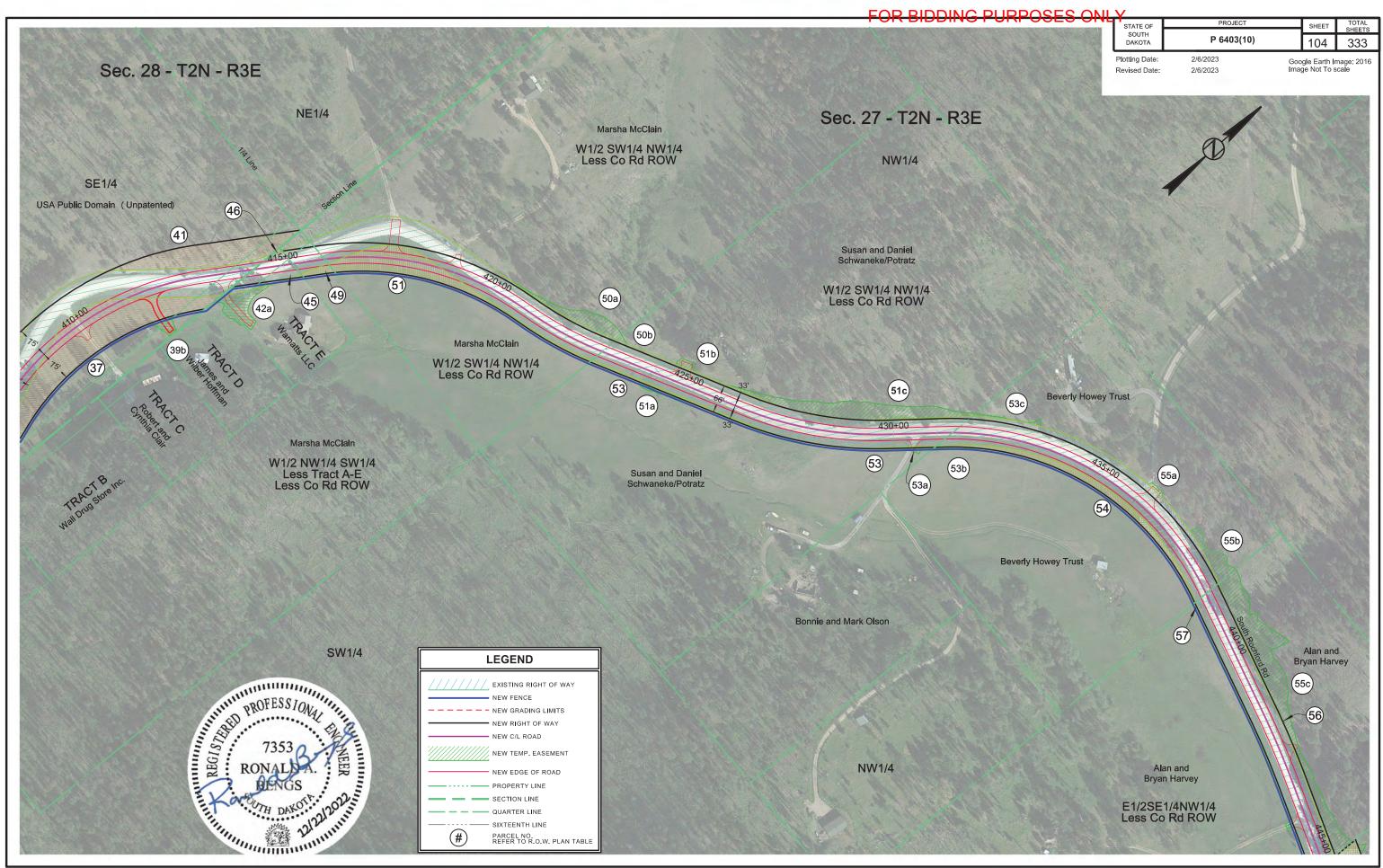


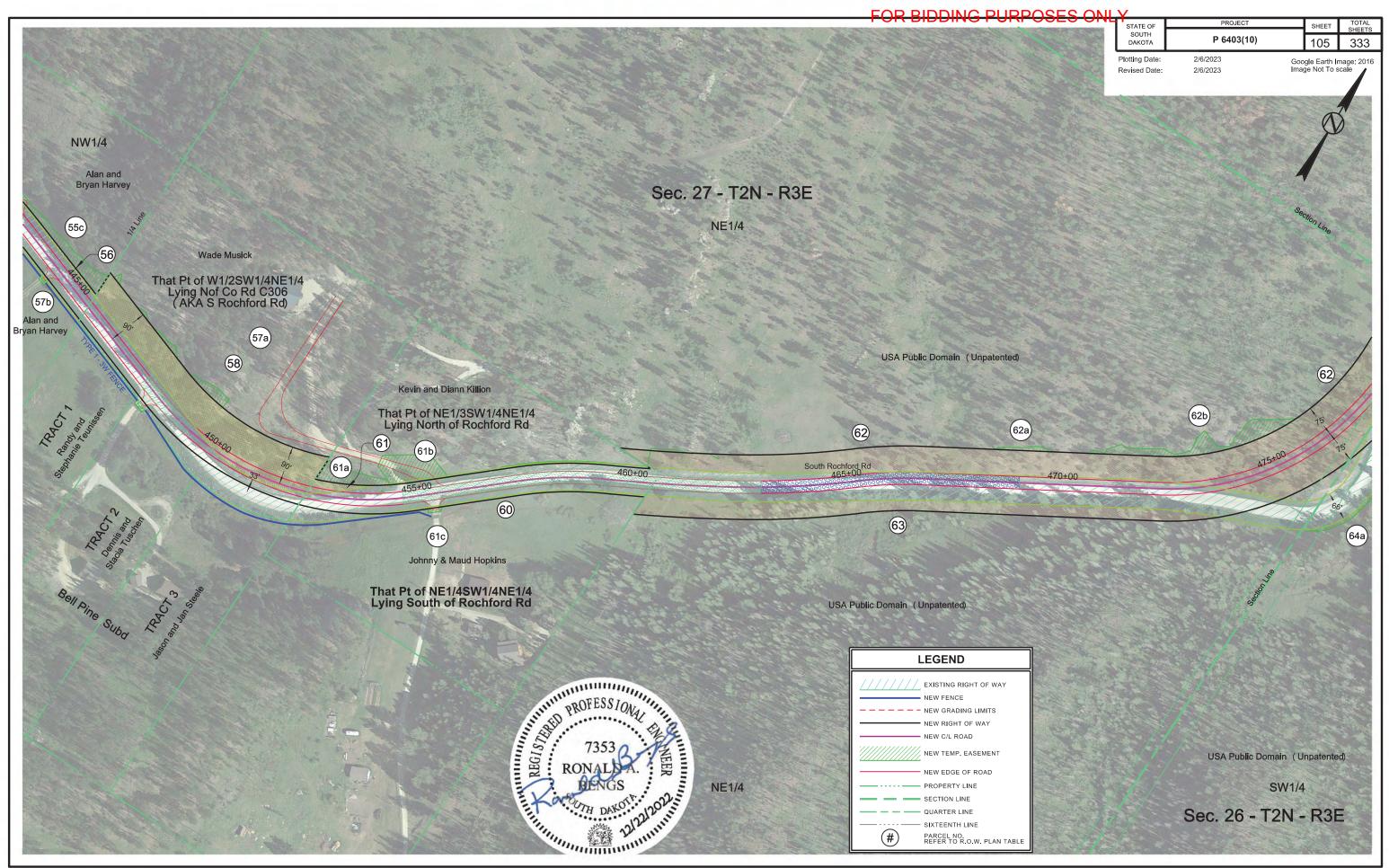


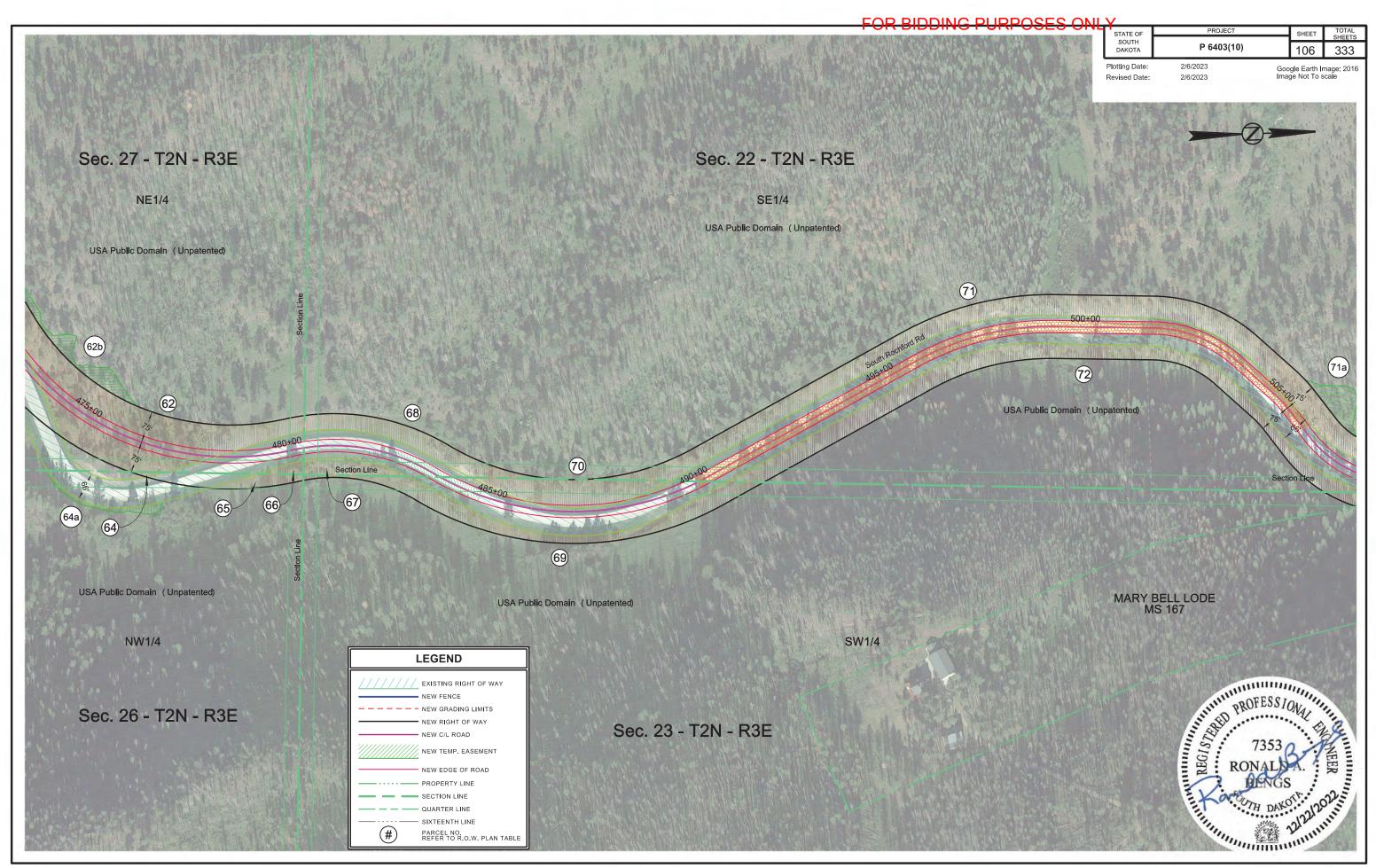


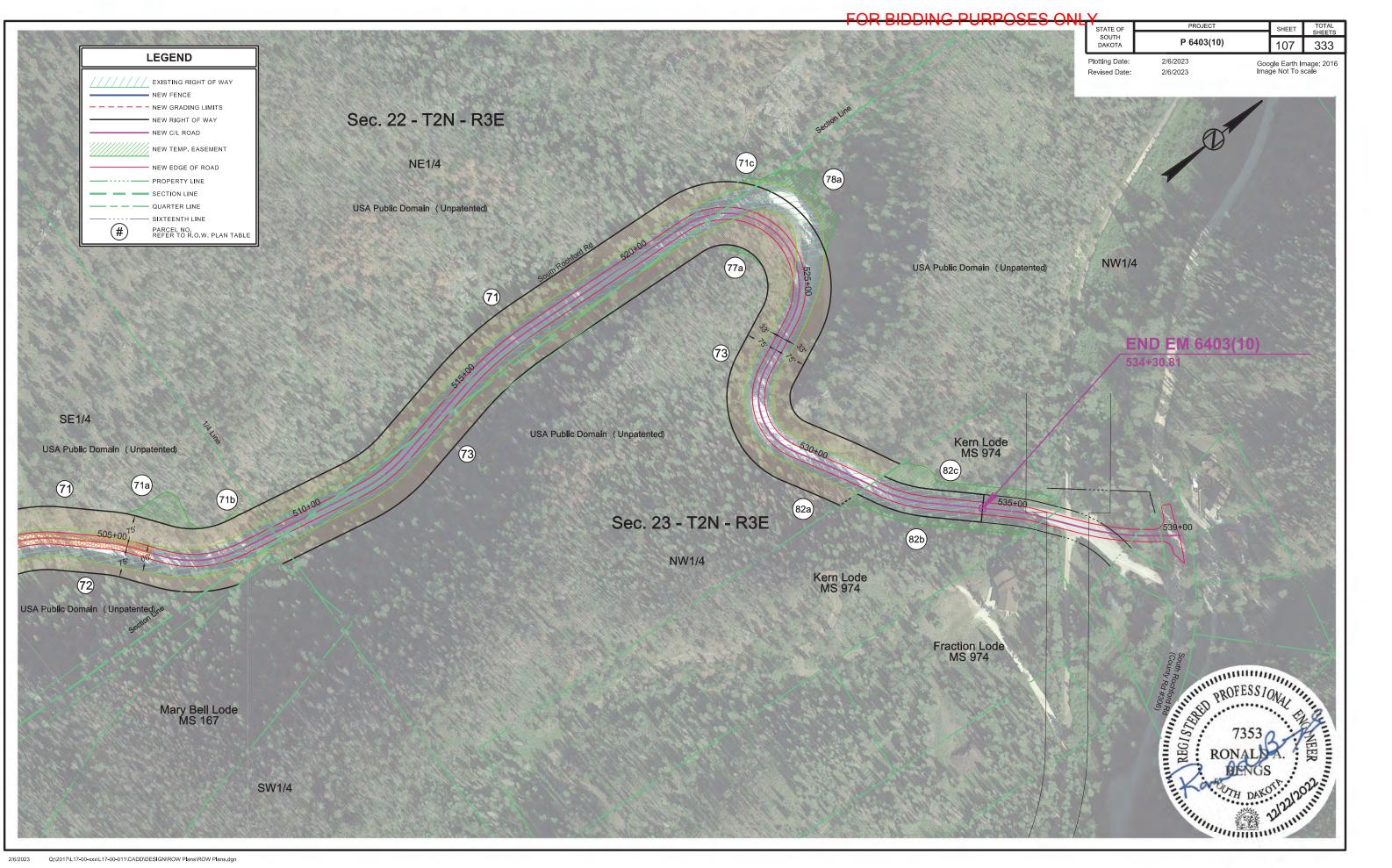












FOR RIDDING PURPOSES ONLY

STATE OF	PROJECT	SHEET	TOTAL
SOUTH			SHEETS
3001H	D 6402/40\		
DAKOTA	P 6403(10)	108	333

Plotting Date: 2/6/2023 Revised: 02/06/2023

S. ROO	OAD CLOSE HFORD RD REEK SOUT	CLOSED F	
e*	83.8*		-1;

SIGN NUMBER Advanced Closure
WIDTH x HGHT. 7"-9" x 2"-3"
BORDER WIDTH 0.63"
CORNER RADIUS 1.5"
MOUNTING Ground
BACKGROUND TYPE: Reflective
COLOR: Orange
LEGEND/BORDER TYPE: Reflective
COLOR: White/Black

SYMBOL	ROT	Х	Υ	WID	HT

Letter locati<u>ons are panel edge to lower left carner</u>

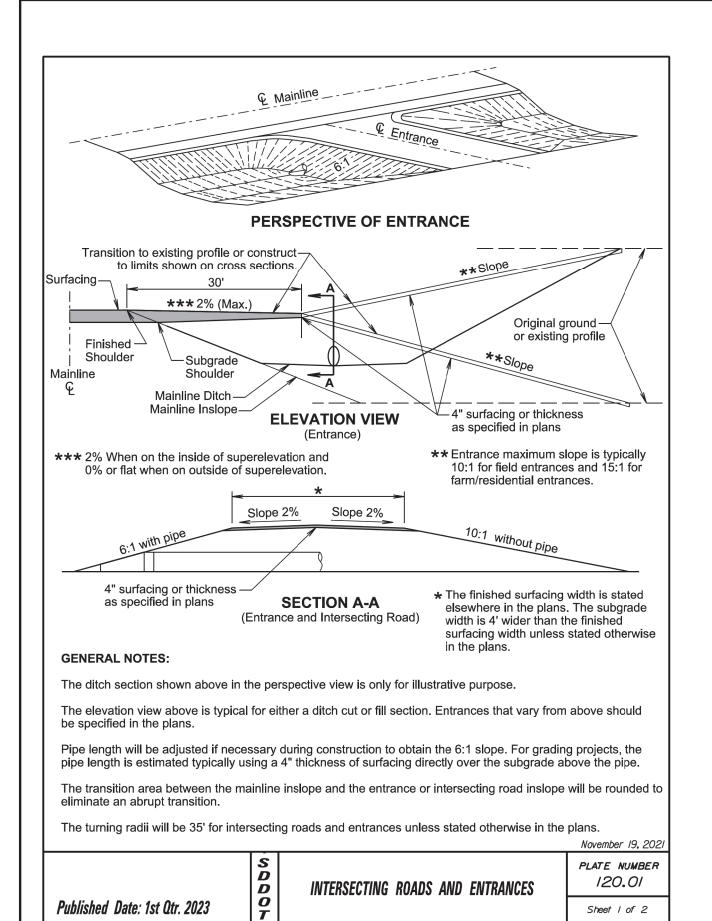
Panel Style: construction_guide.ssi Dimensions are in inches.tenths

SIGN DETAIL 1:15

																	- 1	ЕП	R F	POSIT	TIONS	(X))									LENG	TH SER	IES/SIZE
R	0	Α	D		C	L	0	S	E	D		A	Н	Ε	A	D																	С	
21.7	24.5	27.5	30.7	32.9	36.9	39.7	42.4	45.4	48.4	51.1	53.3	57.3	60.5	63.5	66	69.1							11									49.	3 4	
s			R	0	C	Н	F	0	R	D		R	D		C	L	0	S	E	D		F	R	0	М								С	
8.9	11.6	12.4	16.4	19.2	22.2	25.1	28.1	30.8	33.9	36.9	39.1	43.1	45.2	48.4	52.4	55.2	57.9	60.9	63.9	66.6	68.8	72.8	75.4	78.3	81.5							75.	1 4	
R	Α	Р	ı	D		C	R	E	E	ĸ		S	0	U	T	Н		Ť	0		D	T)	٧	1	D	Е		R	D				С	
4.6	7.4	10.6	13.6	15	17.2	21.2	24.1	27.1	29.8	32.5	34.7	38.7	41.5	44.7	47.5	50.2	52.4	56.4	59.1	61.4	65.4	68.4	69.5	72.7	74.1	77.2	79.2	83.2	86.2			83.	3 4	

	LOCATIONS FOR ADVANCED CLOSURE*
1	NB MYSTIC RD N OF INT WITH DEERFIELD RD
2	NB S. ROCHFORD RD N OF INT WITH DEERFIELD RD
3	EB BLACK FOX RD W OF INT WITH DEERFIELD RD
4	SB N. ROCHFORD RD S OF INT WITH SD HWY 14A/85
5	WB ROCHFORD RD W OF INT WITH SD HWY 385

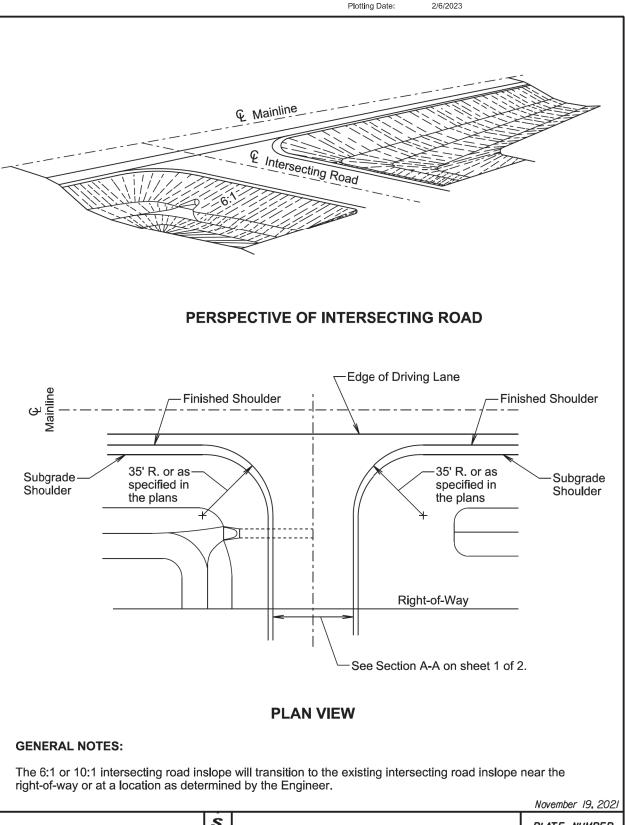




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STATE OF SOUTH P 6403(10) 109 333

2/6/2023



D D O T INTERSECTING ROADS AND ENTRANCES

Published Date: 1st Qtr. 2023

PLATE NUMBER 120.01

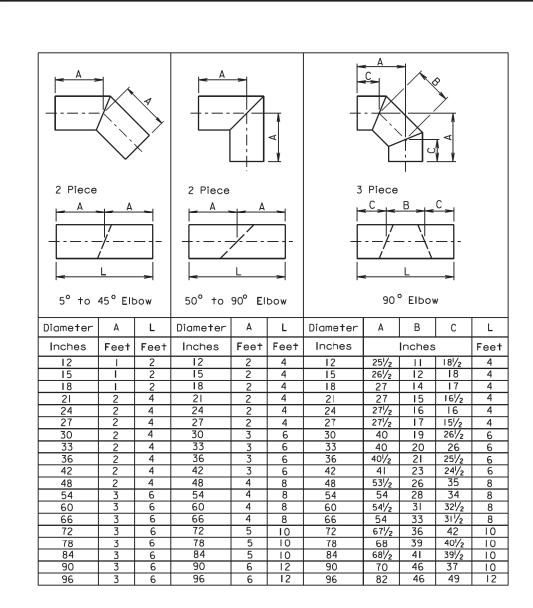
Sheet 2 of 2

 STATE OF SOUTH DAKOTA
 PROJECT
 SHEET
 TOTAL SHEETS

 110
 333

Plotting Date:

e: 2/6/2023



FABRICATED ELBOW LENGTHS FOR ALL CORRUGATIONS

GENERAL NOTES:

All dimensions shown are nominal.

L = Linear Feet of C.M.P. required to fabricate fitting.

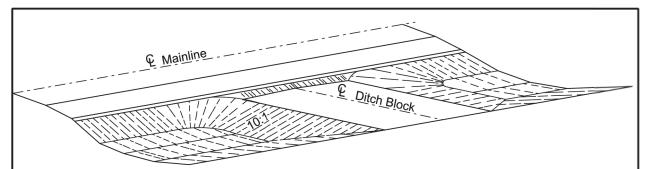
June 26, 2001

D D O T

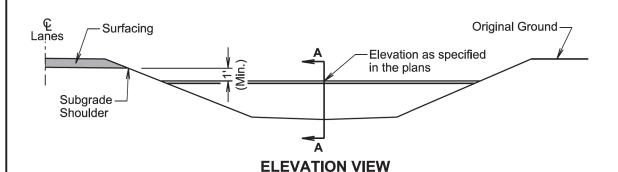
C.M.P. FABRICATED LENGTHS FOR ELBOWS

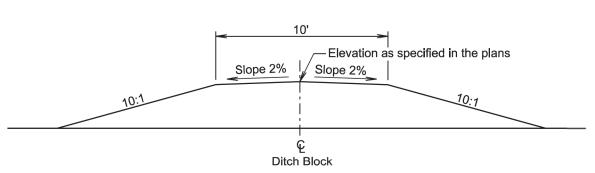
PLATE NUMBER 450.32

Sheet | of |



PERSPECTIVE OF DITCH BLOCK





SECTION A-A

GENERAL NOTES:

The ditch section shown above in the perspective and elevation view is only for illustrative purpose.

The inslopes of the ditch block will be 10:1 or as specified in the plans.

S D

D O

The transition area between the mainline inslope and the ditch block inslope will be rounded to eliminate an abrupt transition.

September 14, 2018

Sheet I of I

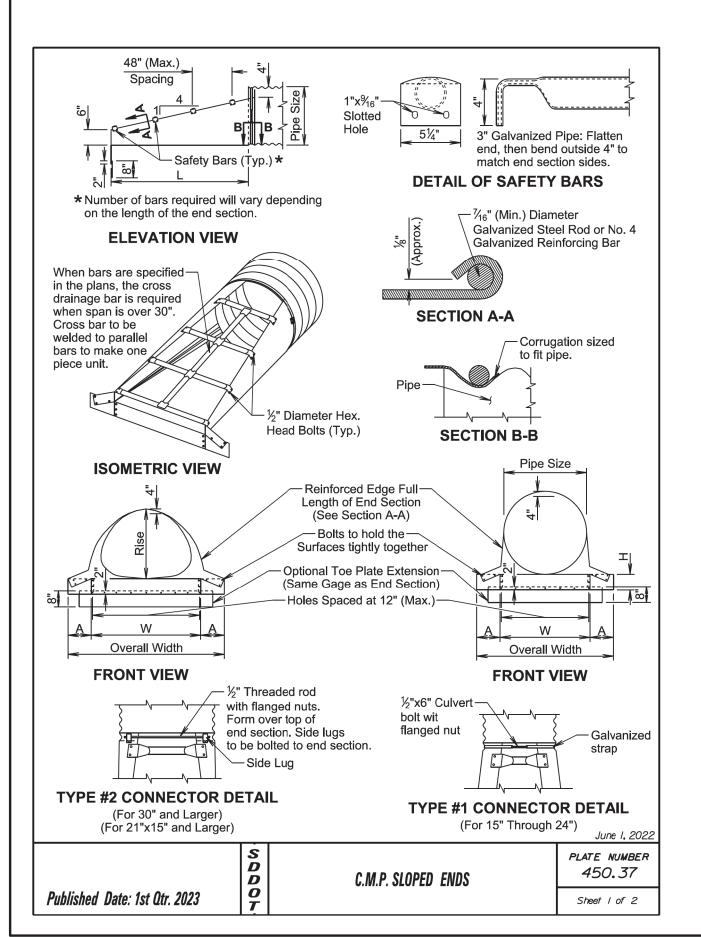
PLATE NUMBER 120.02

Published Date: 1st Qtr. 2023

DITCH BLOCK

Published Date: 1st Qtr. 2023

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EOD BIDDING DUDDOSES ONLY

STATE OF SOUTH DAKOTA P 6403(10) 111 333

Plotting Date:

2/6/2023

			ARCH	C.M.F	P. SL	OPE	DE	NDS			
Equiv.	(Incl	nes)	(Min.)	Thick.	Dim	Dimensions (Inches)				L Dimensions	
Dia. (Inch)	Span	Rise	Inch	Gage	Α	Н	W	Overall Width	Slope	Length (Inch)	
18	21	15	.064	16	8	6	27	43	4:1	20	
21	24	18	.064	16	8	6	30	46	4:1	32	
24	28	20	.064	16	8	6	34	50	4:1	40	
30	35	24	.079	14	12	9	41	65	4:1	56	
36	42	29	.109	12	12	9	48	72	4:1	76	
42	49	33	.109	12	16	12	55	87	4:1	92	
48	57	38	.109	12	16	12	63	95	4:1	112	
54	64	43	.109	12	16	12	70	102	4:1	132	
60	71	47	.109	12	16	12	77	109	4:1	148	
72	83	57	.109	12	16	12	89	121	4:1	188	

	CIF	RCULA	R	C.M	.P. \$	SLOPED E	NDS						
Pipe	(Min.)	Thick.	Dir	nen	sior	ns (Inches)	L Dime	ensions					
Dia. (Inch)	Inch	Gage	Α	Н	W	Overall Width	Slope	Length (Inch)					
15	.064	16	8	6	21	37	4:1	20					
18	.064	16	8	6	24	40	4:1	32					
21	.064	16	8	6	27	43	4:1	44					
24	.064	16	8	6	30	46	4:1	56					
30	.109	12	12	9	36	60	4:1	80					
36	.109	12	12	9	42	66	4:1	104					
42	.109	12	16	12	48	80	4:1	128					
48	.109	12	16	12	54	86	4:1	152					
54	.109	12	16	12	60	92	4:1	176					
60	.109	12	16	12	66	98	4:1	200					

GENERAL NOTES:

Published Date: 1st Qtr. 2023

Safety bars will be provided when specified in the plans.

Sloped ends will be fabricated from galvanized steel and will conform to the requirements of the Specifications.

Safety bars will be fabricated from steel schedule 40 pipe in conformance with ASTM A53, grade B or HSS 3.5x.216 in conformance with ASTM A500, grade B.

Slotted holes for safety bar attachment will be provided for all end sections.

Attachment to circular pipes 15" through 24" diameter will be made with Type #1 straps. All other sizes will be attached with Type #2 rods and lugs.

When stated in the plans, optional toe plate extension will be punched and bolted to end section apron lip with %" diameter galvanized bolts. Steel for toe plate extension will be same gauge as end section. Dimensions will be overall width less 6" by 8" high.

Installation will be performed in accordance with the Specifications.

Cost of all work and materials required for fabrication and installation of sloped ends will be incidental to the bid items for the various sizes of sloped ends.

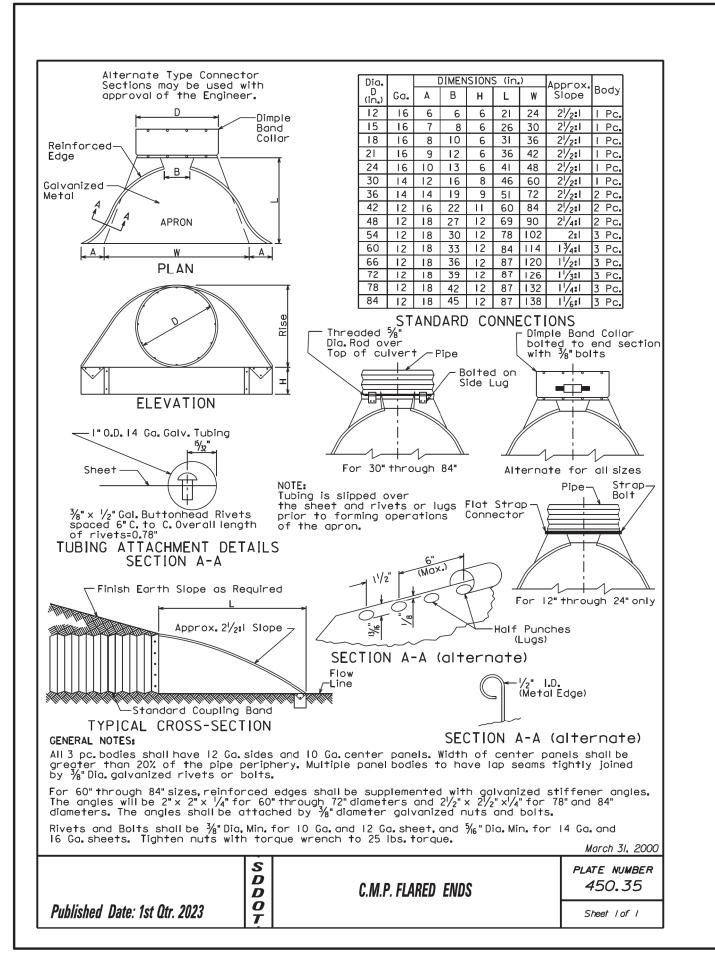
June I, 2022

D D O T

C.M.P. SLOPED ENDS

PLATE NUMBER 450.37

Sheet 2 of 2



P 6403(10) 112 333 DAKOTA Plotting Date: 2/6/2023 24" (Max.) Spacing 1"x% Slotted Hole 5¼" 3" Galvanized Pipe: Flatten end, then bend outside 4" to match end section sides. Safety Bars (Typ.) * ₩ **DETAIL OF SAFETY BARS** * Number of bars required will vary depending $-\frac{7}{16}$ " (Min.) Diameter on the length of the end section. Galvanized Steel Rod or No. 4 **ELEVATION VIEW** Galvanized Reinforcing Bar **SECTION A-A** Corrugation sized to fit pipe. Pipe %" Diameter Hex. Head **SECTION B-B** Bolts (Typ.) Pipe Size **ISOMETRIC VIEW** Reinforced Edge Full-Length of End Section (See Section A-A) -Bolts to hold the-Surfaces tightly together Optional Toe Plate Extension (Same Gage as End Section) <u>√</u> ∞ Holes Spaced at 12" (Max.) W W Overall Width Overall Width **FRONT VIEW FRONT VIEW** $-\frac{1}{2}$ " Threaded rod ½"x6" Culvert with flanged nuts. bolt with Form over top of end section. Side lugs flanged nut Galvanized to be bolted to end section. strap Side Lug **TYPE #2 CONNECTOR DETAIL TYPE #1 CONNECTOR DETAIL** (For 30" and Larger) (For 15" Through 24") (For 21"x15" and Larger) June 1, 2022 S PLATE NUMBER D *450.38* C.M.P. SAFETY ENDS D 0 Published Date: 1st Qtr. 2023 Sheet I of 2

STATE OF

SHEET

EOD BIDDING DUDDOGES ONI

	ARCH C.M.P. SAFETY ENDS											
Equlv.	(Incl	nes)	(Min.)	Thick.	Dim	ensi	ons ((Inches)	L Dime	ensions		
Dia. (Inch)	Span	Rise	Inch	Gage	Α	Н	W	Overall Width	Slope	Length (Inch)		
18	21	15	.064	16	8	6	27	43	6:1	30		
21	24	18	.064	16	8	6	30	46	6:1	48		
24	28	20	.064	16	8	6	34	50	6:1	60		
30	35	24	.079	14	12	9	41	65	6:1	84		
36	42	29	.109	12	12	9	48	72	6:1	114		
42	49	33	.109	12	16	12	55	87	6:1	138		
48	57	38	.109	12	16	12	63	95	6:1	168		
54	64	43	.109	12	16	12	70	102	6:1	198		
60	71	47	.109	12	16	12	77	109	6:1	222		
72	83	57	.109	12	16	12	89	121	6:1	282		

	CIF	RCULA	R	C.M	.P. :	SAFETY E	NDS	
Pipe	(Min.)	Thick.	L Dime	ensions				
Dia. (Inch)	Inch	Gage	Α	Н	W	Overa l l Width	Slope	Length (Inch)
15	.064	16	8	6	21	37	6:1	30
18	.064	16	8	6	24	40	6:1	48
21	.064	16	8	6	27	43	6:1	66
24	.064	16	8	6	30	46	6:1	84
30	.109	12	12	9	36	60	6:1	120
36	.109	12	12	9	42	66	6:1	156
42	.109	12	16	12	48	80	6:1	192
48	.109	12	16	12	54	86	6:1	228
54	.109	12	16	12	60	92	6:1	264
60	.109	12	16	12	66	98	6:1	300

GENERAL NOTES:

Safety bars will be provided when specified in the plans.

Safety ends will be fabricated from galvanized steel conforming to the requirements of the Specifications.

Safety bars will be fabricated from steel schedule 40 pipe in conformance with ASTM A53, grade B or HSS 3.5x,216 in conformance with ASTM A500, grade B.

Slotted holes for safety bar attachment will be provided for all end sections.

Attachment to circular pipes 15" through 24" diameter will be made with Type #1 straps. All other sizes will be attached with Type #2 rods and lugs.

When stated in the plans, optional toe plate extension will be punched and bolted to end section apron lip with %" diameter galvanized bolts. Steel for toe plate extension will be same gauge as end section. Dimensions will be overall width less 6" by 8" high.

Installation will be performed in accordance with the Specifications.

Cost of all work and materials required for fabrication and installation of safety ends will be incidental to the bid items for the various sizes of safety ends.

			June 1, 2022
	SDD	C.M.P. SAFETY ENDS	PLATE NUMBER 450.38
Published Date: 1st Qtr. 2023			Sheet 2 of 2

FOR BIDDING PURPOSES ONLY

STATE OF	PROJECT	SHEET	TOTAL SHEETS	
SOUTH DAKOTA	P 6403(10)	113	333	
Plotting Date:	2/6/2023			

SPECIFICATIONS:

Design Specifications: AASHTO Specifications for Highway Bridges, 1996 Edition (Service Load).

Construction Specifications: South Dakota Standard Specifications for Roads and Bridges, Current Edition and required Provisions, Supplemental Specifications, and Special Provisions as included in the Proposal.

GENERAL NOTES:

Design Loading: HS20-44 AASHTO.

Published Date: 1st Qtr. 2023

Cattle guards will be constructed in accordance with Section 610.

All concrete will be Class M6 in accordance with Section 462.

All reinforcing steel will conform to ASTM A615, Grade 60.

Use 1½" clear cover on all reinforcing steel except as shown.

All structural steel will conform to ASTM A709, Grade 36. The end welded deformed bar anchors will conform to ASTM A1064. The $\frac{1}{2}$ " diameter x 6" concrete anchors will conform to Section 970.2 B. All bolts and washers will conform to ASTM A307, except that lock washers will conform to AISI B18.21.1.

¾" diameter concrete inserts will be internally threaded for use with a standard galvanized A307 bolt and will be of such design that when installed in the concrete it will be capable of sustaining a safe working load in tension of 5500 pounds. The inserts will be galvanized or made of a corrosion resistant material.

Place $\frac{3}{4}$ " diameter concrete inserts in ends of abutment foundations for $\frac{3}{4}$ " diameter galvanized connecting bolts. See detail W on sheet 4 of 5.

The armor angles, connecting channels, and bearing plates will be painted with a paint system which conforms to Section 412. 2 and will be applied in accordance with the manufacturer's recommendations. The top coat will be green in color, conforming to Federal Standard 24108.

If the cattle guard must be installed to conform to a grade other than a zero grade, all elements of the cattle guard foundation will be built normal to the grade.

Alternate designs will be considered; submit detailed drawings and specifications of the proposed similar cattle guard through proper channels to the Office of Bridge Design for approval.

In lieu of concrete inserts and A307 bolts, a $\frac{3}{4}$ " diameter A307 rod threaded full length may be drilled and epoxied into the hardened concrete. The diameter of the drilled holes in the concrete for the threaded rod will not be less than $\frac{3}{4}$ " nor more than $\frac{3}{4}$ " greater than the overall diameter of the threaded rod and the depth will be a minimum of ten rod diameters.

The epoxy resin mixture for bonding the threaded rod in place will conform to ASTM C881 Type IV Grade 3 requirements for bonding steel to hardened concrete.

The epoxy resin adhesive will be mixed and applied as recommended by the manufacturer. Special attention must be given to the elimination of voids by using effective grouting procedures as approved by the Engineer.

Soil bearing pressure will be a minimum of 3000 psf as approved by the Engineer.

August 27, 2020

CAST-IN-PLACE CATTLE GUARD FOUNDATIONS

PLATE NUMBER 610.01

Sheet I of 5

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INFORMATIONAL QUANTITIES (For Two Foundations)													
ITEM	UNIT		QUANTITY										
I I ⊏IVI	UNIT	10' GUARD	12' GUARD	14' GUARD	20' GUARD	24' GUARD	30' GUARD						
Class M6 Concrete	Cu. Yd.	2.8	3.3	3.9	5.5	6.6	8.3						
Reinforcing Steel	Lb.	300	359	418	594	712	888						
* Structural Steel	Lb.	487	524	561	671	745	856						

^{*} Includes armor angles with bar anchors, bearing plates with concrete anchors, connecting channels, bolts and washers; excludes concrete inserts.

	REINFORCING SCHEDULE (For Two Foundations)													
	Mk.	No.	Size	Length	Туре		Mk.	No.	Size	Length	Туре	Bending Details		
NA NA	а	22	4	5'-4"	21A	K	а	42	4	5'-4"	21A	0.11		
GUAR	b	22	4	3'-11"	17A	M	b	42	4	3'-11"	17A	<u>→</u> < 3"		
	С	22	4	9'-9"	Str.	ß	С	22	4	19'-9"	Str.	l. 		
10	f	18	4	1'-9"	Str.	20	f	38	4	1'-9"	Str.	[-		
												Type 21A		
	а	26	4	5'-4"	21A		а	50	4	5'-4"	21A			
씸	b	26	4	3'-11"	17A	AR AR	b	50	4	3'-11"	17A	1'-3" a		
GUARD	С	22	4	11'-9"	Str.	30	С	22	4	23'-9"	Str.			
2' G	f	22	4	1'-9"	Str.	4	f	46	4	1'-9"	Str.	9"		
12						24								
	а	30	4	5'-4"	21A	۵	а	62	4	5'-4"	21A	I ≞⊼ ┌─		
GUARD	b	30	4	3'-11"	17A	AR	b	62	4	3'-11"	17A	Type 17A		
ΙΞ	С	22	4	13'-9"	Str.	30	С	22	4	29'-9"	Str.	- -		
4 6	f	26	4	1'-9"	Str.	_	f	58	4	1'-9"	Str.			
14						30						 		
NO	OTE	: All	dime	nsions a	are ou	ıt t	o ou	t of I	oars.			•		

BILL OF MATERIALS (For Two Foundations)										
ITEM	UNIT			_,	NTITY					
I I ⊏IVI	ONLI	10' GUARD	12' GUARD	14' GUARD	20' GUARD	24' GUARD	30' GUARD			
Bearing P (6" Width)	Each	4	4	4	4	4	4			
Bearing PL (12" Width)	Each	8	10	12	18	22	28			
Connecting Channels	Each	2	2	2	2	2	2			
Clamping Bolts	Each	20	24	28	40	48	60			
Connecting Bolts	Each	24	24	24	24	24	24			

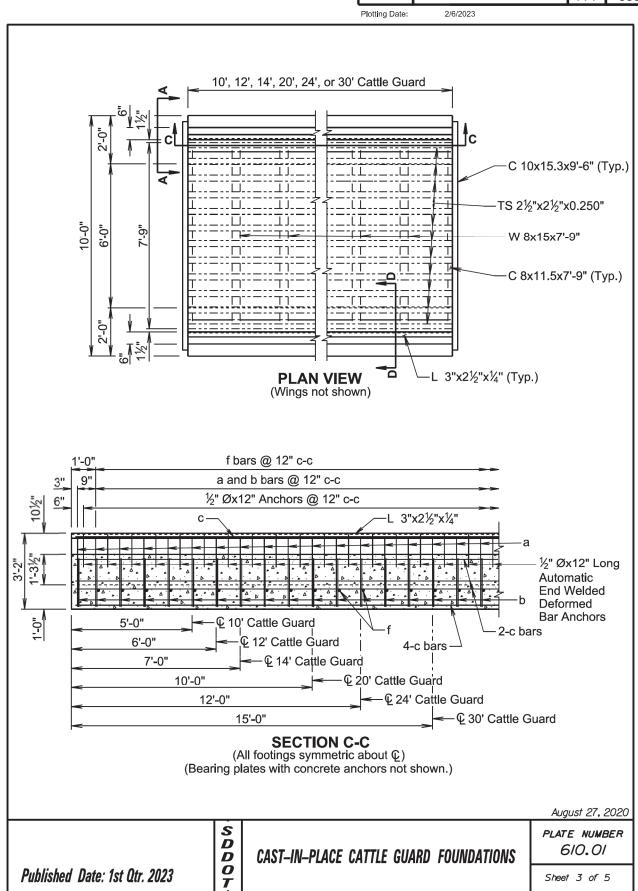
August 27, 2020

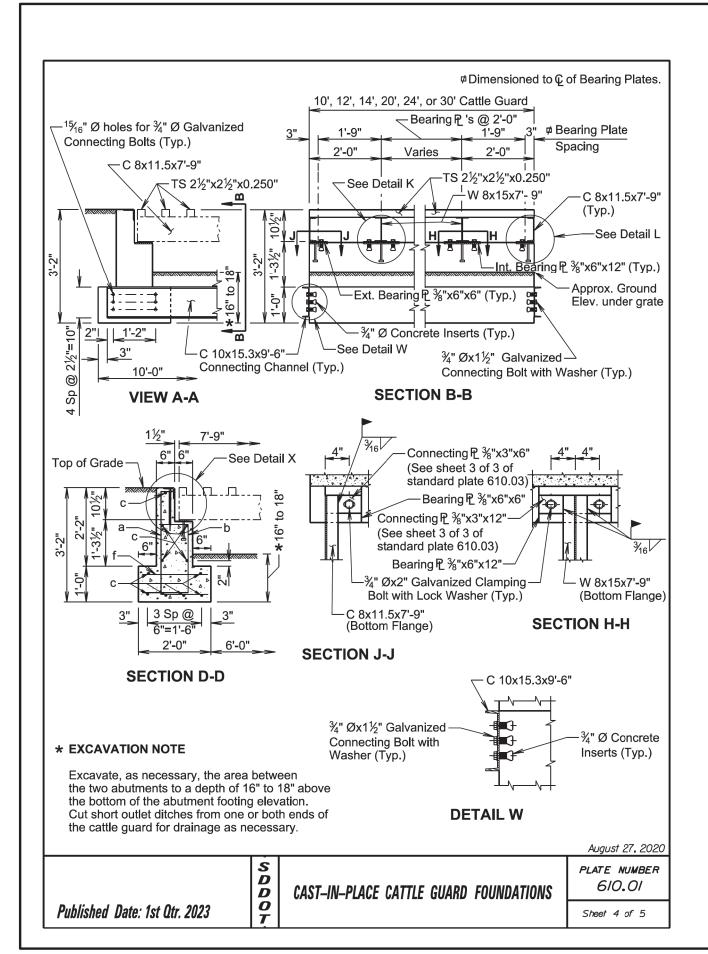
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	S D D	CAST-IN-PLACE CATTLE GUARD FOUNDATIONS	PLATE NUMBER 610.01
Published Date: 1st Qtr. 2023	0 T		Sheet 2 of 5

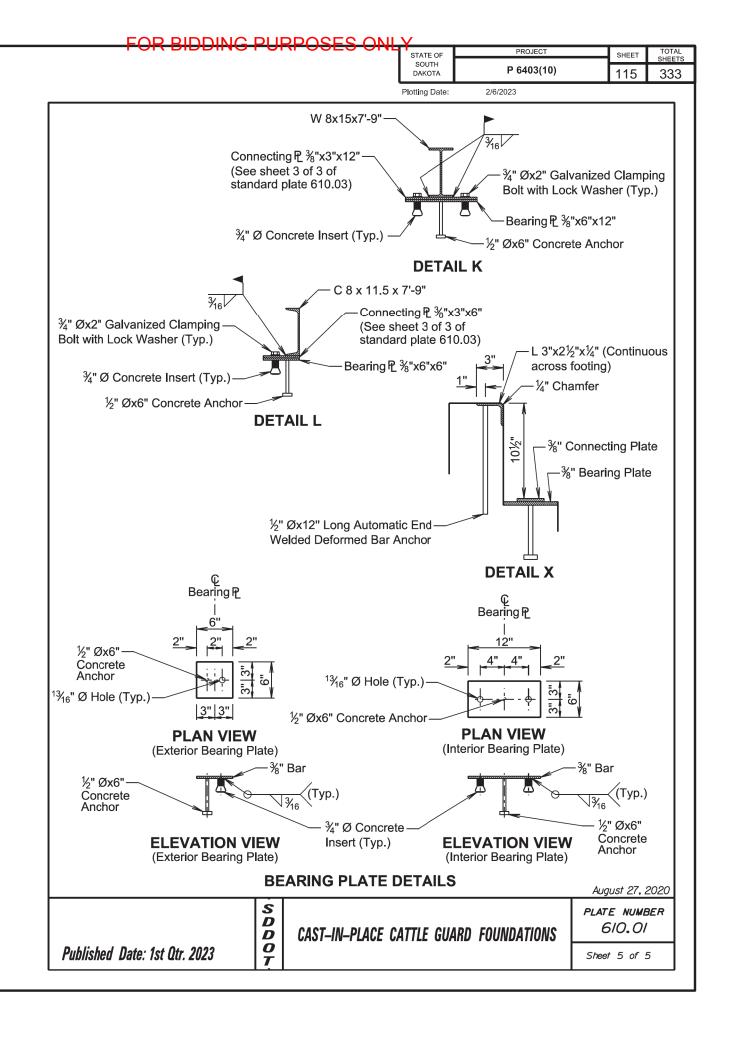
FOR RIDDING PURPOSES ONLY

 STATE OF SOUTH DAKOTA
 PROJECT
 SHEET SHEET
 TOTAL SHEETS

 114
 333







SPECIFICATIONS:

Design Specifications: AASHTO Specifications for Highway Bridges, 1996 Edition (Service Load).

Construction Specifications: South Dakota Standard Specifications for Roads and Bridges, Current Edition and required Provisions, Supplemental Specifications, and Special Provisions as included in the Proposal.

GENERAL NOTES:

Design Loading: HS20-44 AASHTO.

Cattle guards will be constructed in accordance with Section 610.

All concrete will be Class M6 in accordance with Section 462.

All reinforcing steel will conform to ASTM A615, Grade 60.

Use 1½" clear cover on all reinforcing steel except as shown.

All structural steel will conform to ASTM A709, Grade 36. The end welded deformed bar anchors will conform to ASTM A1064. The $\frac{1}{2}$ " diameter x 6" concrete anchors will conform to Section 970.2 B. All bolts, nuts, and washers will conform to ASTM A307, except that lock washers will conform to AISI B18.21.1.

Welding and weld inspection will be in accordance with AWS D1.1-(Current Year).

¾" diameter concrete inserts will be internally threaded for use with a standard galvanized A307 Bolt and will be of such design that when installed in the concrete it will be capable of sustaining a safe working load in tension of 5500 pounds. The inserts will be galvanized or made of a corrosion resistant material.

The armor angles, connecting plates, connecting channels, and bearing plates will be painted with a paint system which conforms to Section 412.2 and will be applied in accordance with the manufacturer's recommendations. The top coat will be green in color, conforming to Federal Standard 24108.

If the cattle guard must be installed to conform to a grade other than a zero grade, all elements of the cattle guard foundation will be built normal to the grade.

Alternate designs will be considered; submit detailed drawings and specifications of the proposed similar cattle guard through proper channels to the Office of Bridge Design for approval.

Soil bearing pressure will be a minimum of 3000 psf as approved by the Engineer.

	BILL OF MATERIALS FOR CATTLE GUARD FOUNDATIONS												
WIDTH OF CATTLE	PRECAST	FOUNDAT	ION UNITS	CONN. CHANNELS	CLAMP.								
GUARD	6'-0"	8'-0"	10'-0"	CHANNELS	BOLIS	BOLIS							
10'			2	2	20	24							
12'	4			2	24	36							
14'	2	2		2	28	36							
20'			4	2	40	36							
24'		6		2	48	48							
30'			6	2	60	48							

December 23, 2019

S D D

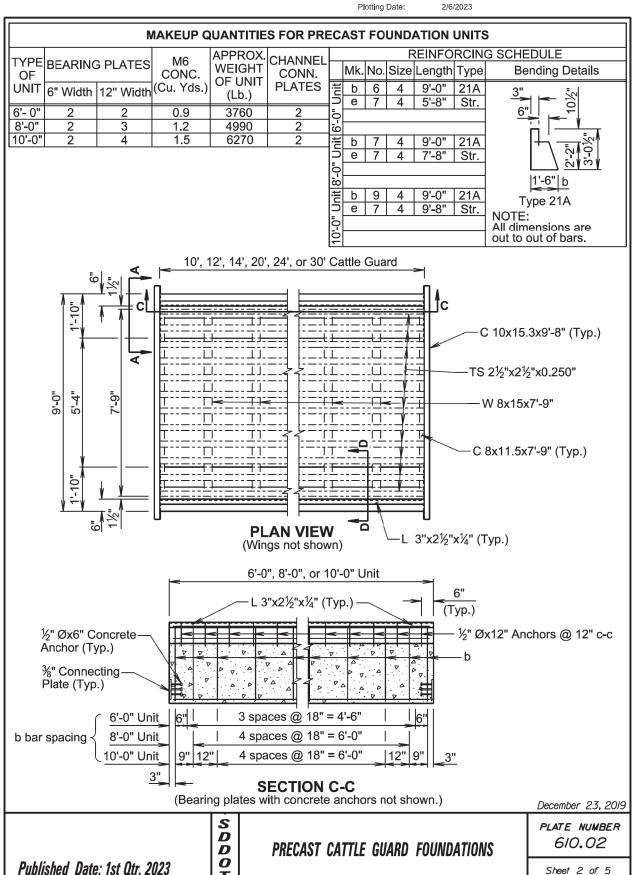
PRECAST CATTLE GUARD FOUNDATIONS

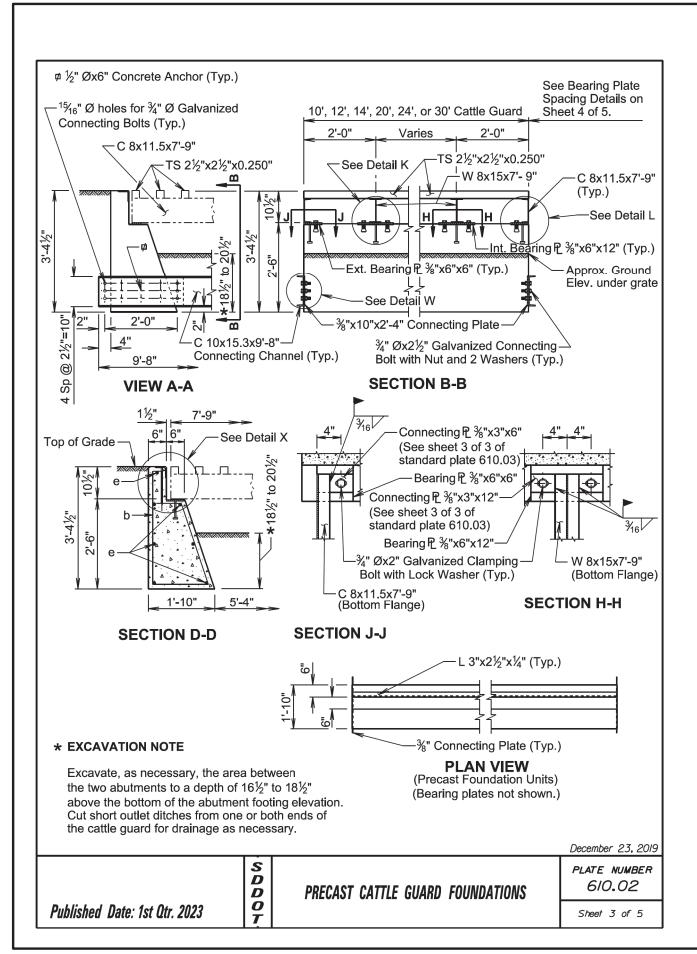
PLATE NUMBER 610.02

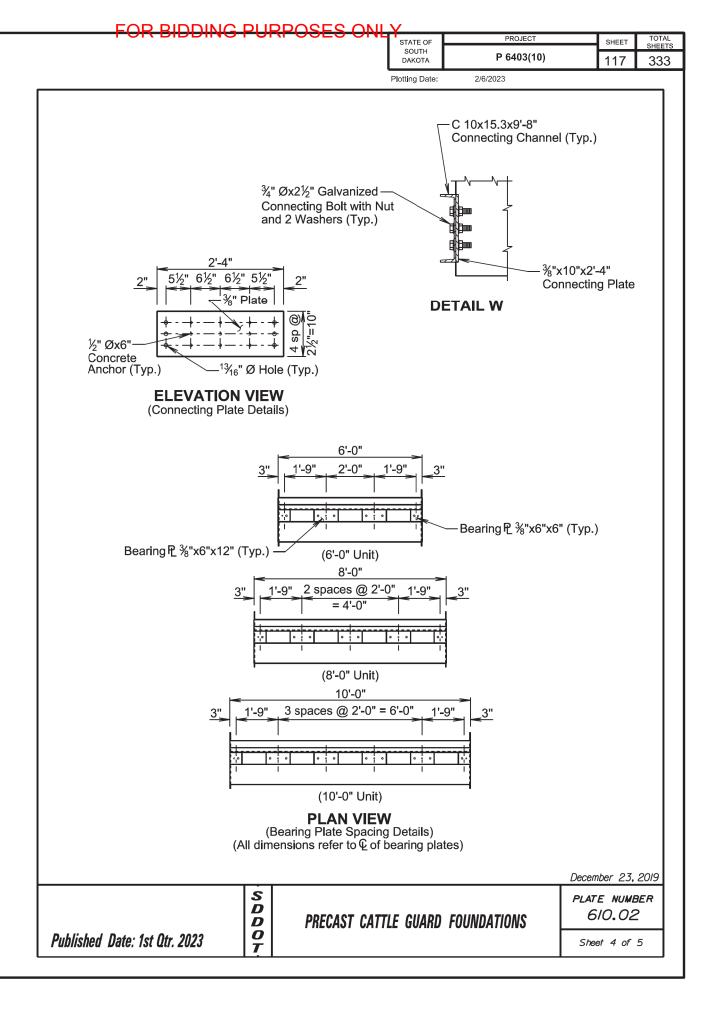
Published Date: 1st Qtr. 2023 Sheet 1 of 5

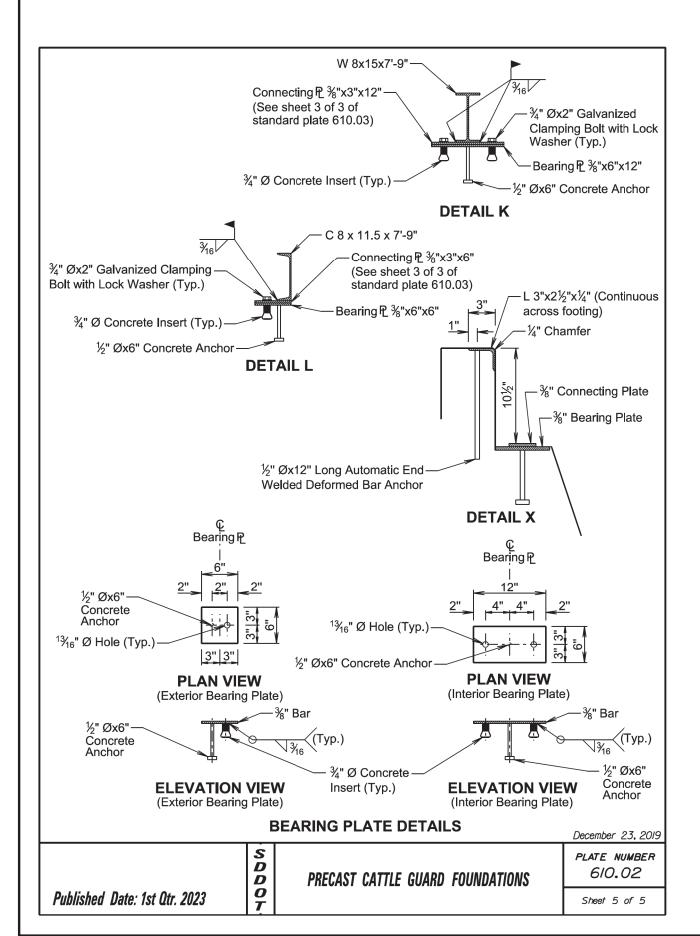
FOR BIDDING PURPOSES ONLY

| STATE OF | SOUTH | DAKOTA | P 6403(10) | 116 | 333









T	STATE OF	PROJECT	SHEET	TOTAL SHEETS
	SOUTH DAKOTA	P 6403(10)	118	333
Ī	Plotting Date:	2/6/2023		

SPECIFICATIONS:

Design Specifications: AASHTO Specifications for Highway Bridges, 1996 Edition (Service Load).

Construction Specifications: South Dakota Standard Specifications for Roads and Bridges, Current Edition and required Provisions, Supplemental Specifications, and Special Provisions as included in the Proposal.

GENERAL NOTES:

Design Loading: HS20-44 AASHTO.

Cattle guards will be constructed in accordance with Section 610.

All structural steel will conform to ASTM A709, Grade 36. Structural tubing will conform to ASTM A500, Grade B. All bolts and nuts will be galvanized and will conform to ASTM A307. All lock washers will be galvanized and will conform to AISI B18.21.1.

Welding and weld inspection will be in accordance with AWS D1.1-(Current Year).

Cattle guard grate, wings, and connecting plates will be painted with a paint system which conforms to Section 412. 2 and will be applied in accordance with the manufacturer's recommendations. The top coat will be green in color, conforming to Federal Standard 24108.

Grate sections may be combined to obtain larger grate widths. Refer to the detail of multiple installation joint on sheet 3 of 3 when larger grate widths are required.

Cattle guard grate and wing details will be used in conjunction with cast-in-place or precast cattle guard foundation details on standard plate 610.01 or 610.02 when cattle guard foundations are required.

Alternate designs will be considered; submit detailed drawings and specifications of the proposed similar cattle guard grate or wing through proper channels to the Office of Bridge Design for approval.

INFORMATIONAL QUANTITIES						
ITEM	UNIT	QUANTITY				
	OINIT	10' GRATE	12' GRATE	14' GRATE	2-WINGS	
Structural Steel	Lb.	1496	1783	2070	114	

	BILL OF MATERIALS FOR CATTLE GUARD GRATES						
WIDTH OF CATTLE	CATTLE			CONNE		GRATE CONN.	
GUARD	10'-0"	12'-0"	14'-0"	6"	12"	BOLTS	
10'	1			4	8	_	
12'		1		4	10		
14'			1	4	12		
20'	2			4	18	6	
24'		2		4	22	6	
30'	3			4	28	12	

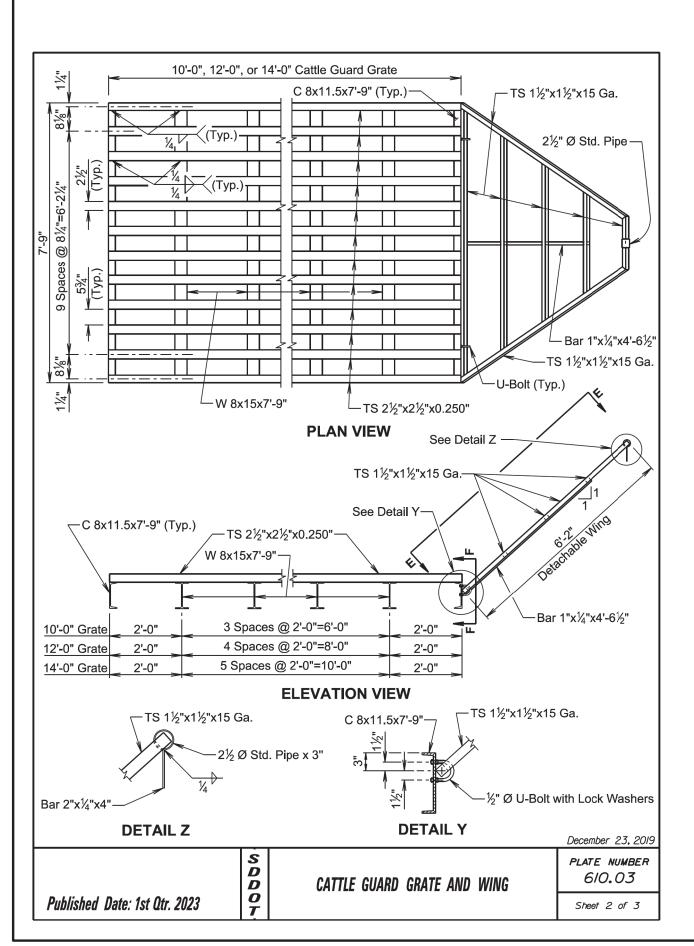
December 23, 2019

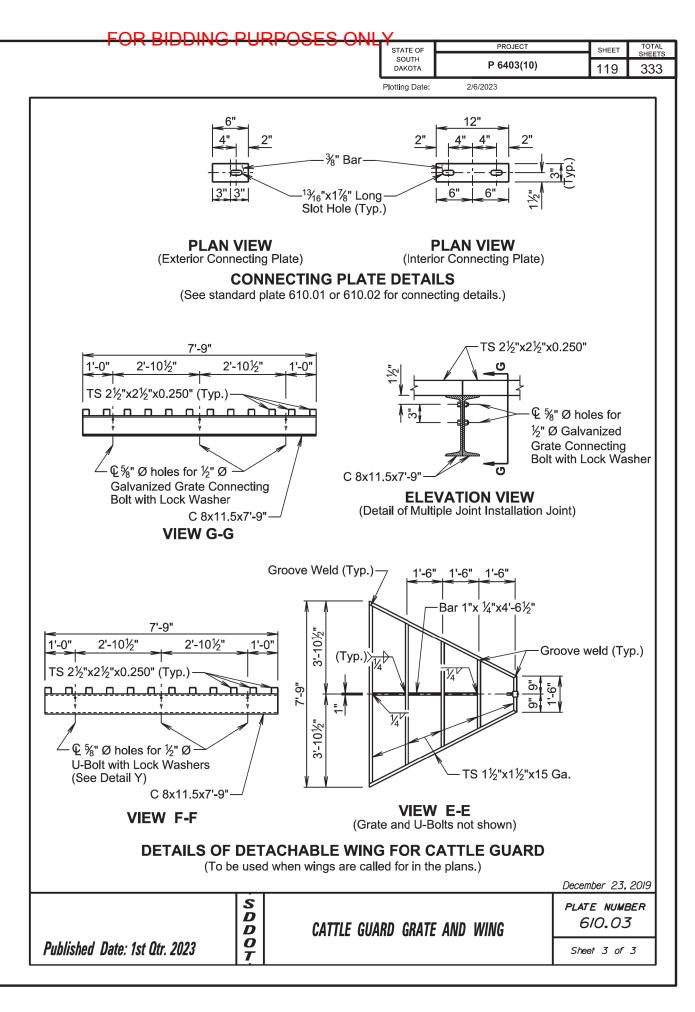
CATTLE GUARD GRATE AND WING

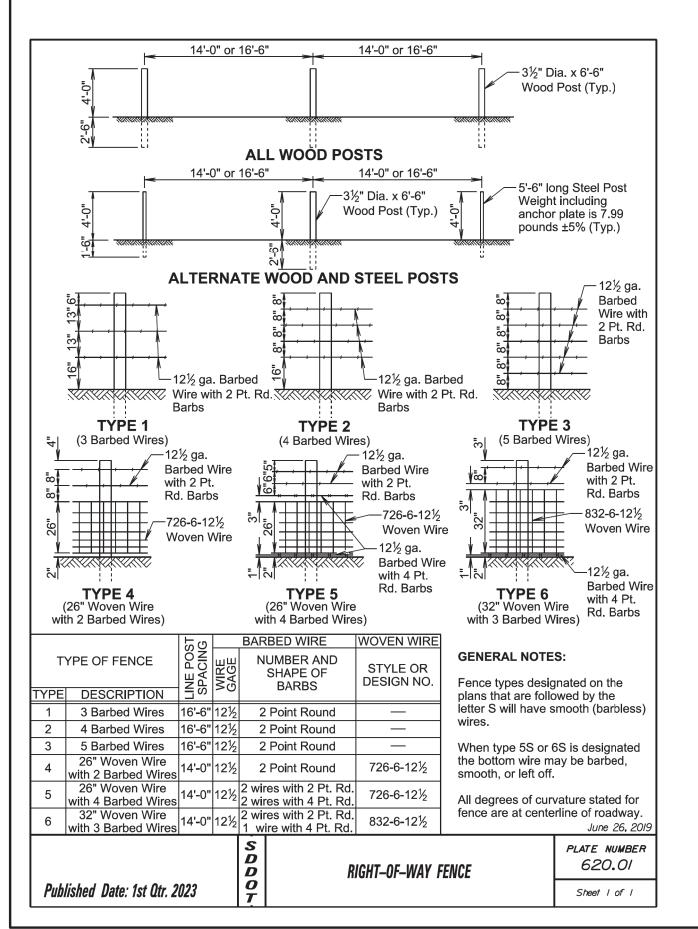
PLATE NUMBER 6/0.03

Published Date: 1st Qtr. 2023

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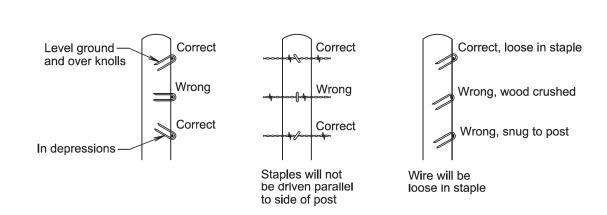




 STATE OF SOUTH DAKOTA
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Plotting Date: 2/6/2023



STAPLE INSTALLATION

GENERAL NOTES:

Published Date: 1st Qtr. 2023

The Right-of-Way fence will consist of barbed wire or a combination of woven wire and barbed wire. The barbed wire and/or woven wire will be fastened to all wood posts or fastened to alternating wood and steel posts. Only wood posts will be used for brace panels. Gates will be of the type designated in the plans or as otherwise directed by the Engineer. Fence will be constructed conforming to the details on the standard plates and in the plans unless otherwise directed by the Engineer.

Right-of-Way fence on Interstate Projects will be constructed one foot within the Interstate Right-of-Way lines except at bridge openings, cattle passes, and as otherwise directed by the Engineer.

Right-of-Way fence other than on Interstate Projects will be constructed within one foot of the Right-of-Way on the Landowner's side except at bridge openings, cattle passes, and as otherwise directed by the Engineer.

Barbs will be fabricated from zinc coated 14 ga. wire. Two point barbs will be wrapped twice around one main strand at four-inch spacings and the four point barbs will be interlocked and wrapped around both main strands at five-inch spacings.

The gages of wire and wood post lengths and sizes are the minimum acceptable unless otherwise specified in the plans. The tolerances for steel posts will be as stated in AASHTO M281. Woven wire will conform to design and specifications of ASTM A116 and barbed wire will conform to ASTM A121.

June 26, 2019

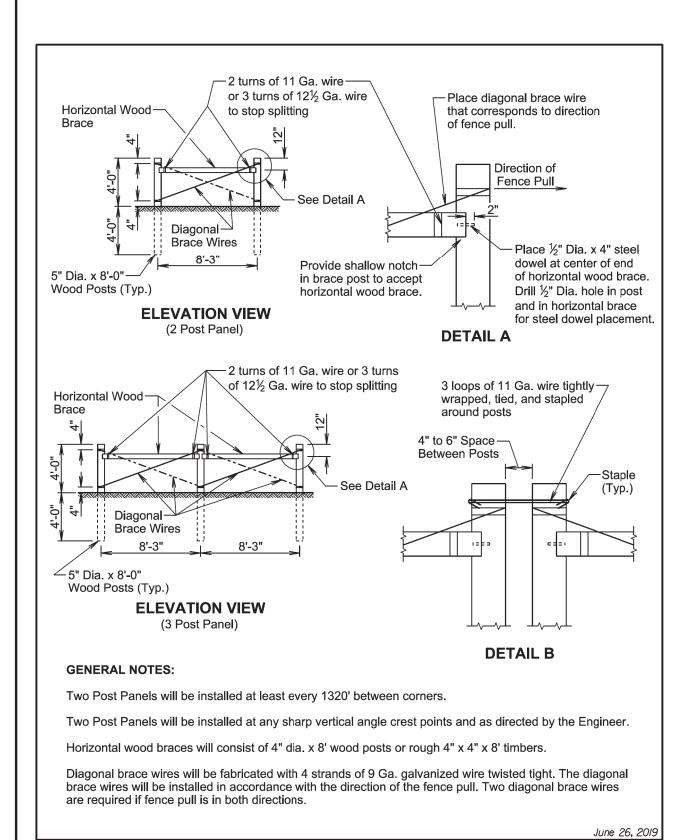
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STAPLE INSTALLATION AND GENERAL RIGHT-OF-WAY FENCE NOTES

PLATE NUMBER 620.02

Sheet I of I

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EOD RIDDING DURDOSES ONLY

****** 1320'

**At P.C., P.T., and at every

1320' between P.C. and P.T.

STATE OF SOUTH DAKOTA P 6403(10) 121 333 2/6/2023

Plotting Date:

GENERAL NOTE:

All degrees of curvature stated for fence are at centerline of roadway.

* If fence length is less than 600' to next corner use a 2 post panel.

If fence length is greater than 600' to next corner use a 3 post panel.

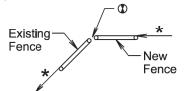
SPACING OF 2 POST PANELS WITHIN CURVES DEGREE OF CURVE SPACING OF 2 POST PANEL

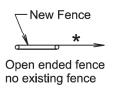
**Fence lengths greater than 1320' and less than 2640' place 2 Post Panel approximately at midpoint.

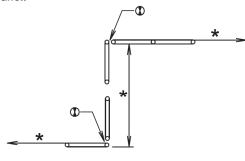
① See Detail B on Sheet 1 of 3.

less than 3°15'

3°15' and greater



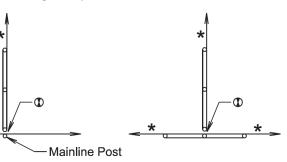




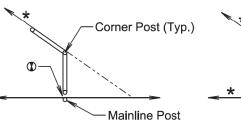
SHORT JOGS IN FENCE

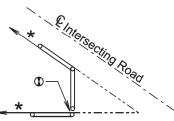
BEGIN OR END FENCE

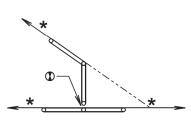
(Where new fence ties into existing fence)



CROSS FENCE

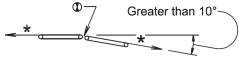






SHARP ANGLES IN CROSS FENCE





Additional fence panel is NOT required when an angle in the mainline fence is 10° and less.

Additional fence panel is required when an angle in the mainline fence is greater than 10°.

ANGLES IN MAINLINE FENCE

June 26, 2019

D D O T

BRACE PANELS AND APPLICATIONS OF BRACE PANELS PLATE NUMBER 620.03

Sheet 2 of 3

APPLICATIONS OF BRACE PANELS Published Date: 1st Qtr. 2023 Sheet I of 3

PLATE NUMBER

620.03

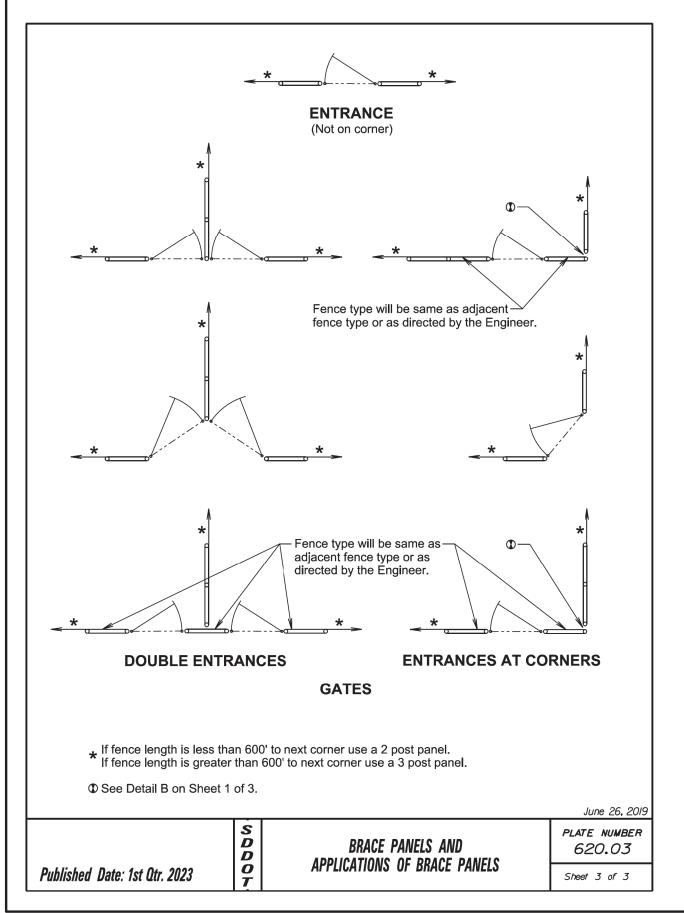
Published Date: 1st Qtr. 2023

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BRACE PANELS AND



EOD BIDDING DUDDOSES ONLY STATE OF P 6403(10) 122 333 Plotting Date: 2/6/2023 —2 or 3 Post Panel as necessary, 2 or 3 Post Panel as necessary, see standard plate 620.03 see standard plate 620.03 6" Gap 6" Gap Do Not loop wires around these posts. Fences will be independent. (Tvp.) The type of fencing for Wide Depressions will be the same type as the adjacent fence. -Place deadmen in accordance with the Specifications. Fence type as specified in the plans. Fence type as specified -

14' or 16'-6" | 14' or 16'-6"

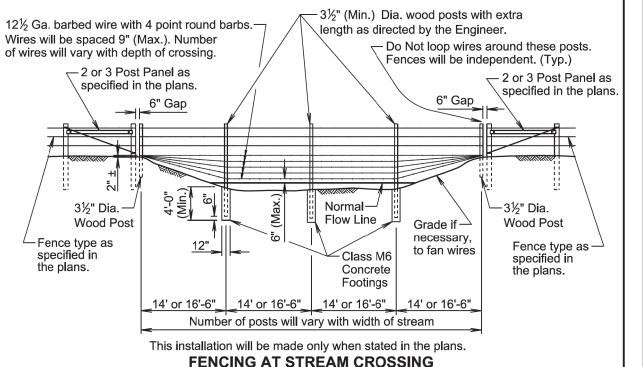
in the plans.

This installation will be made when requested by the Engineer. FENCING AT WIDE DEPRESSION

Number of posts will vary with width of depression

14' or 16'-6"

(Subject to Flooding)



GENERAL NOTES:

There will be no extra payment for the additional work and materials required to construct the fencing at the wide depression(s) and/or the fencing at the stream crossing(s). The deadmen will be paid for in accordance with 620.5 A of the Specifications.

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Measurement and payment for the fencing at the wide depression(s) and/or the fencing at the stream crossing(s) will be at the contract unit price per foot for the corresponding Right-of-Way fence contract item.

June 26, 2019

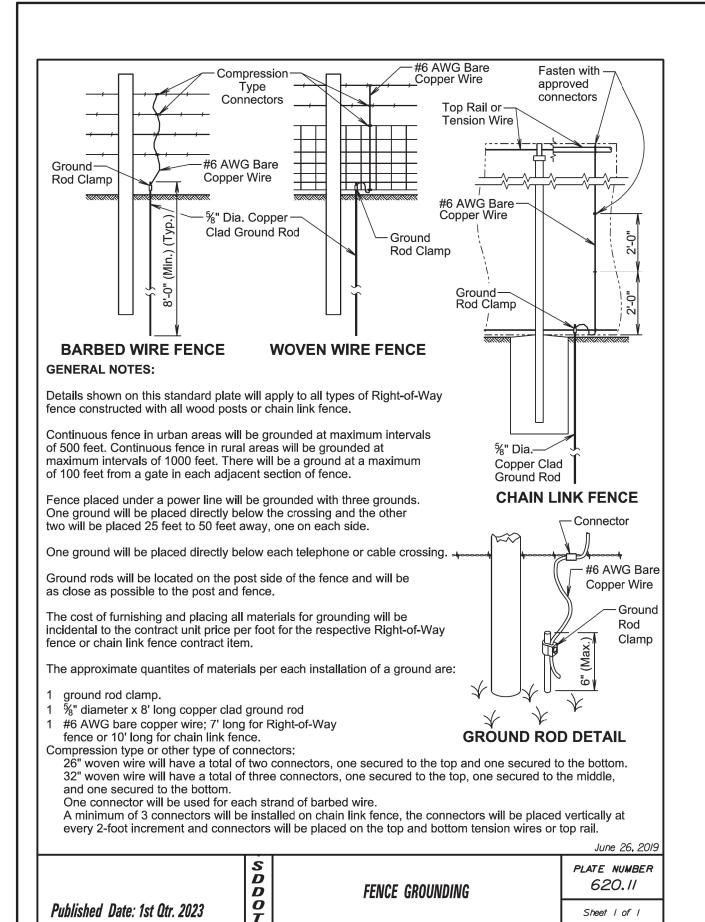
Published Date: 1st Qtr. 2023

3%" Dia. x 6'-6"-Wood Post (Typ.)

> FENCING AT WIDE DEPRESSION(S) AND STREAM CROSSING(S)

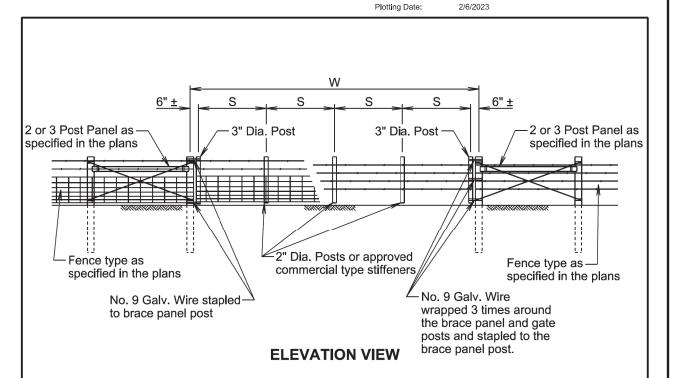
PLATE NUMBER 620.10

Sheet I of I



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W Gate Width (Ft.)	S Post Spacing
16	3 @ 5'-0" ±
20	4 @ 4'-9" ±
24	4 @ 5'-9" ±
30	5 @ 5'-10" ±
40	6 @ 6'-6" ±

GENERAL NOTES:

Creosote treatment of the gate posts will not be accepted.

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The type of fencing in the gate will be of the same type as specified for the adjacent Right-of-Way fence.

All costs for furnishing and constructing the wire gate(s) will be incidental to the contract unit price per foot for the respective Right-of-Way fence contract item.

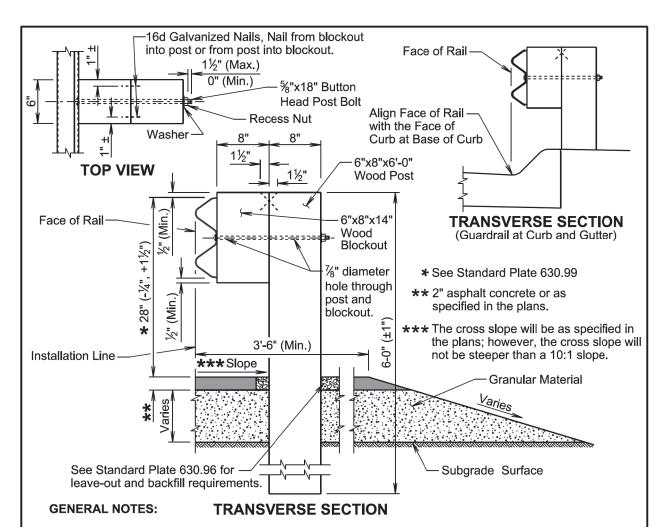
June 26, 2019

Published Date: 1st Qtr. 2023

WIRE GATES

PLATE NUMBER 620.20

Sheet I of I



Asphalt concrete will be the same type used elsewhere on the project or will be as specified in the plans. If asphalt concrete is not specified in the plans, the asphalt concrete will conform to the Specifications for "Asphalt Concrete Composite".

Granular material will be the same type used elsewhere on the project or will be as specified in the plans. If granular material type is not specified in the plans, the material will conform to the Specifications for "Base Course". The granular material will be placed the same thickness as the mainline surfacing or as specified in the plans.

Topsoil is not shown in the transverse section drawing.

All W beam rail will be Type 1 and Class A (12 Ga.) unless specified otherwise in the plans.

W beam rail section lengths may be 12'-6" and/or 25'-0". The combination of section lengths used will be compatible with the total length of rail per site as shown in the plans.

Slots in the rails will be provided as specified in the plans and by the manufacturer. A drilled hole through the rail is not allowed as a replacement for a slot. If the Contractor must create a slot, a cutting torch or plasma cutter is not allowed. The slot edges will be smooth and free of burrs or notches.

The top of post and top of block will have a true square cut. The top of block will be a maximum of $\pm \frac{1}{2}$ inch from the top of the post.

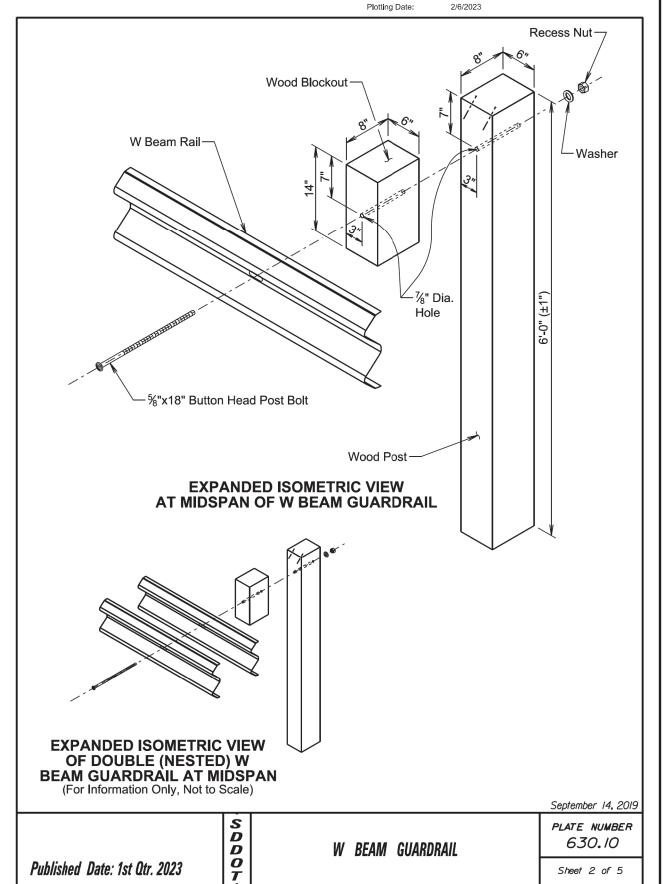
September 14, 2019

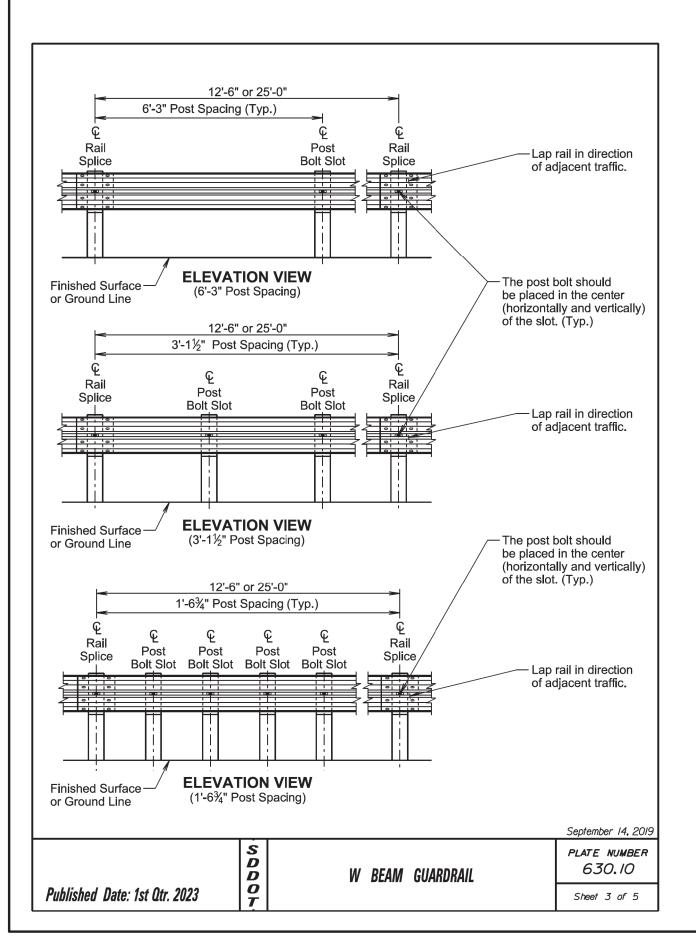
	S D D	W BEAM GUARDRAIL	PLATE NUMBER 630.10
Published Date: 1st Qtr. 2023			Sheet I of 5

FOR BIDDING PURPOSES ONLY

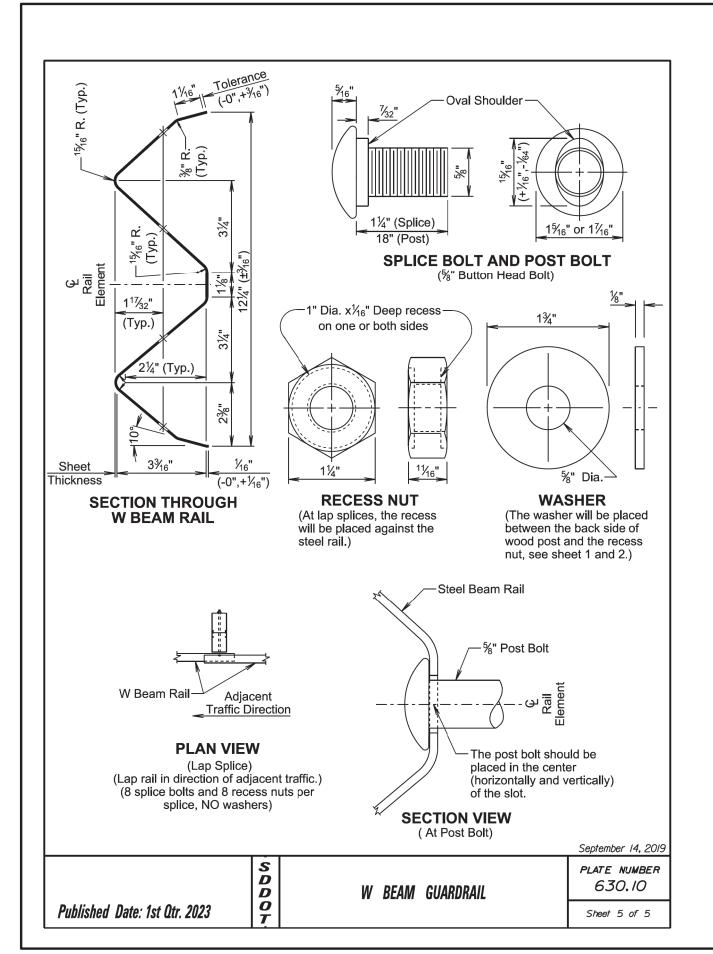
STATE OF SOUTH DAKOTA P 6403(10) SHEET TOTAL SHEETS

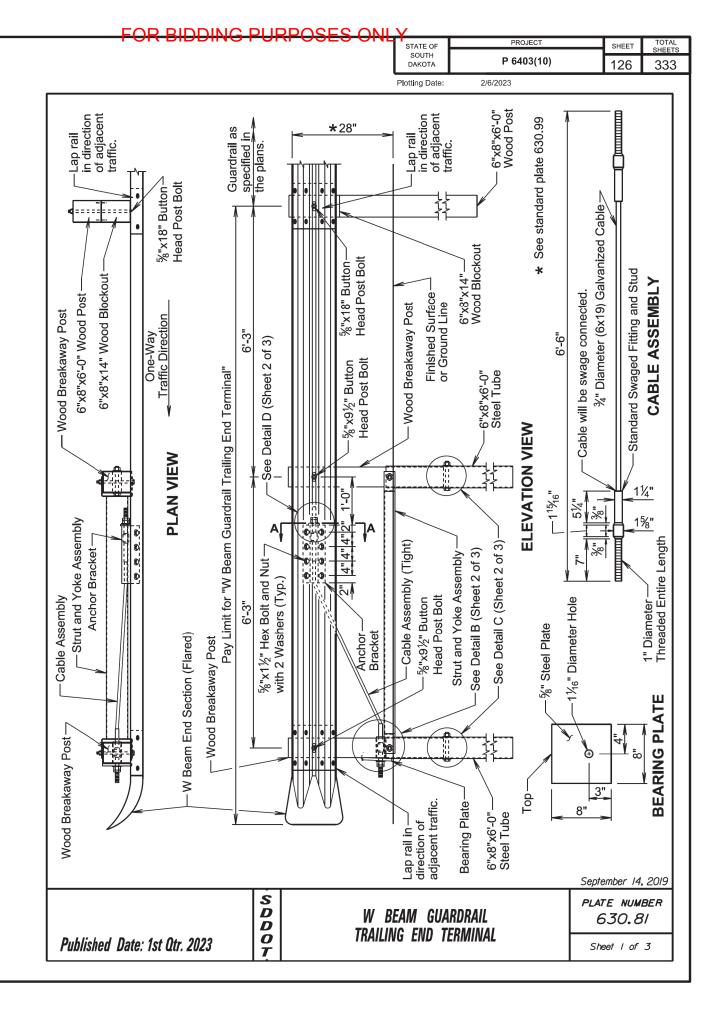
P 6403(10) 124 333

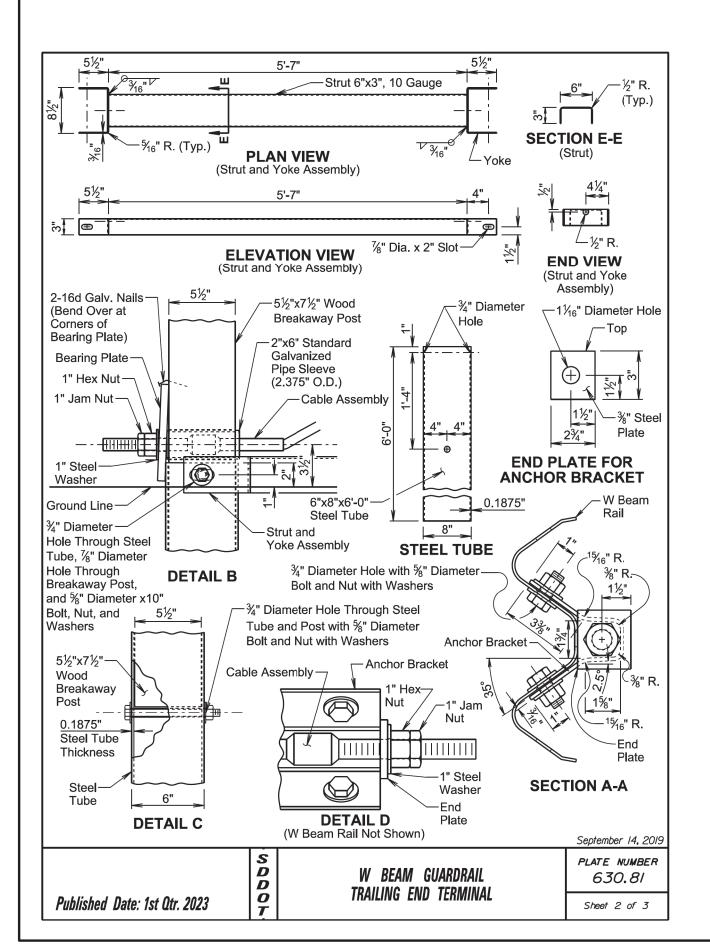




EOD BIDDING DUDDOSES ONLY STATE OF SHEET SOUTH P 6403(10) 125 333 2/6/2023 Plotting Date: 13'-6\%" (12'-6" Rail) 26'-0½" (25'-0" Rail) 6'-3" Post Bolt Slot Spacing (Typ.) 3/4"x21/2" Post Bolt Rail Rail Slot (Typ.) Splice Splice 6¼" (Typ.) 12'-6" or 25'-0" _2" (Typ.) 1 41/4" (Typ.) 4¼" (Typ.) ²%₃₂"x1%" Splice ∠ 12'-6" OR 25'-0" W BEAM RAIL Bolt Slot (Typ.) (6'-3" Post Spacing) 13'-6½" (12'-6" Rail) 26'-0½" (25'-0" Rail) 3'-7¾" 3'-7¾" 3'-1½" Post Bolt Slot Spacing (Typ.) 3'-11/5" 3'-1%" 3/4"x21/2" · Post Bolt Rail Rail Slot (Typ.) Splice Splice 6¼" (Typ.) 12'-6" or 25'-0" 2" (Typ.) 4¼" (Typ.) 4½" (Typ.) $^{2}\%_{2}$ "x1%" Splice $^{\sim}$ 12'-6" OR 25'-0" W BEAM RAIL Bolt Slot (Typ.) (3'-1½" Post Spacing) 13'-6½" (12'-6" Rail) 26'-0½" (25'-0" Rail) 2'-1" 1'-6¾" Post Bolt Slot Spacing (Typ.) 2'-1" 1'-6¾" 1'-6¾" 3/4"x21/2" Post Bolt Rail Rail Slot (Typ.) Splice Splice 6¼" (Typ.) 12'-6" or 25'-0" 2" (Typ.) 4¼" (Typ.) ²%₂"x1%" Splice → - 4¼" (Typ.) 12'-6" OR 25'-0" W BEAM RAIL Bolt Slot (Typ.) (1'-6¾" Post Spacing) September 14, 2019 S D PLATE NUMBER 630.10 W BEAM GUARDRAIL D O T Published Date: 1st Qtr. 2023 Sheet 4 of 5

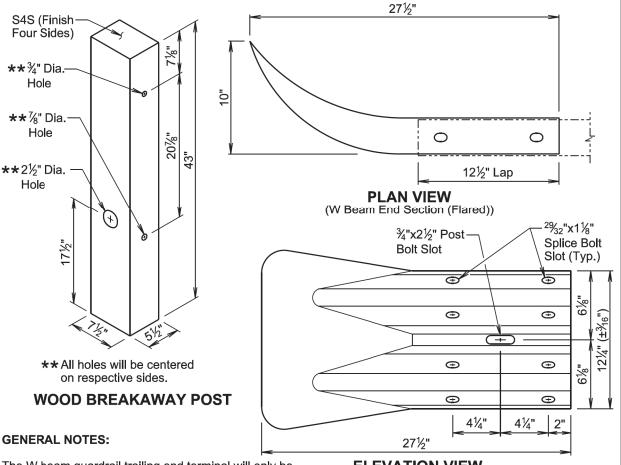






EOD BIDDING DUDDOSES ONI

STATE OF P 6403(10) 127 333 2/6/2023



Plotting Date:

The W beam guardrail trailing end terminal will only be used in a one-way traffic situation on the downstream traffic flow end.

ELEVATION VIEW (W Beam End Section (Flared))

W beam end section (flared) will be 12 gauge.

The cable will be \(\frac{\pi}{\pi} \), Type II, with Class A coating in conformance with AASHTO M30.

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The steel tube will meet the requirements of ASTM A500, Grade B, and will be galvanized after fabrication in accordance with the requirements of AASHTO M111.

All hardware will be galvanized in accordance with ASTM A153.

The anchor bracket, strut and yoke assembly, and bearing plate will be fabricated from steel that meets ASTM A36 Specifications. They will be galvanized after fabrication in accordance with ASTM A123.

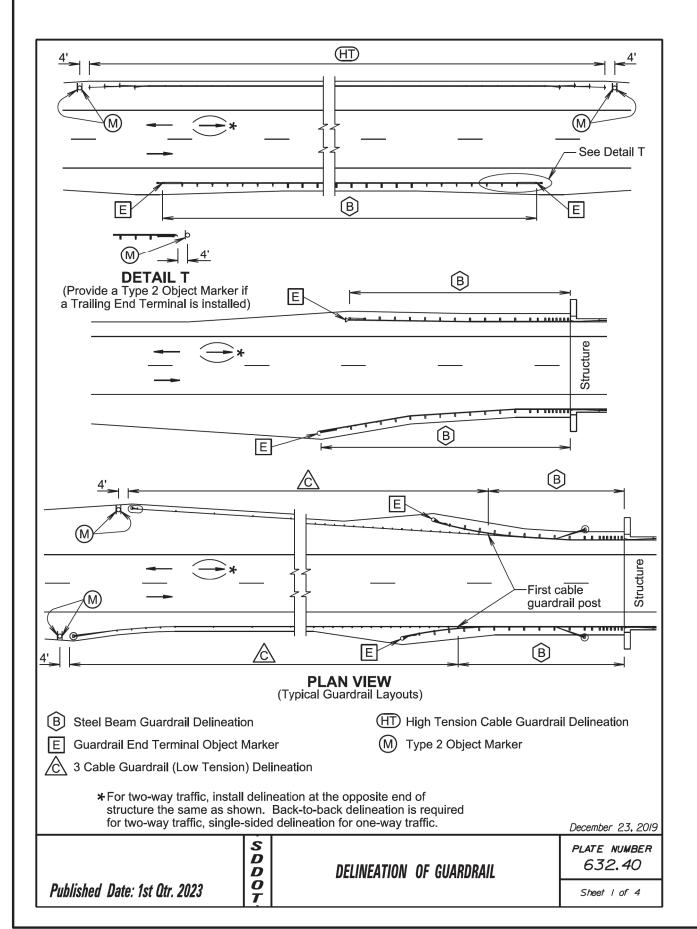
Slots in the rails will be provided as specified in the plans and by the manufacturer. A drilled hole through the rail is not allowed as a replacement for a slot. If the Contractor must create a slot, a cutting torch or plasma cutter is not allowed. The slot edges will be smooth and free of burrs or notches.

All costs for furnishing and constructing the W beam guardrail trailing end terminal including labor, equipment, materials which includes W beam rail section, two wood breakaway posts, steel tubes, strut and yoke assembly, cable assembly, bearing plate, anchor bracket, W beam end section (flared), one wood post and blockout, hardware, and incidentals will be included in the contract unit price per each for "W Beam Guardrail Trailing End Terminal". September 14, 2019

Published Date: 1st Qtr. 2023

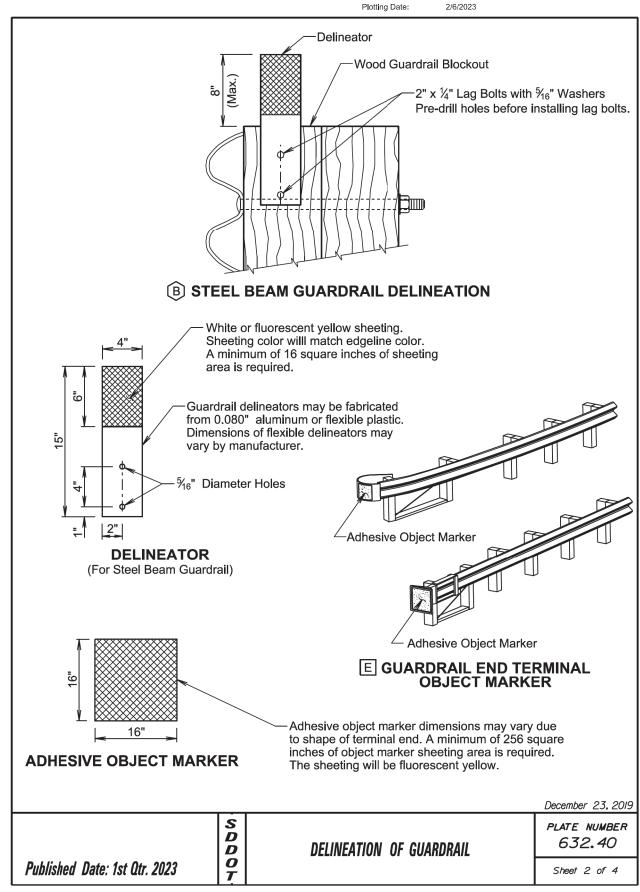
W BEAM GUARDRAIL TRAILING END TERMINAL PLATE NUMBER 630.81

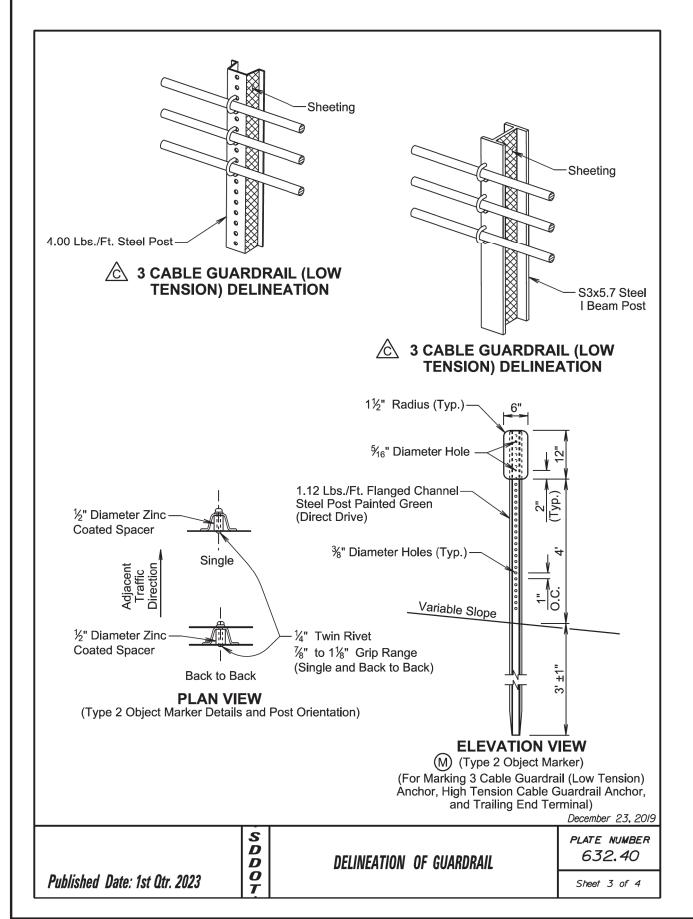
Sheet 3 of 3



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EOD BIDDING DUDDOSES ONI

STATE OF P 6403(10) 129 333

Plotting Date 2/6/2023

GENERAL NOTES:

The delineation of high tension cable guardrail will be reflective sheeting placed back to back on every other post cap or cable spacer. The sheeting will be type XI in conformance with ASTM D4956. The color of the reflective sheeting shall be the same as the nearest pavement marking.

The delineators for steel beam guardrail and sheeting on 3 cable guardrail (low tension) posts will be covered with a minimum of 16 square inches of reflective sheeting. The reflective sheeting will be type XI in conformance with ASTM D4956. Along two-way roadways the sheeting will be on both sides of the delineators and guardrail posts and will be white in color. For one-way roadways the sheeting will only be required on the side facing traffic and the color will be the same as the nearest pavement marking, yellow on the left side of the roadway and white on the right side.

When steel beam guardrail is attached to a bridge the first delineator will be attached to the post nearest the

At bridges with guardrail less than 200 feet in length, a minimum of 4 delineators will be placed in addition to the end terminal yellow object marker. The spacing between the delineators will be approximately one third of the length of the guardrail.

At bridges with quardrail 200 feet and greater in length, including bridges that have steel beam guardrail transitioning to 3 cable guardrail (low tension), the delineators will be placed at a spacing of approximately 50 feet. Delineation will extend throughout the length of the guardrail system.

Steel beam guardrail that is not attached to a bridge and is less than 200 feet in length, a minimum of 4 delineators will be placed in addition to the end terminal yellow object markers. The spacing between the delineators will be approximately one third of the length of the guardrail.

Steel beam guardrail that is not attached to a bridge and is 200 feet and greater in length, including steel beam quardrail transitioning to 3 cable quardrail (low tension), the delineators will be placed at a spacing of approximately 50 feet. Delineation will extend throughout the length of the guardrail system.

All costs for furnishing and installing single or back to back guardrail delineation on 3 cable guardrail and steel beam quardrail will be included in the contract unit price per each for "Guardrail Delineator".

All costs for furnishing and installing the reflective sheeting on the cable spacers or post caps for the high tension cable guardrail will be incidental to the respective high tension cable guardrail contract item.

An adhesive object marker will be placed on the end of the W beam guardrail or MGS end terminal. The adhesive object marker dimensions may vary due to the shape of the terminal end. A minimum of 256 square inches of object marker reflective sheeting area is required. The reflective sheeting will be fluorescent yellow type XI sheeting in conformance with ASTM D4956. All costs for furnishing and installing the adhesive object marker will be incidental to various contract items.

A type 2 object marker will be placed adjacent to the 3 cable guardrail (low tension) anchor, high tension cable quardrail anchor, and trailing end terminal at the location noted on sheet 1 of this standard plate. The type 2 object marker (6" x 12") will have fluorescent yellow type XI sheeting in conformance with ASTM D4956. All costs for furnishing and installing the type 2 object marker including the steel post, 6" x 12" reflective panel, and hardware will be included in the contract unit price per each for "Type 2 Object Marker" for single-sided and "Type 2 Object Marker Back to Back" for back to back type 2 object markers.

December 23, 2019

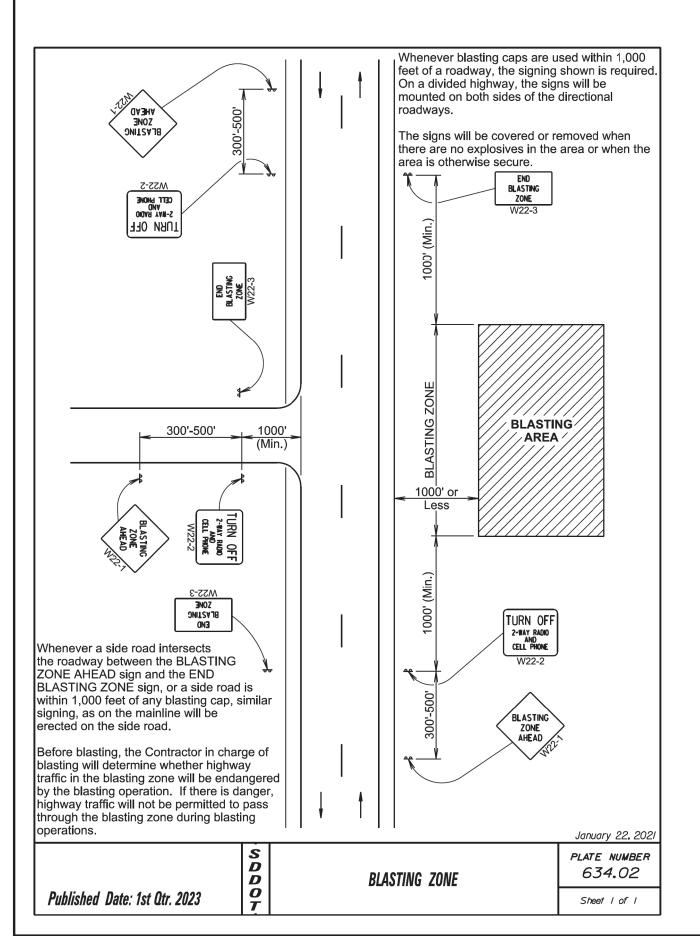
D D O T

DELINEATION OF GUARDRAIL

PLATE NUMBER 632.40

Published Date: 1st Qtr. 2023

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 2/6/2023

Posted	Spacing of	Spacing of
	Advance Warning	Channelizing
Prior to	Signs	Devices
Work	(Feet)	(Feet)
(M.P.H.)	(A)	(G)
0 - 30	200	25
35 - 40	350	25
45	500	25
50	500	50
55	750	50
60 - 65	1000	50

■ Flagger

■ Channelizing Device

For low-volume traffic situations with short work zones on straight roadways where the flagger is visible to road users approaching from both directions, a single flagger may be used.

The ROAD WORK AHEAD and the END ROAD WORK signs may be omitted for short duration operations (1 hour or less).

For tack and/or flush seal operations, when flaggers are not being used, the FRESH OIL sign (W21-2) will be displayed in advance of the liquid asphalt areas.

Flashing warning lights and/or flags may be used to call attention to the advance warning signs.

The channelizing devices will be drums or 42" cones.

Channelizing devices are not required along the centerline adjacent to work area when pilot cars are utilized for escorting traffic through the work area.

GSO-5 KOAD WORK END

Channelizing devices and flaggers will be used at intersecting roads to control intersecting road traffic as required.

The buffer space should be extended so that the two-way traffic taper is placed before a horizontal or vertical curve to provide adequate sight distance for the flagger and queue of stopped vehicles.

The length of A may be adjusted to fit field conditions.

Warning sign sequence in opposite direction same as below. XXX FEET (Optional) ROAD ROAD WORK January 22, 2021 PLATE NUMBER

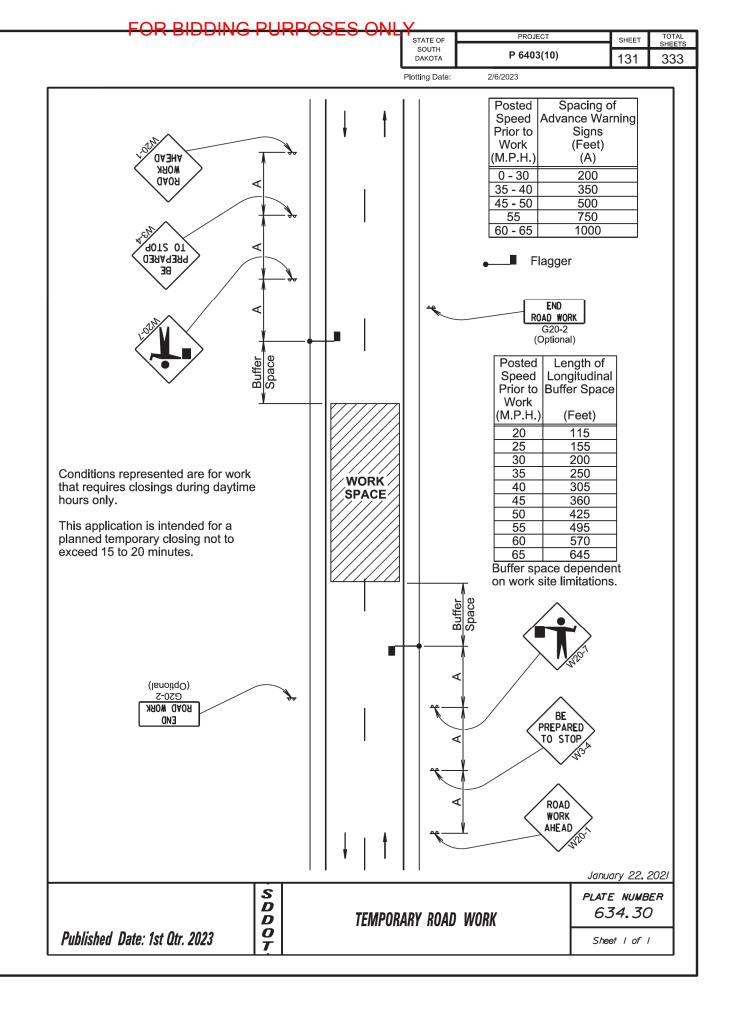
D D O T

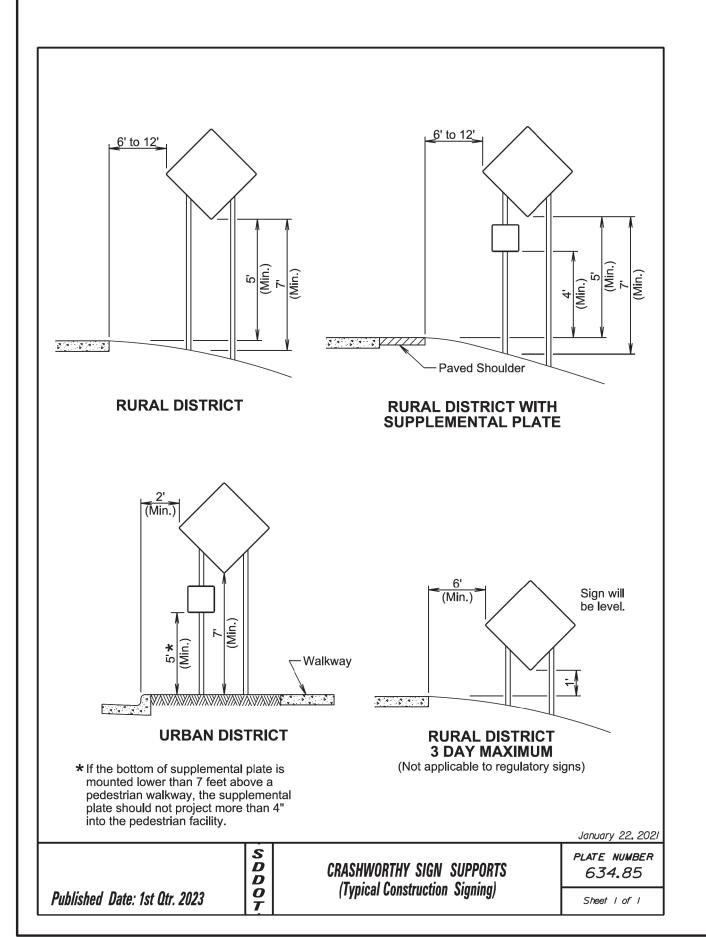
LANE CLOSURE WITH FLAGGER PROVIDED

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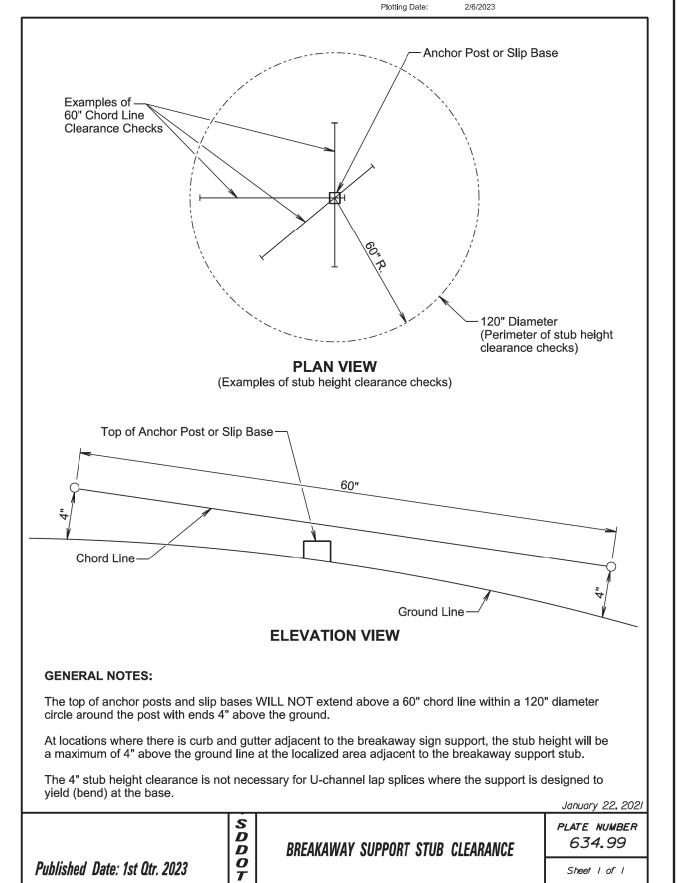
Spacing of Posted **AHE** AD Advance Warning Speed MOKK Prior to Signs MOAD AHEAD Work (Feet) MACHINERY (M.P.H. (A) DAOR 200 0 - 30 35 - 40 350 45 - 50 500 CRAVEL 55 750 **TOOSE** 60 - 65 1000 END ** ROAD WORK G20-2 (Optional) ____ Flagger Type 1 or Type 2 Barricades WINDROWING OR LAYDOWN OPERATIONS *Need for flagger to be determined by the Engineer. **Signs not required if activity is 5.9" within a construction zone. 55" Buffer Space 66.9" Type 1 Barricades will only be used on roadways with posted speed limits of 48.0" across sides 1.1" Radius, 40 mph or less. 0.3" Border, 0.5" Indent, Black on Orange; "WINDROW", E Mod 2K Work areas which are duplicated less than one mile apart, may be classified as one work area for purposes of sign installation. LOOSE Type 1 or 2 Barricades, KEEP RIGHT signs GRAVEL (as required) will mark each windrow left overnight. ROAD MACHINERY MINDROW AHEAD / 4"x4" delineators placed back-to-back Use R4-7 sign WINDROW will be installed along the outside only when windrow is o Z-4A windrow is on edge of roadway at 264 feet maximum center of spacing on tangent on alternating roadway sides and 132 feet spacing on outside of curves greater than 3 degrees The length of A may be adjusted to fit field conditions. WINDROW ROAD (IsnotiqO) WORK C50-5 ROAD WORK END November 19, 2021 S D D O T PLATE NUMBER *634.27* WINDROWING OR LAYDOWN OPERATION Published Date: 1st Qtr. 2023 Sheet I of I





EOD RIDDING DUDDOSES ONLY

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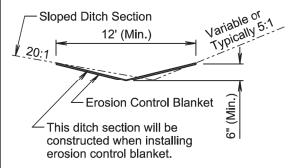
BREAKAWAY SUPPORT STUB CLEARANCE

634.99

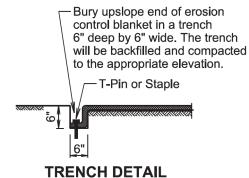
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STANDARD DITCH SECTION



SLOPED DITCH SECTION

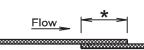


GENERAL NOTES:

Median 20' (Min.) 15' 15' 10' Area will be excavated **Erosion** Control 15' 15' Blanket

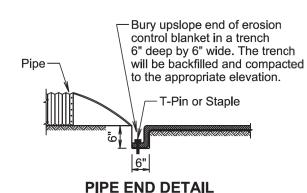
The median will be shaped to the limits shown in this detail where the erosion control blanket will be placed.

MEDIAN SECTION



- *Use a 4" (Min.) overlap wherever two widths of erosion control blanket are applied side by side.
- *Use a 6" (Min.) overlap wherever one roll of erosion control blanket ends and another begins.

OVERLAP DETAIL



Prior to placement of the erosion control blanket, the areas will be properly prepared, shaped, seeded, and

Erosion control blanket will be unrolled in the direction of the flow of water when placed in ditches and on slopes. The upslope end of the erosion control blanket will be buried in a trench 6" wide by 6" deep. There will be at least a 6" overlap wherever one roll of erosion control blanket ends and another begins, with the upslope erosion control blanket placed on top of the downslope erosion control blanket.

The erosion control blanket will be pinned to the ground according to the manufacturer's installation recommendations.

After the placement of the erosion control blanket, the Contractor will fine grade along all edges of the blanket to maintain a uniform slope adjacent to the blanket and level any low spots which might prevent uniform and unrestricted flow of side drainage directly onto the erosion control blanket.

All ditch sections will be shaped when installing the erosion control blanket. All costs for shaping the ditches will be incidental to the contract unit price per foot for "Shaping for Erosion Control Blanket".

February 14, 2020

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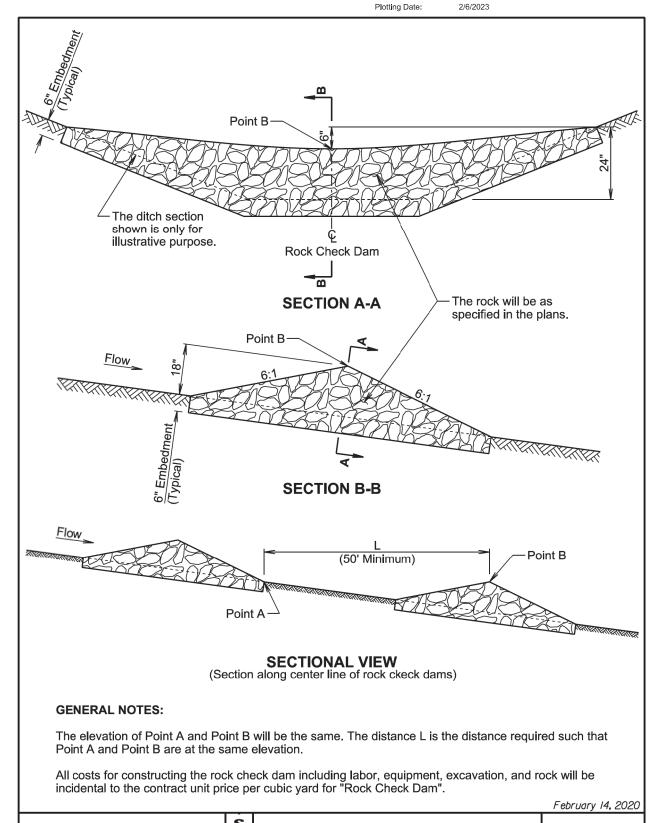
EROSION CONTROL BLANKET

PLATE NUMBER 734.01

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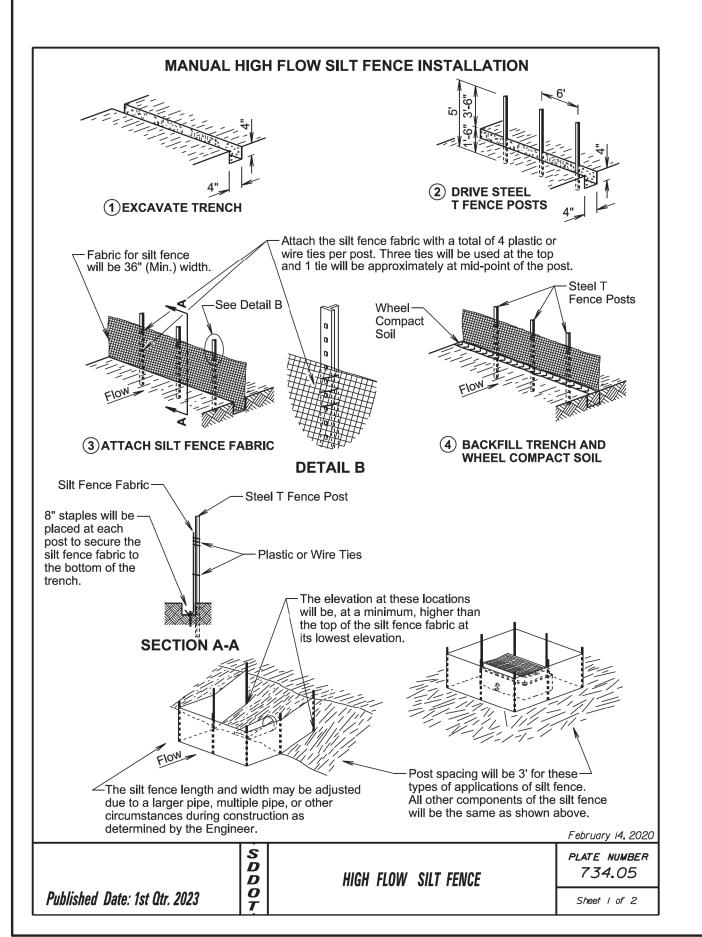
S ROCK CHECK DAM D O T

Published Date: 1st Qtr. 2023

PLATE NUMBER 734.03

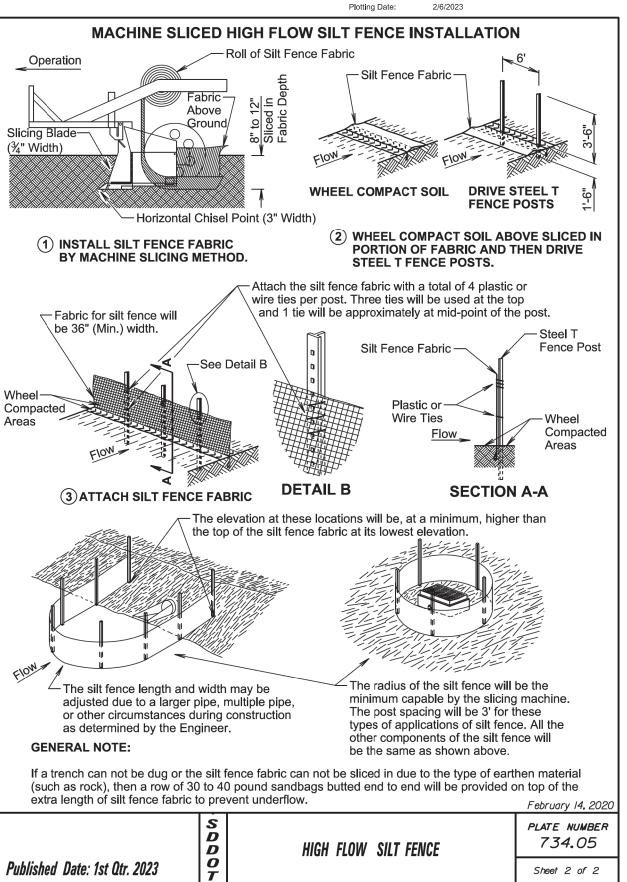
Sheet I of I

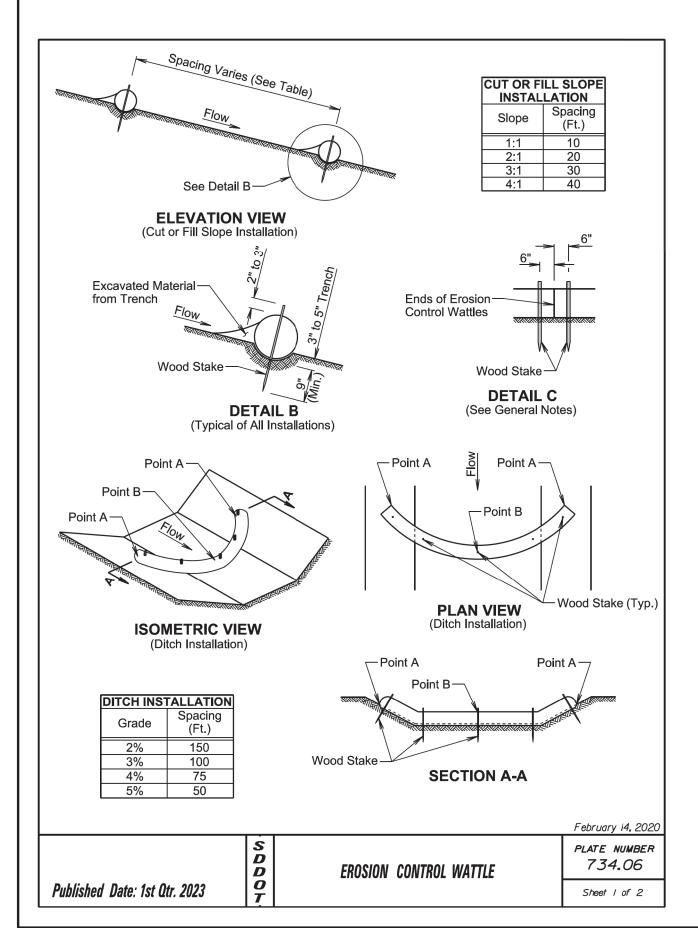
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GENERAL NOTES:

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At cut or fill slope installations, wattles will be installed along the contour and perpendicular to the water flow.

At ditch installations, point A must be higher than point B to ensure that water flows over the wattle and not around the ends.

The Contractor will dig a 3" to 5" trench, install the wattle tightly in the trench so that daylight can not be seen under the wattle, and then compact the soil excavated from the trench against the wattle on the uphill side. See Detail B.

The stakes will be 1"x2" or 2"x2" wood stakes, however, other types of stakes such as rebar may be used only if approved by the Engineer. The stakes will be placed 6" from the ends of the wattles and the spacing of the stakes along the wattles will be 3' to 4'.

Where installing running lengths of wattles, the Contractor will butt the second wattle tightly against the first and will not overlap the ends. See Detail C.

The Contractor and Engineer will inspect the erosion control wattles in accordance with the storm water permit. The Contractor will remove, dispose, or reshape the accumulated sediment when necessary as determined by the Engineer.

Sediment removal, disposal, or necessary shaping will be as directed by the Engineer. All costs for removing accumulated sediment, disposal of sediment, and necessary shaping will be incidental to the contract unit price per cubic yard for "Remove Sediment".

All costs for furnishing and installing the erosion control wattles including labor, equipment, and materials will be incidental to the contract unit price per foot for the corresponding erosion control wattle contract item.

All costs for removing the erosion control wattle from the project including labor, equipment, and materials will be incidental to the contract unit price per foot for "Remove Erosion Control Wattle".

February 14, 2020

S D D EROSION CONTROL WATTLE

PLATE NUMBER 734.06

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GENERAL NOTES:

Where practical, surface roughening will be done on slopes 3:1 and steeper and on slopes deemed necessary by the Engineer.

The equipment used for surface roughening will be equipped with tracks that are capable of creating ridges in the soil that are perpendicular to the slope. The final condition of the surface roughening will be approved by the Engineer.

Measurement for surface roughening will be to the nearest tenth of an acre.

All costs associated with surface roughening including labor, equipment, and materials will be incidental to the contract unit price per acre for "Surface Roughening".

February 14, 2020

PLATE NUMBER
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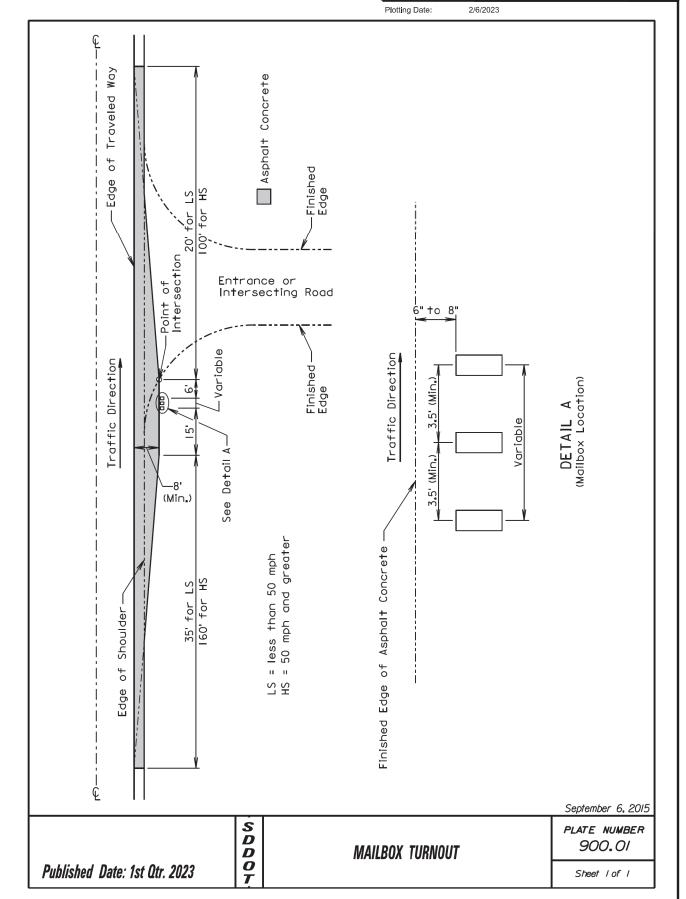
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FOR BIDDING PURPOSES ONLY

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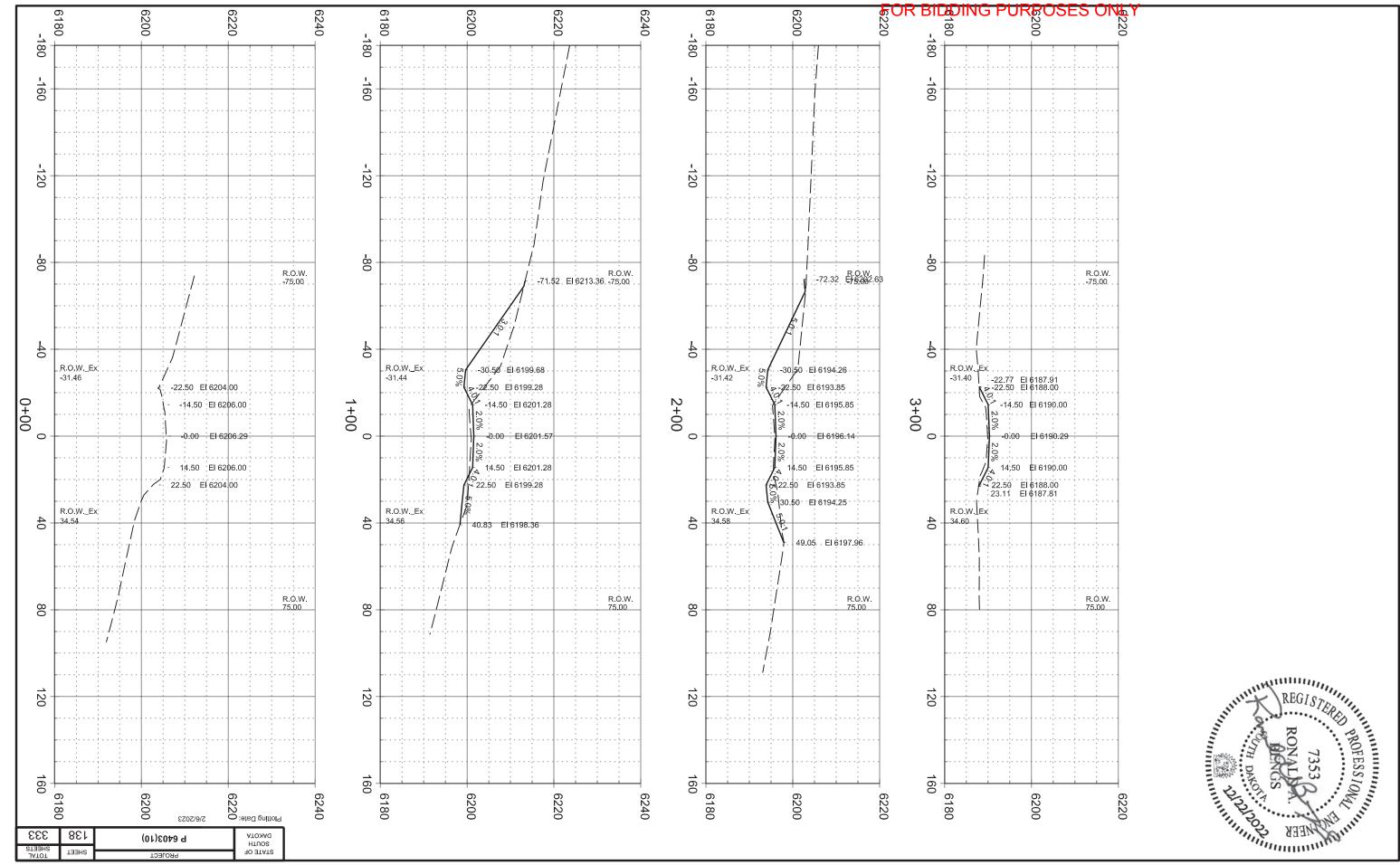
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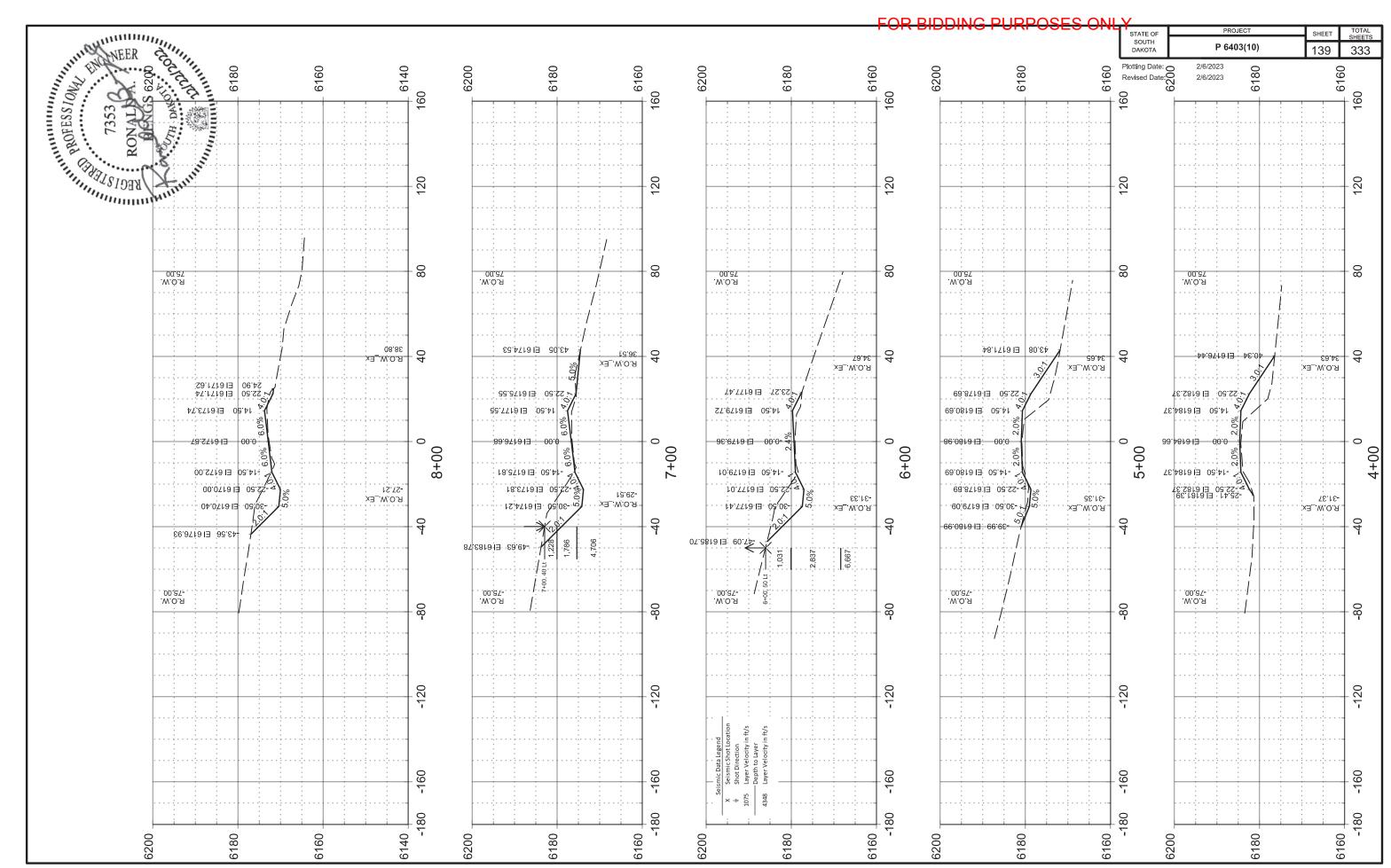


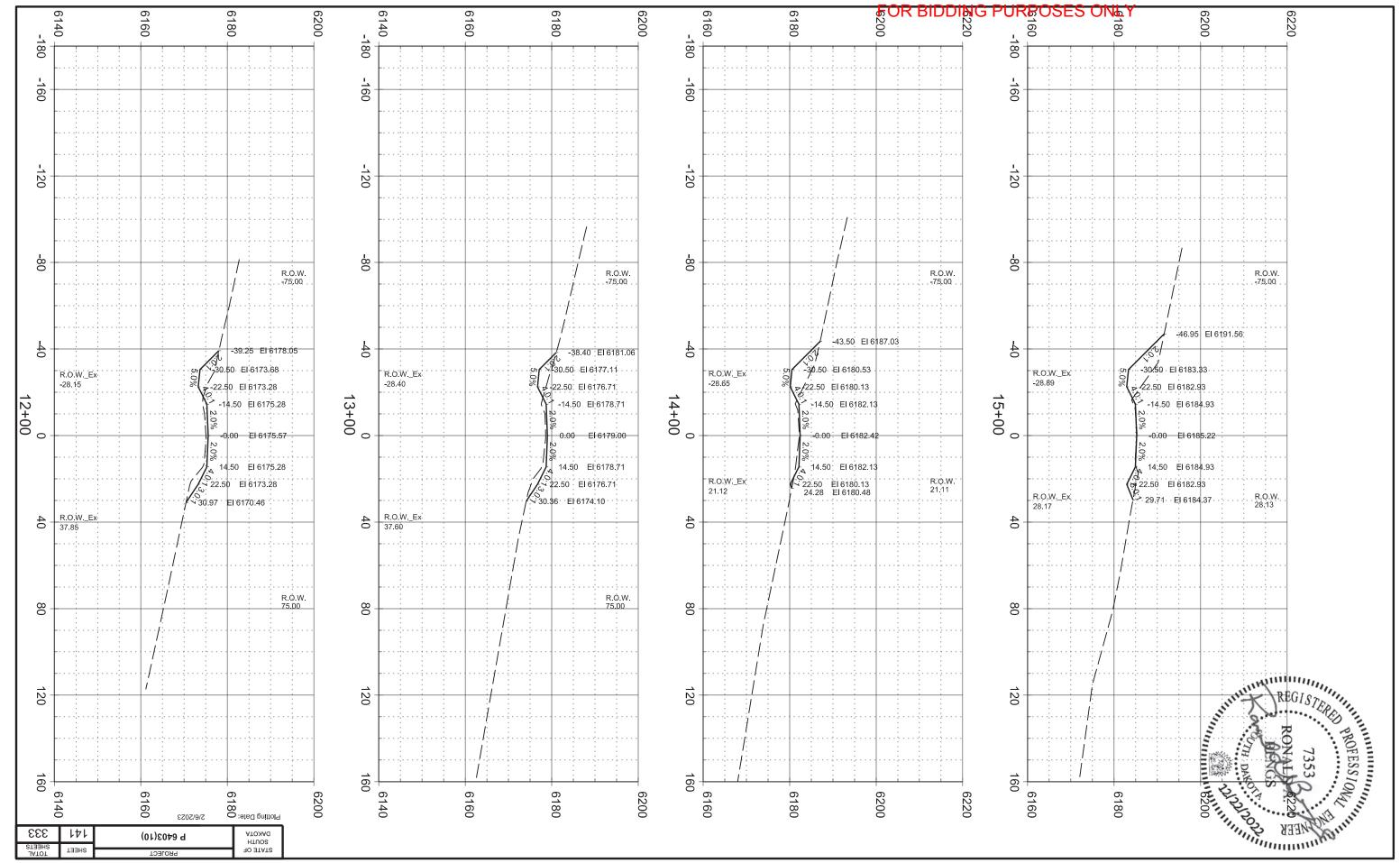
 $-\frac{3}{8}$ " - 16 x $\frac{3}{4}$ " Hex Bolt 2 Washers, (Stove Bolt) 2 Washers, I Washer I Lockwasher I Nut I Nut #8-32 X 3/4" Slotted RD, HD. Bolt (Stove Bolt) 2 Washers, I Washer ------Pla**t**form Bracket 3/8" - 16 x 5" Hex Bolt — 2 Washers, I Lockwasher 4" x 4" Square ----or 4" Round I Nut Wood Post -------Shelf ===== Bracket $\frac{3}{8}$ " - 16 × 5" He× Bolt 4" x 4" Square 2 Washers, I Lockwasher **:**::::: or 4" Round Wood Post H = 39" to 48" Mail Stop Surface (As established by U.S. Postal Service) Groundline SPACING FOR MULTIPLE POST INSTALLATION The post support assemblies provided should be consistant throughout the project. Single and double mailboxes may be in any sequence. Post support assemblies shall be one from the approved products list, a 4"x4" or 4" round wood post, or an alternate post support assembly that meets the test level 3 crash testing requirements of NCHRP 350 or MASH. Alternate mailbox support assemblies shall be approved by the Engineer prior to installation. The Contractor shall provide the Engineer written certification that the mailbox support assembly has met the crash testing requirements and will be installed in accordance with the manufacturer's installation instructions. September 6, 2013 PLATE NUMBER D 900.02 SINGLE AND DOUBLE MAILBOX ASSEMBLIES D 0 Published Date: 1st Qtr. 2023 Sheet I of I

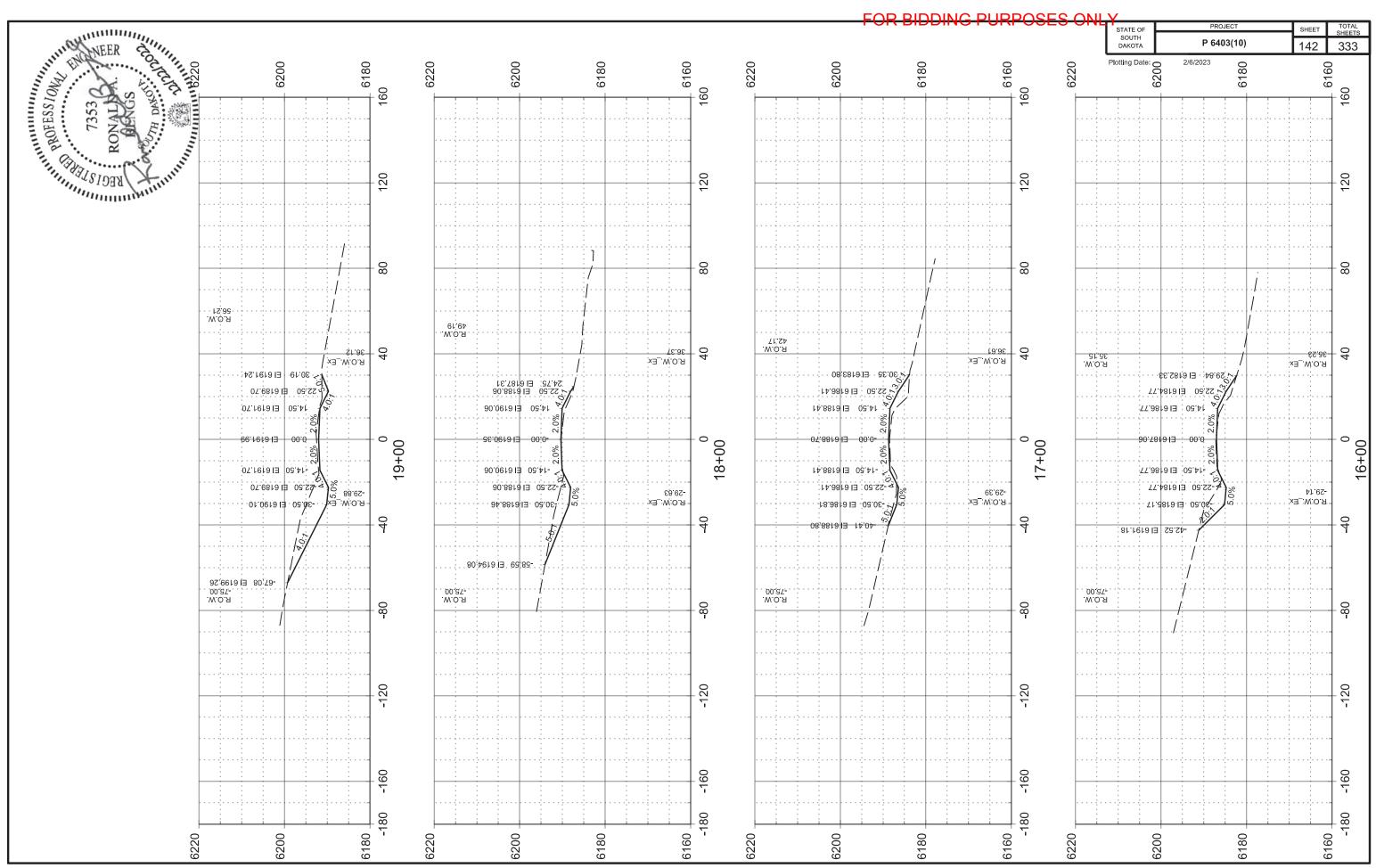
STATE OF SHEET P 6403(10) 137 333 DAKOTA 2/6/2023 Plotting Date: 20 LENGTH ┥╓╩╢┝ BRACKET Assemblies) " 9½ I " 9½ TYPICAL OCATION SHELF Asse (Double .. 9½ I .. 9½ PLATFORM All Assemblies) \bigoplus March 31, 2000 S D D O T PLATE NUMBER 900.03 MAILBOX SUPPORT HARDWARE Published Date: 1st Qtr. 2023 Sheet I of I

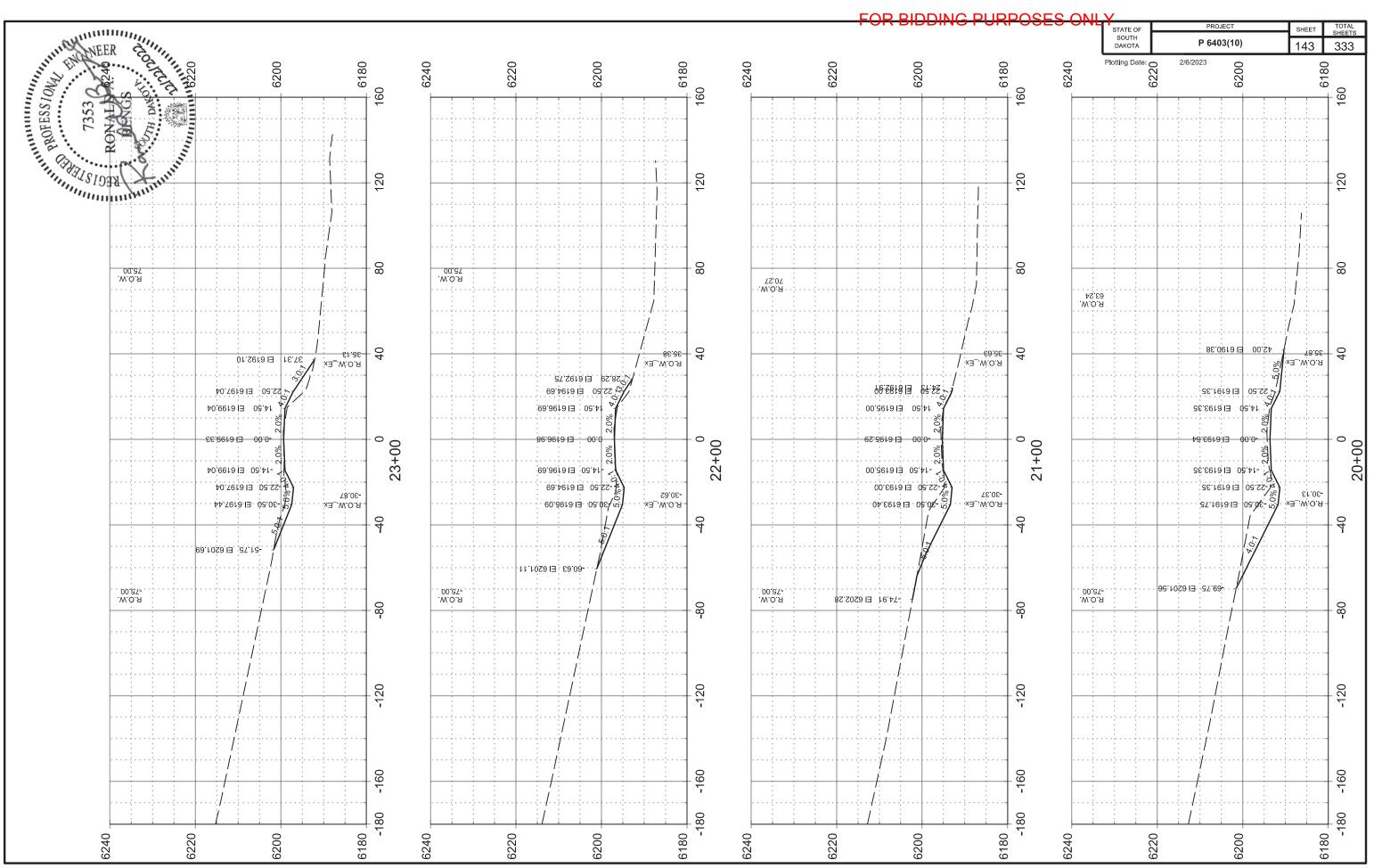
EOD RIDDING DUDDOSES ONLY

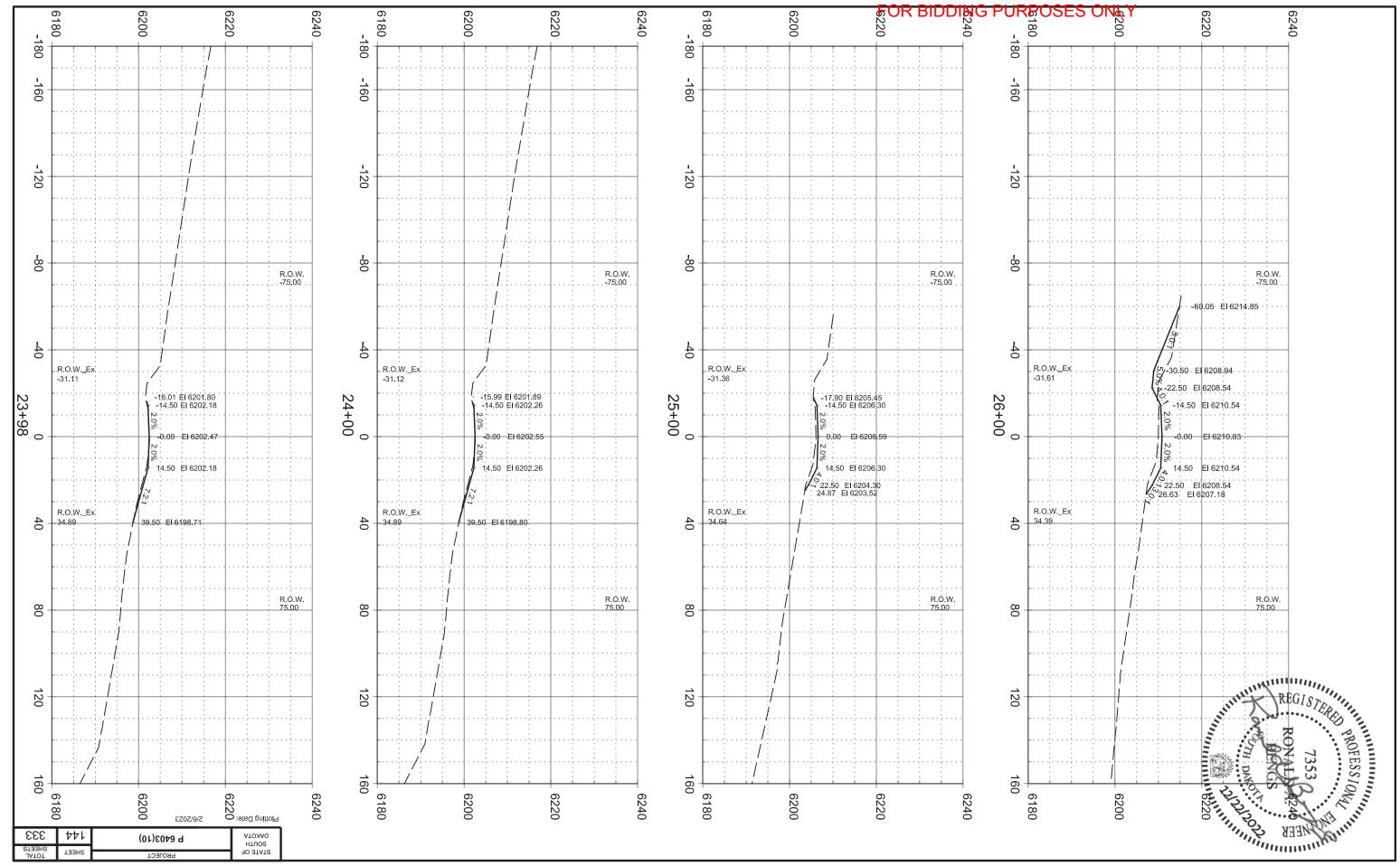


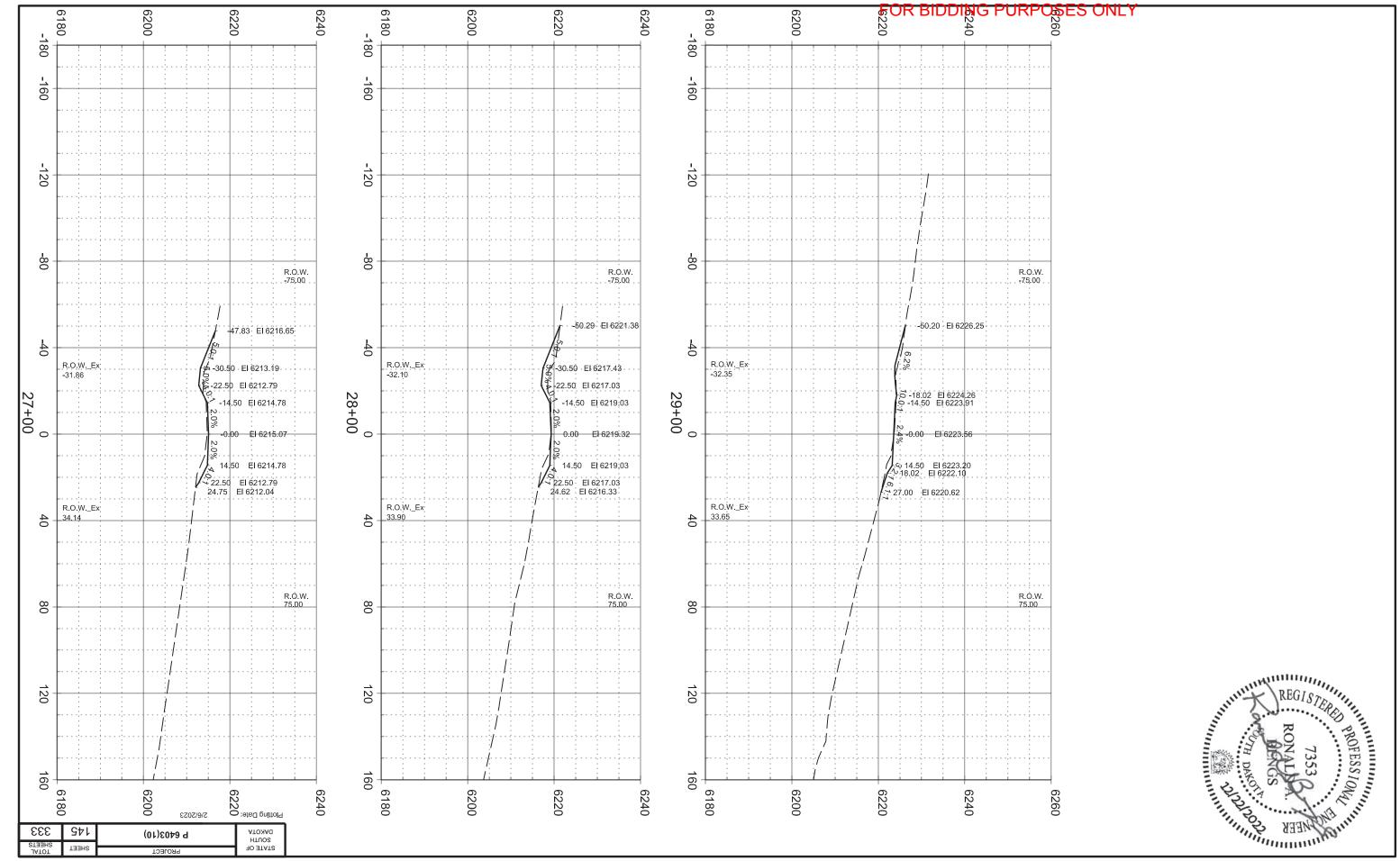


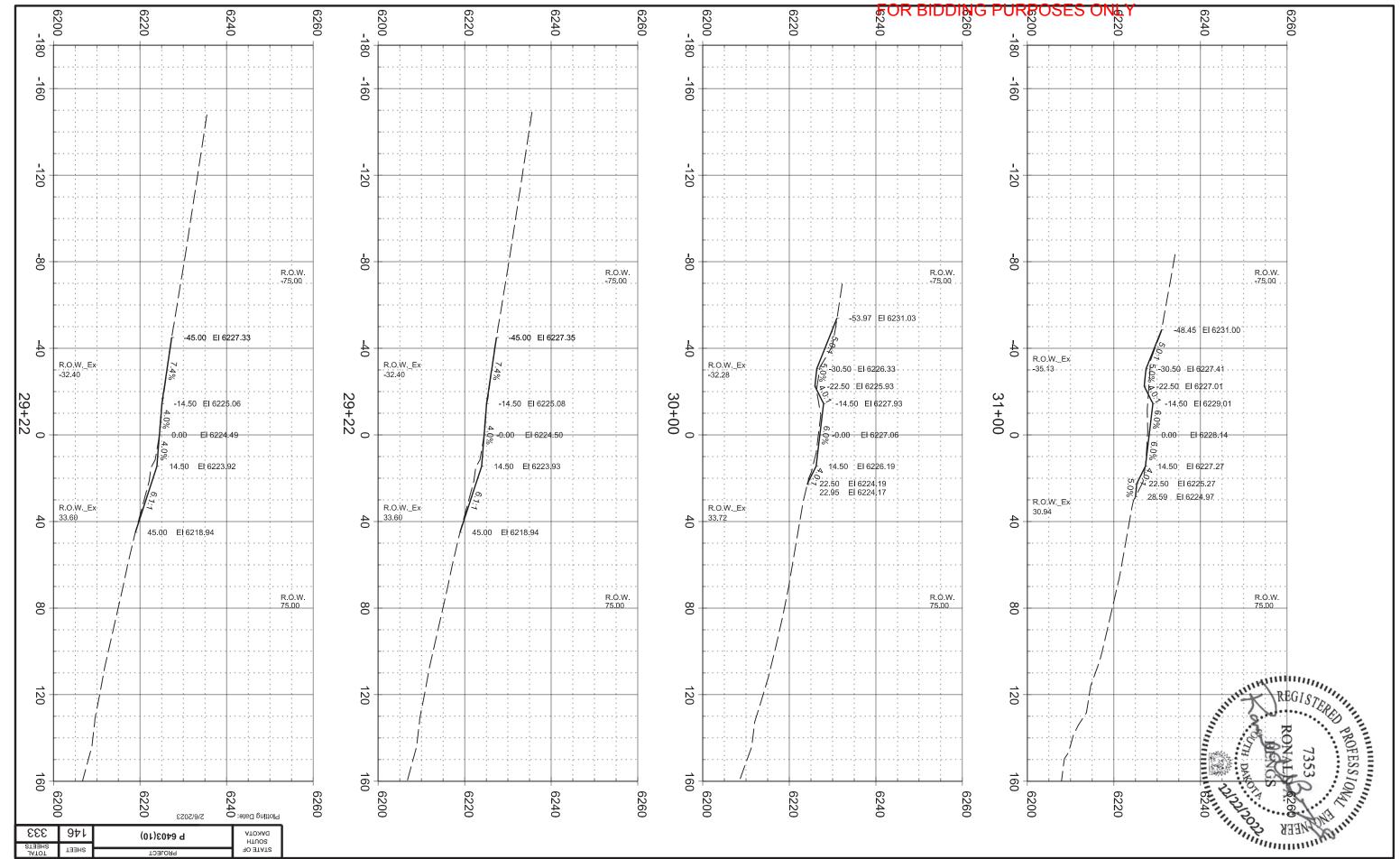


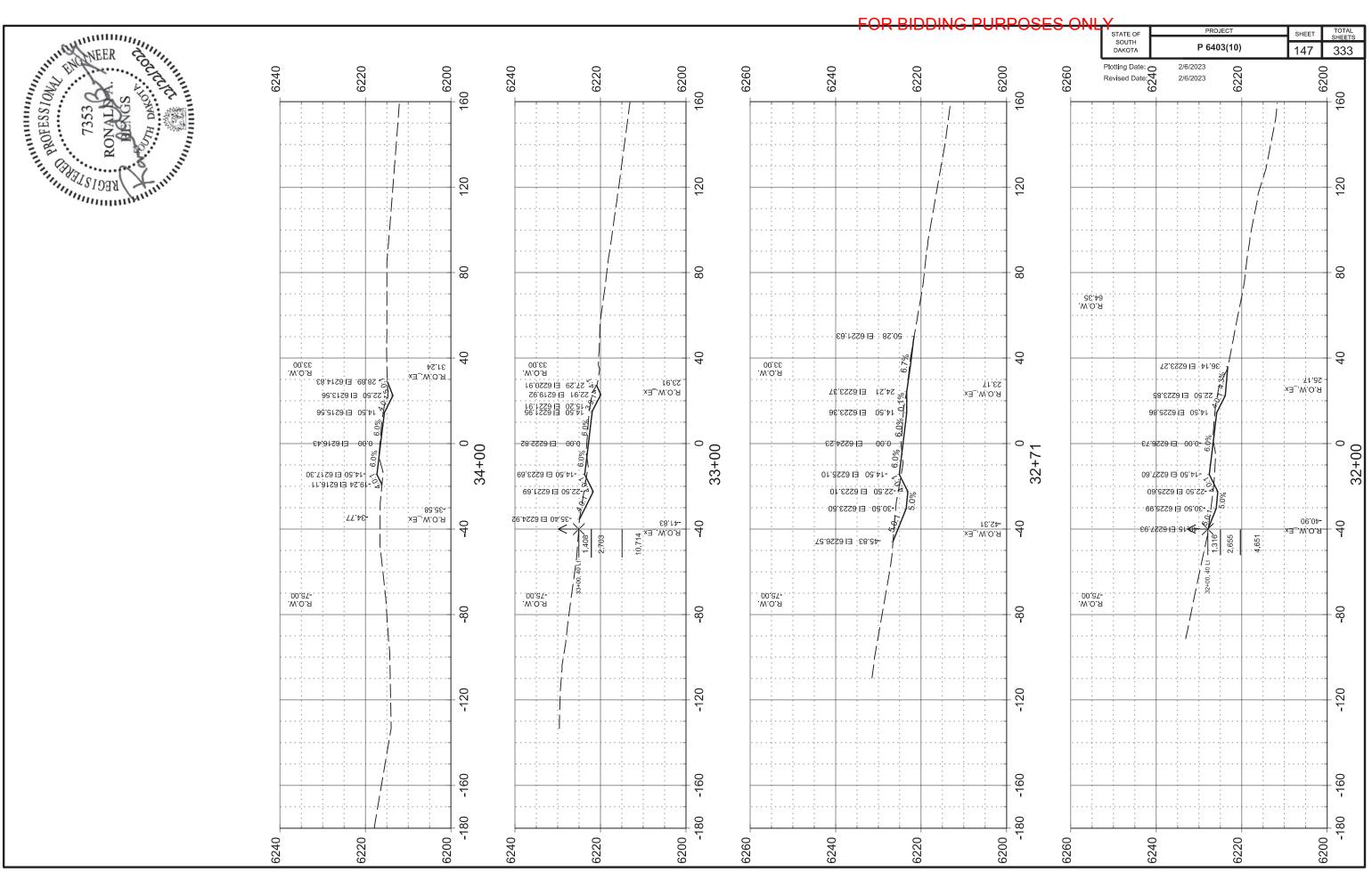


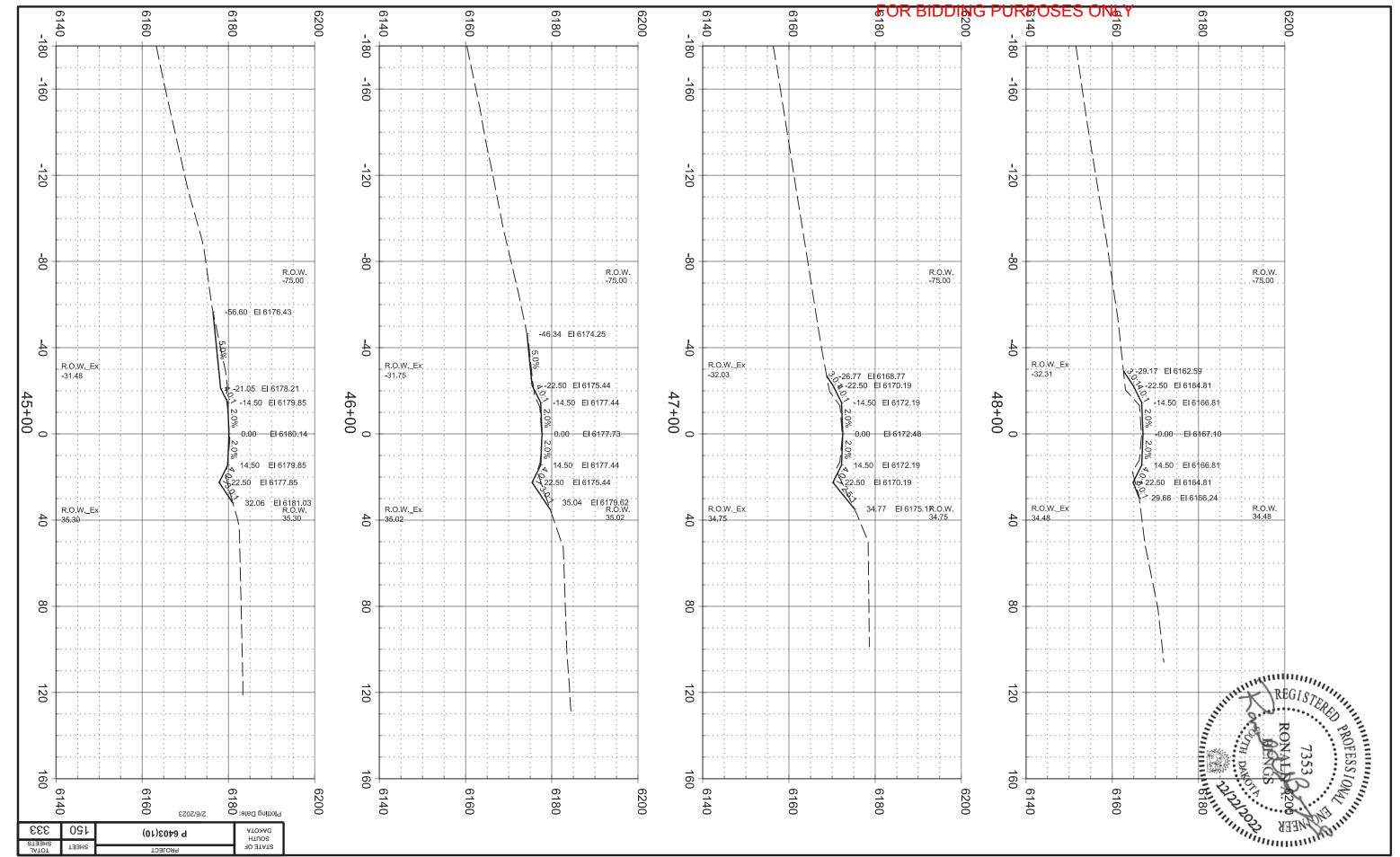


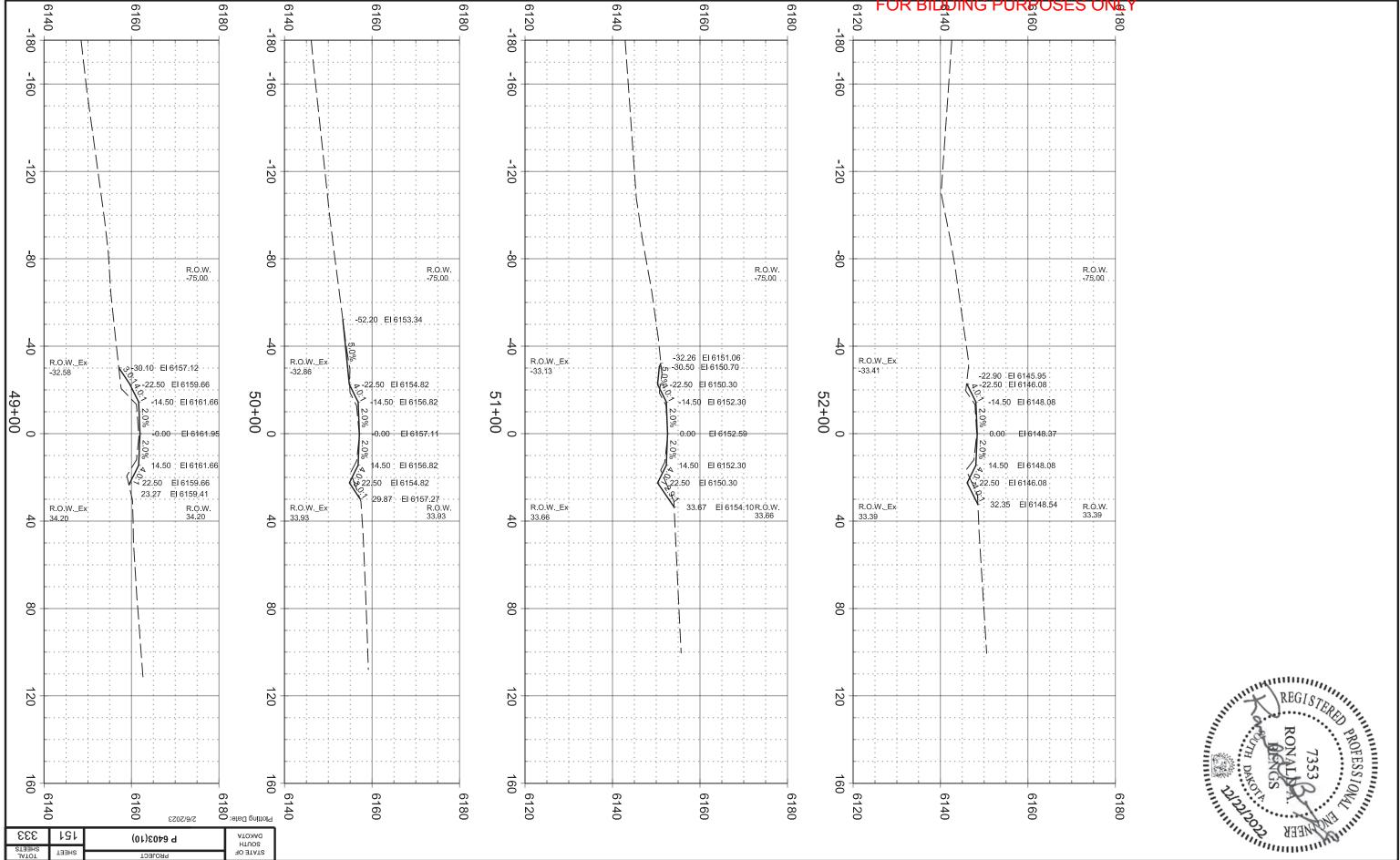


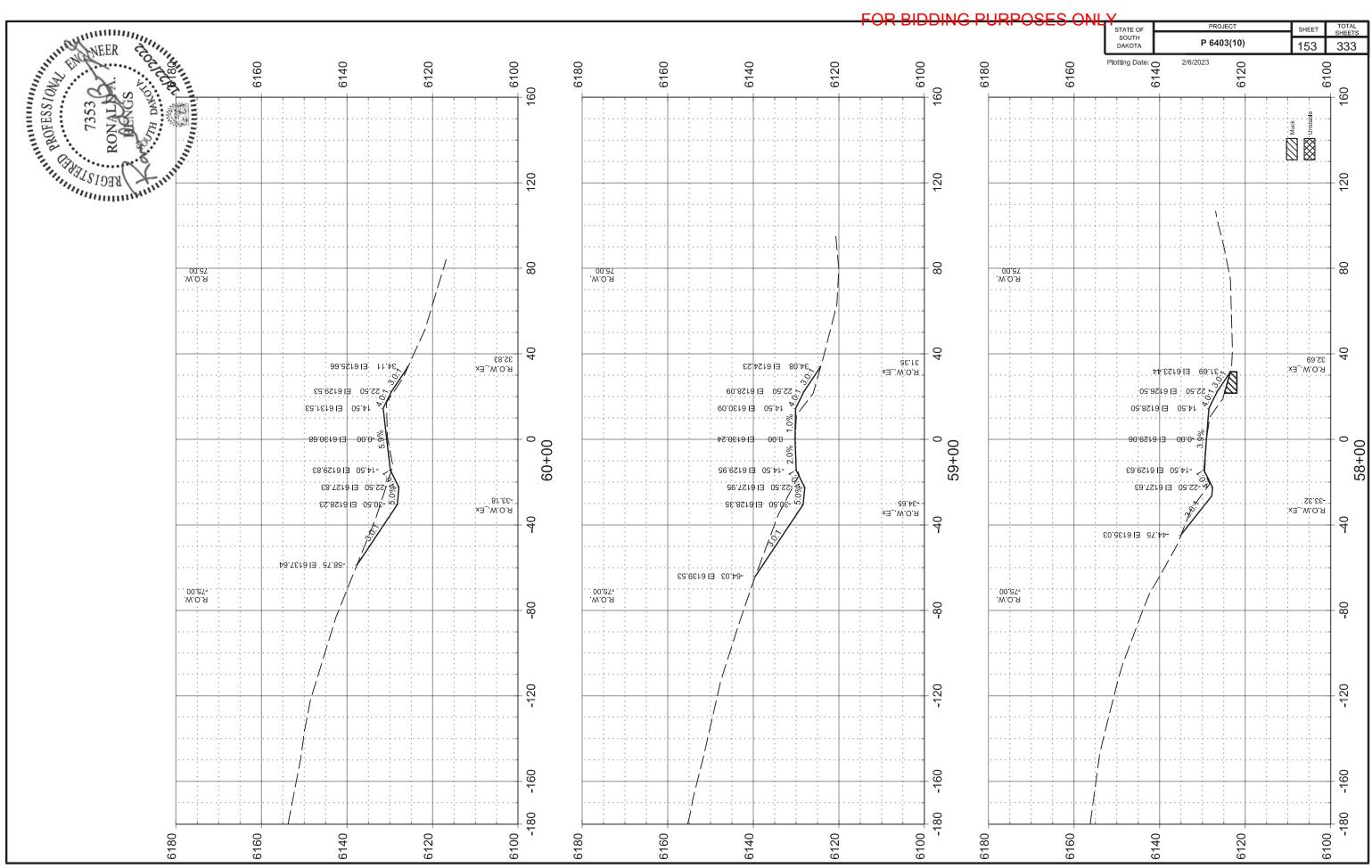


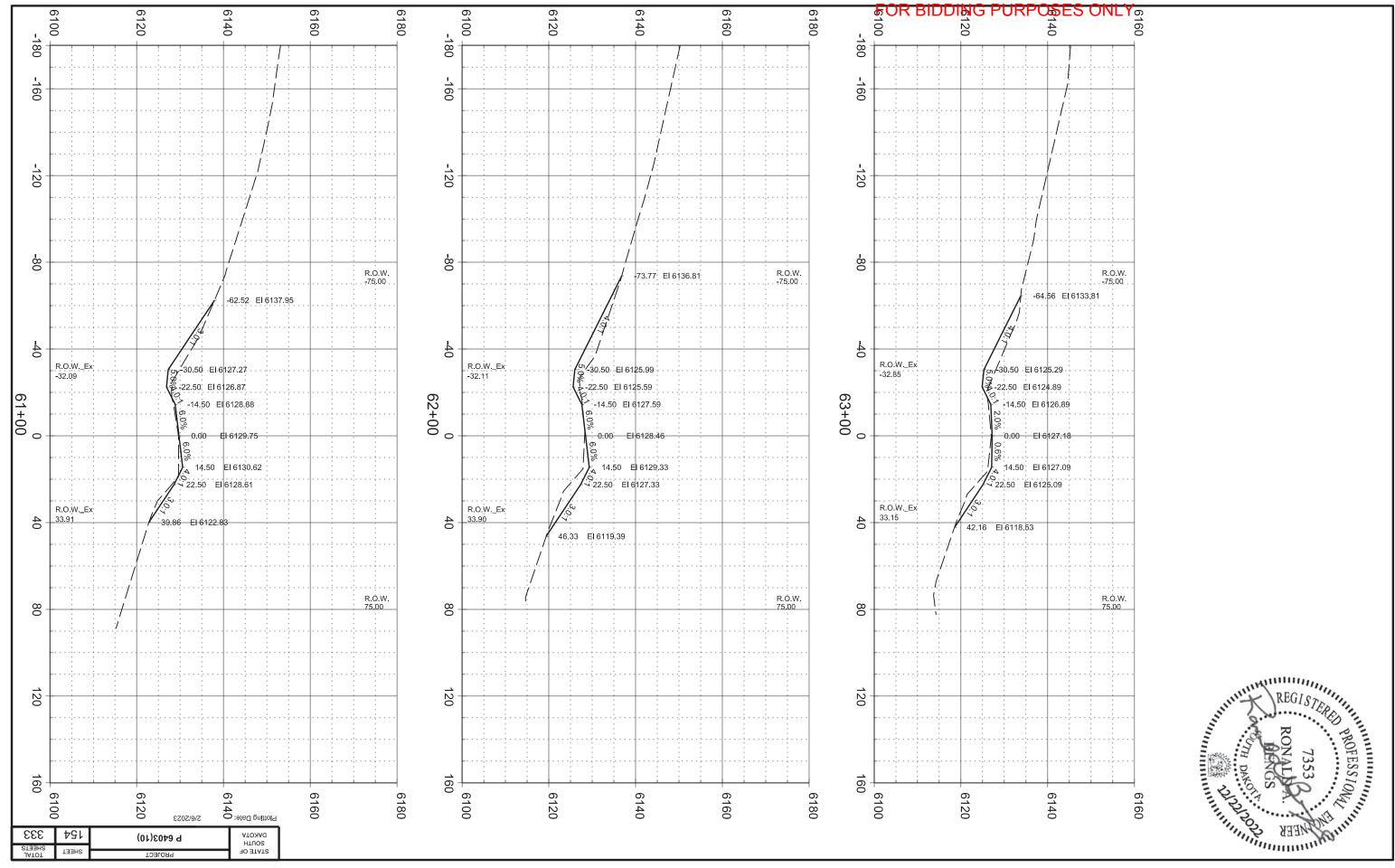


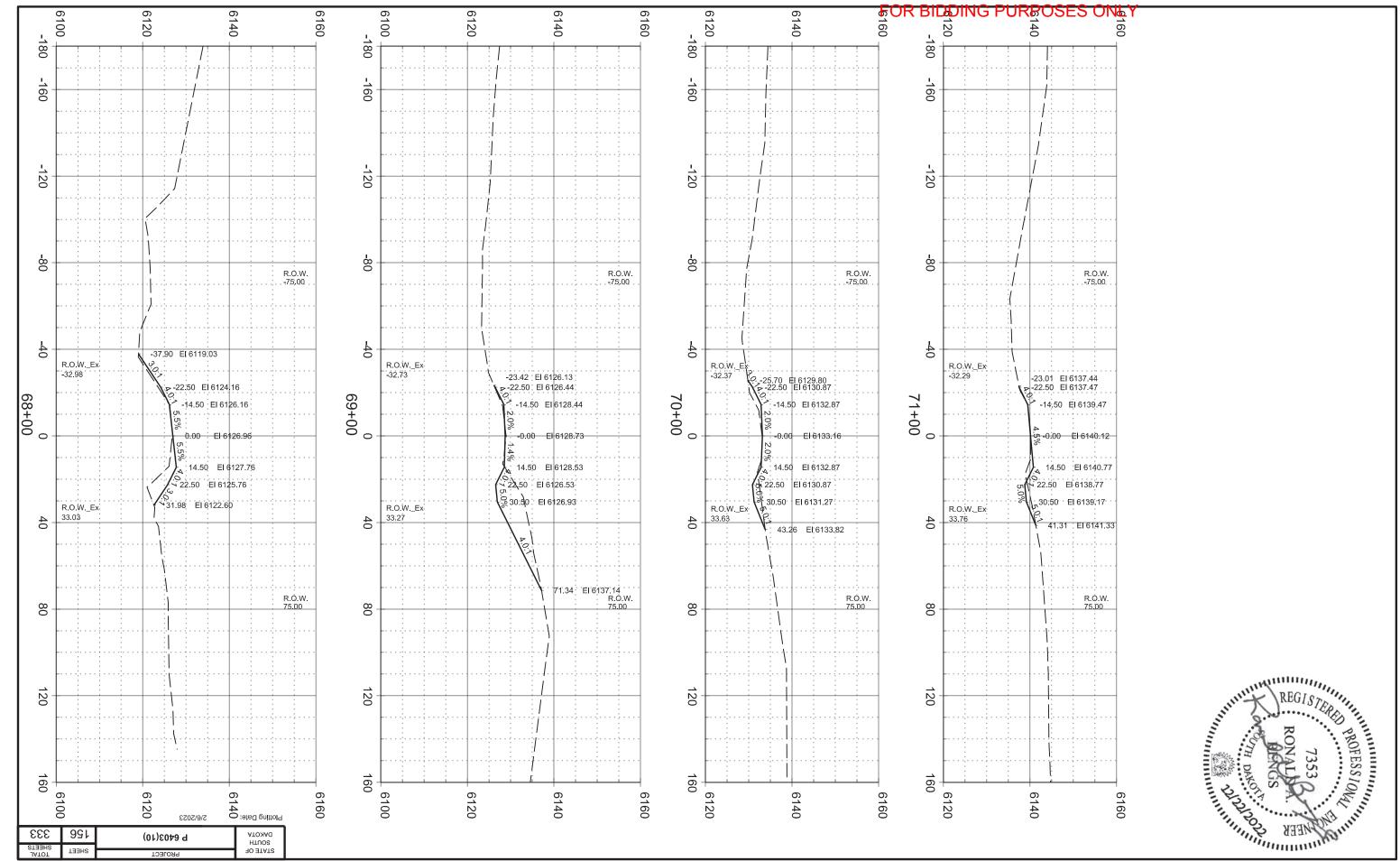


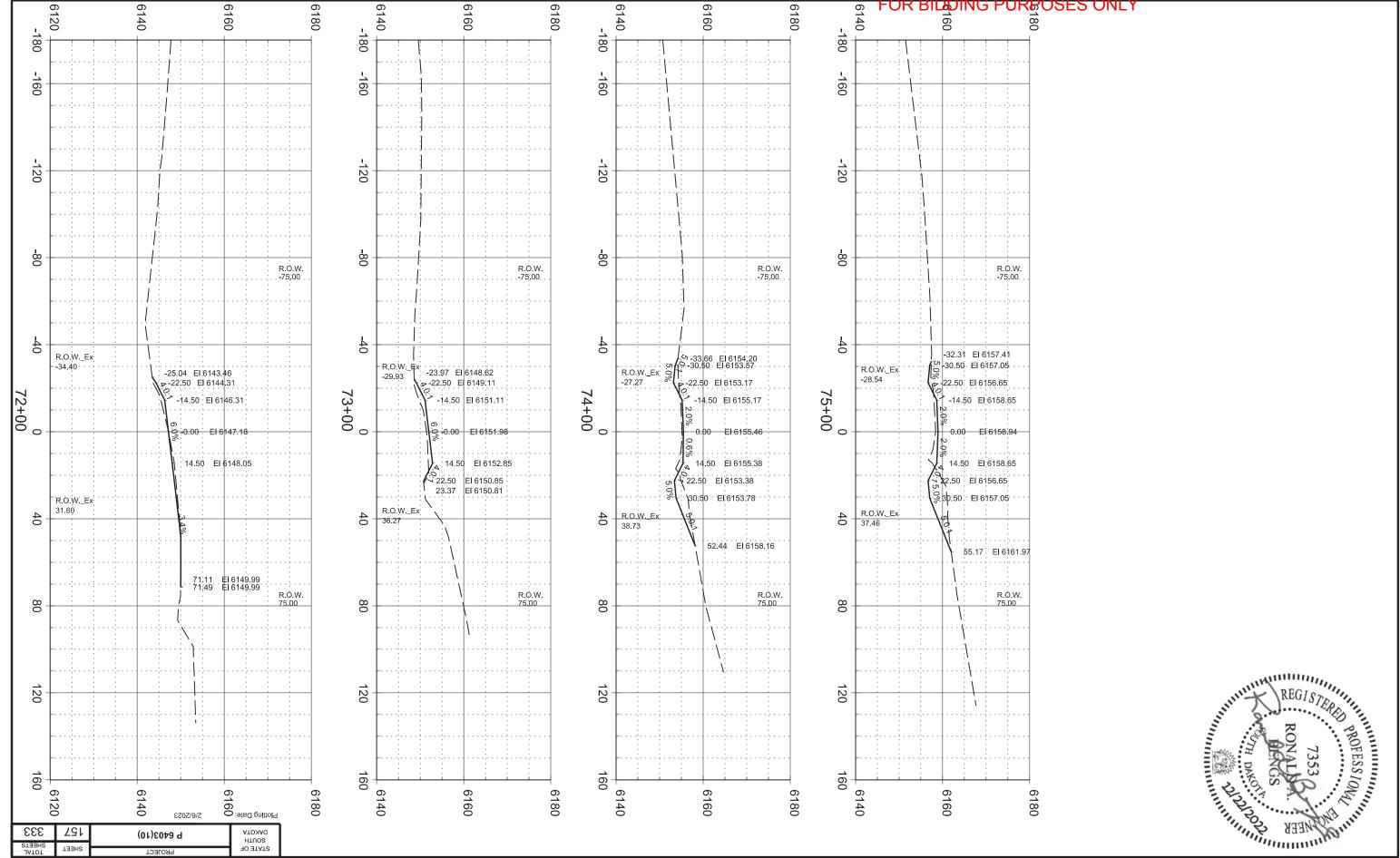


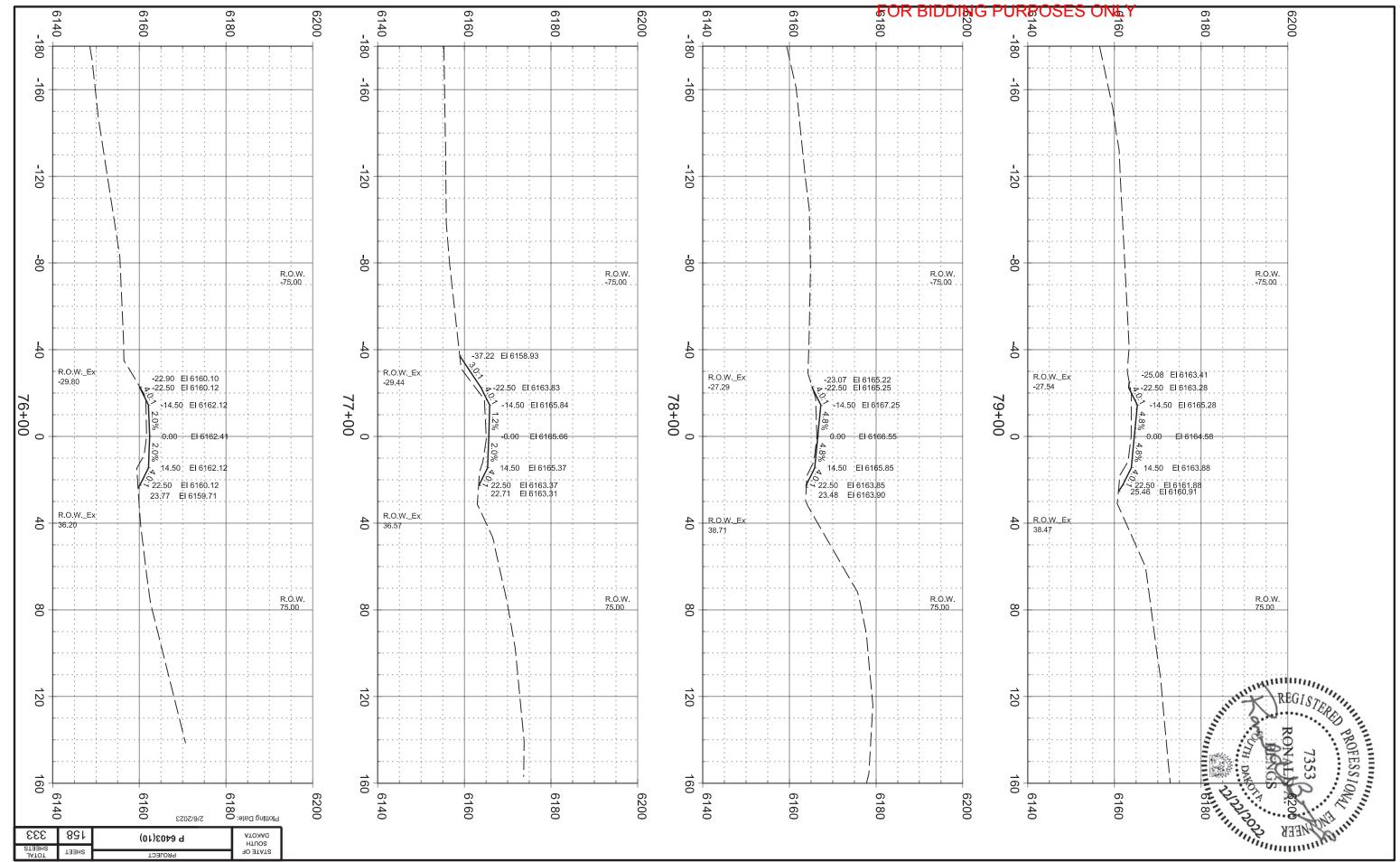


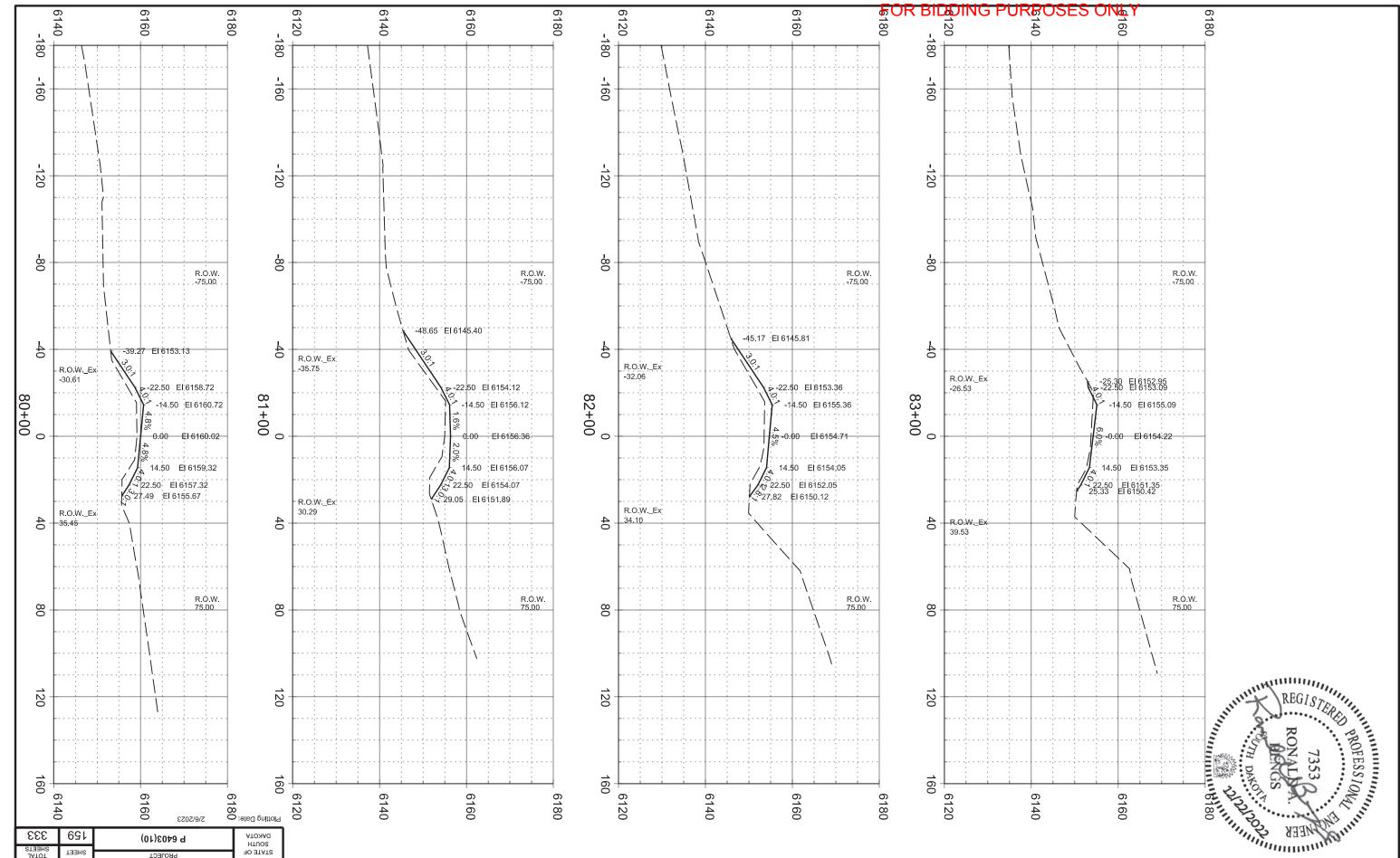


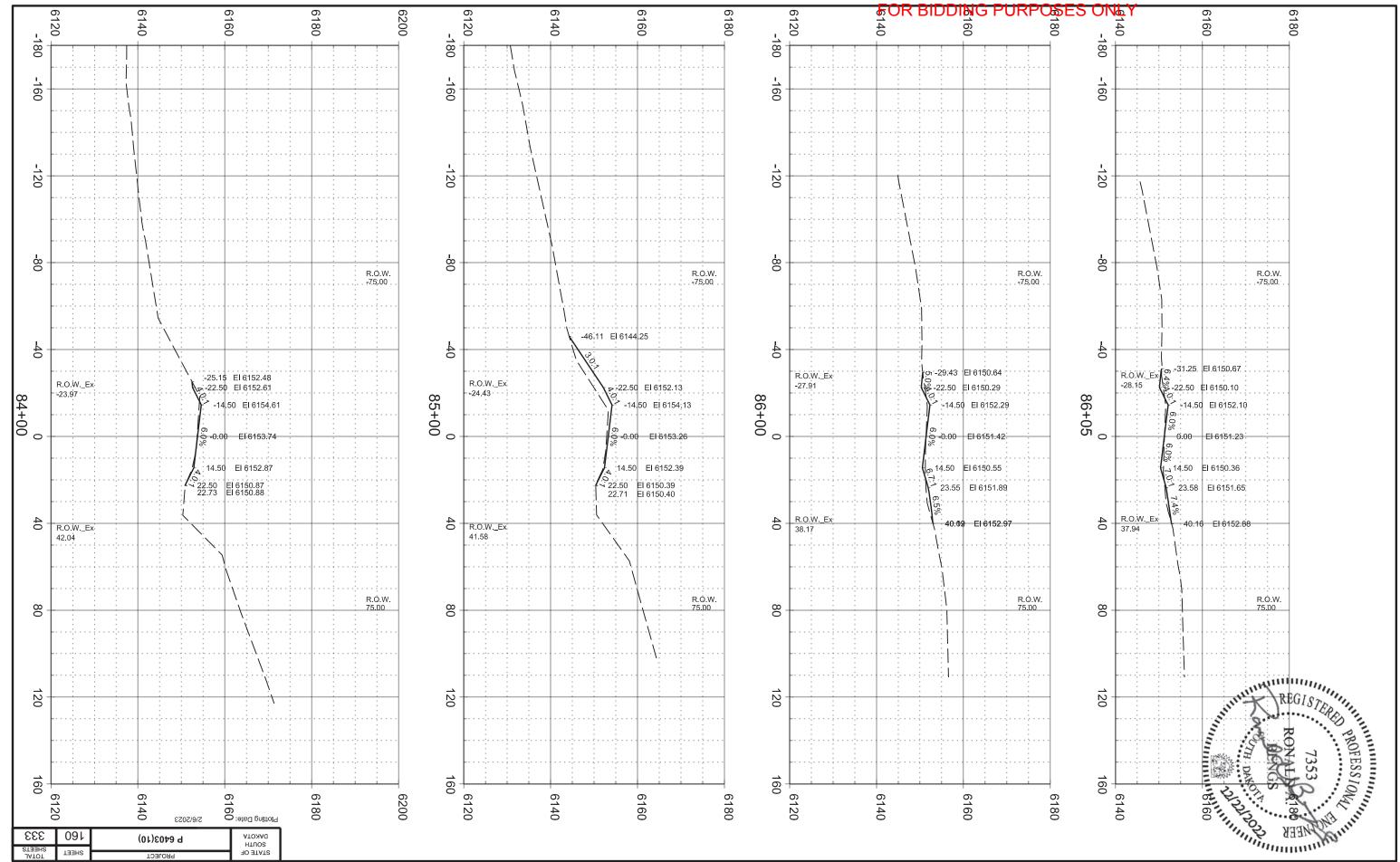


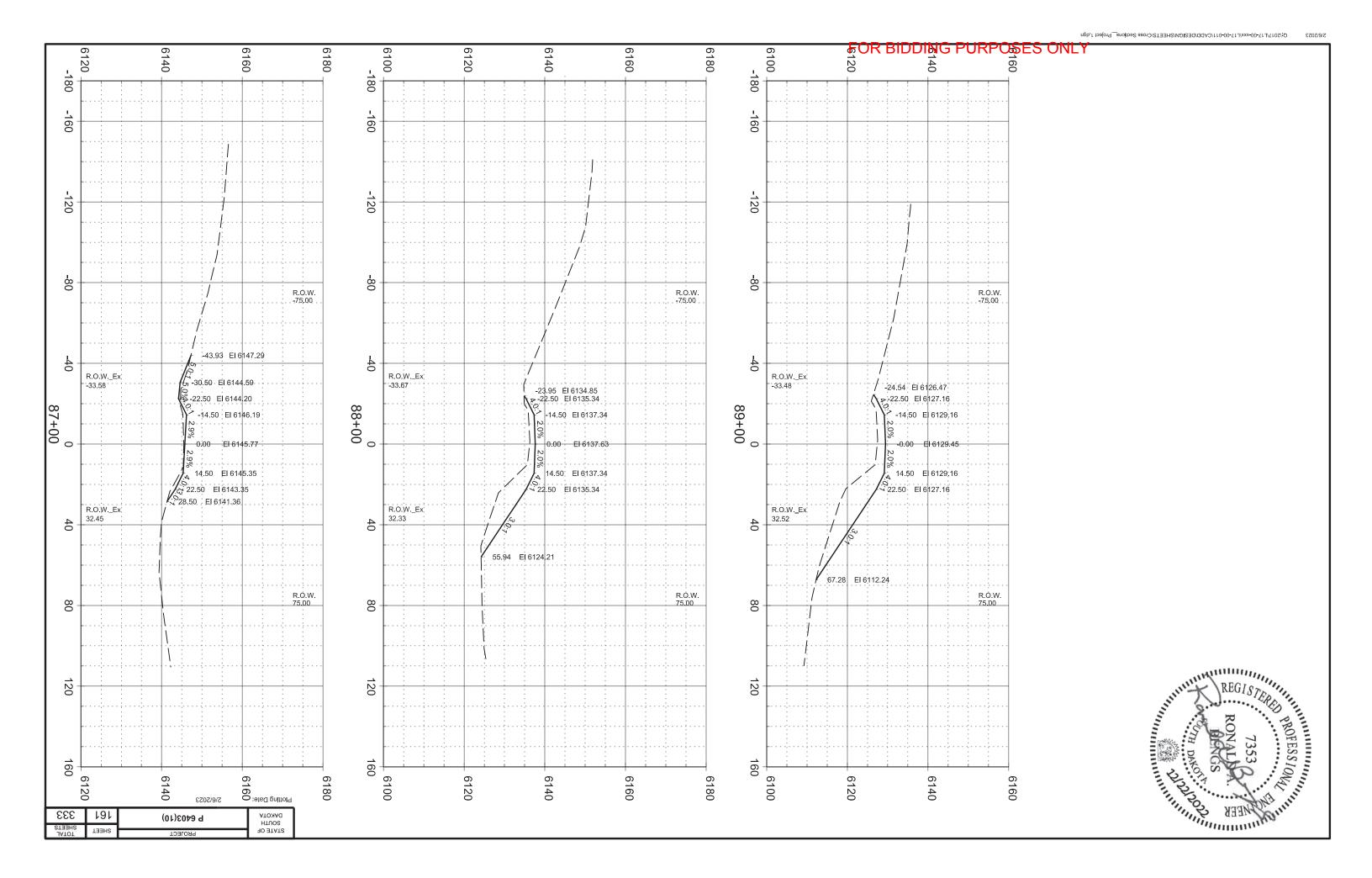


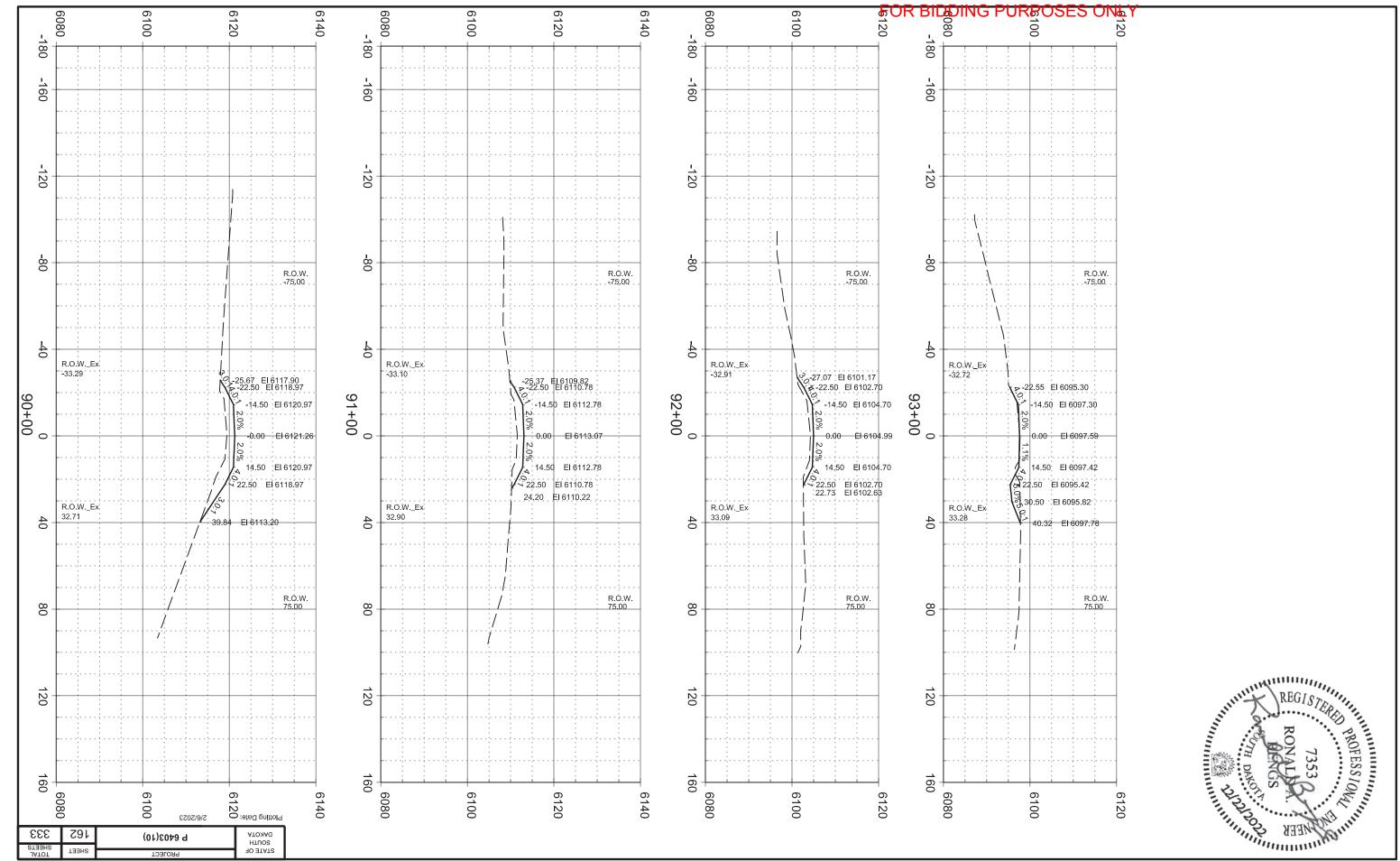


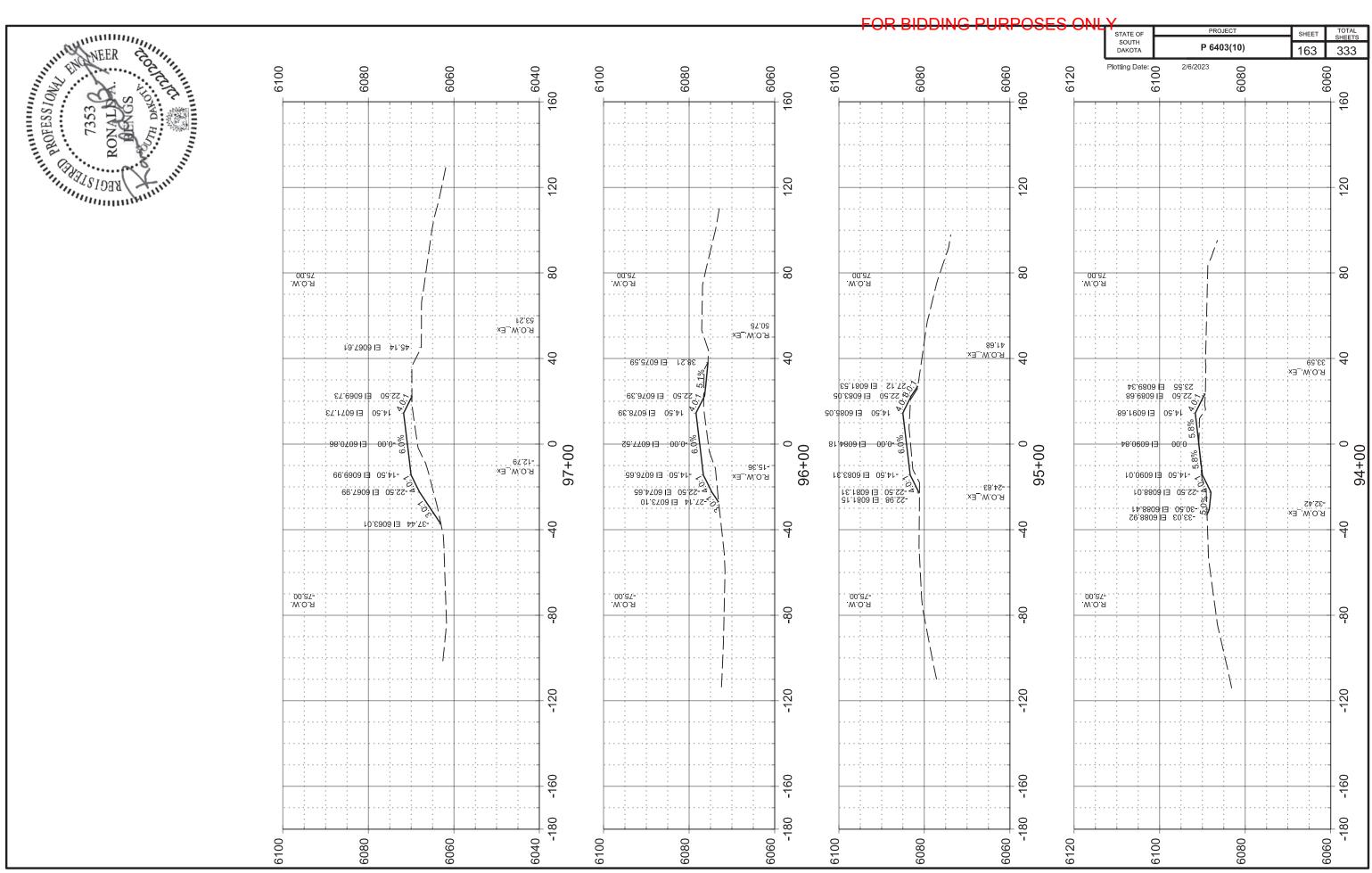


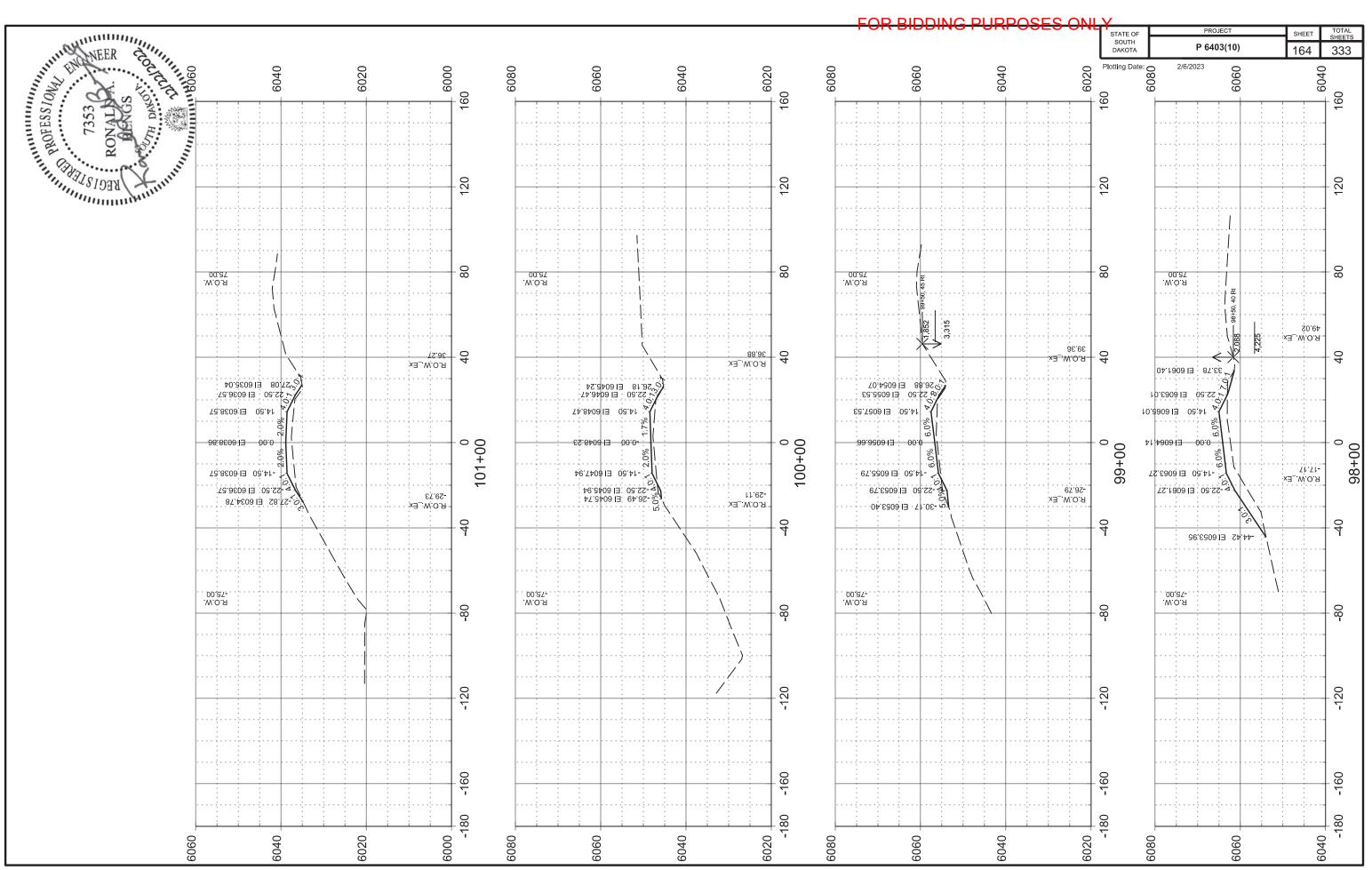


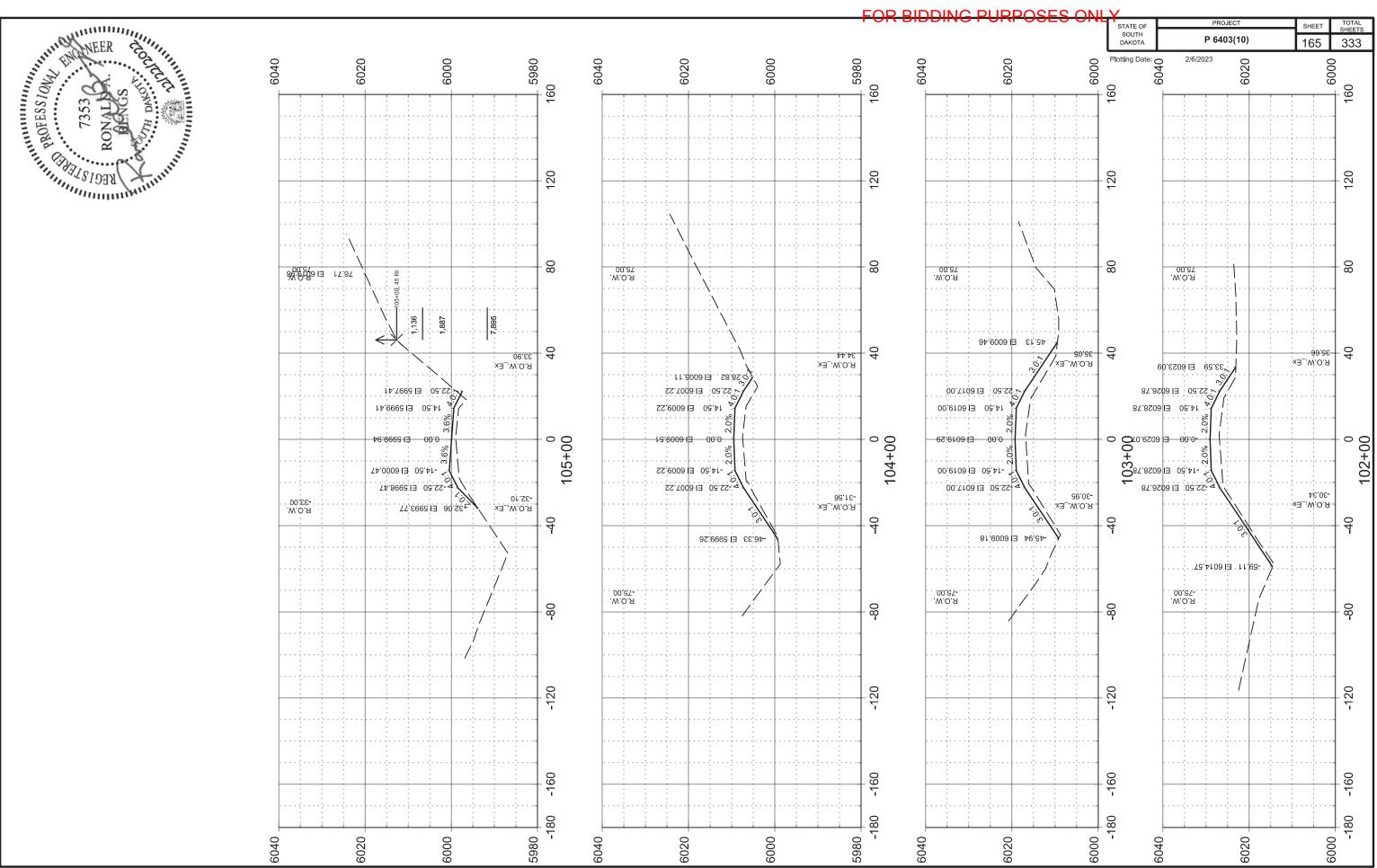


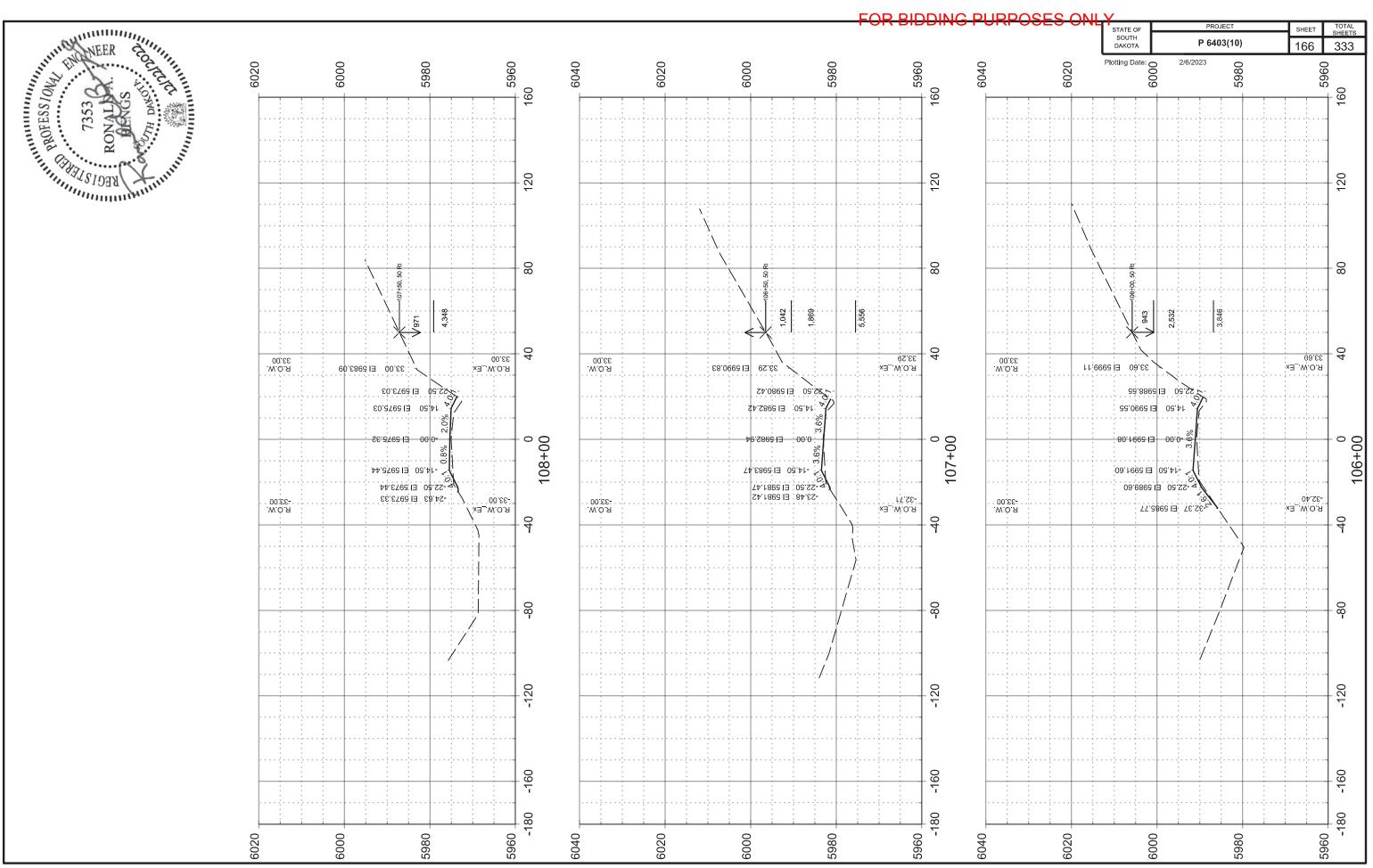


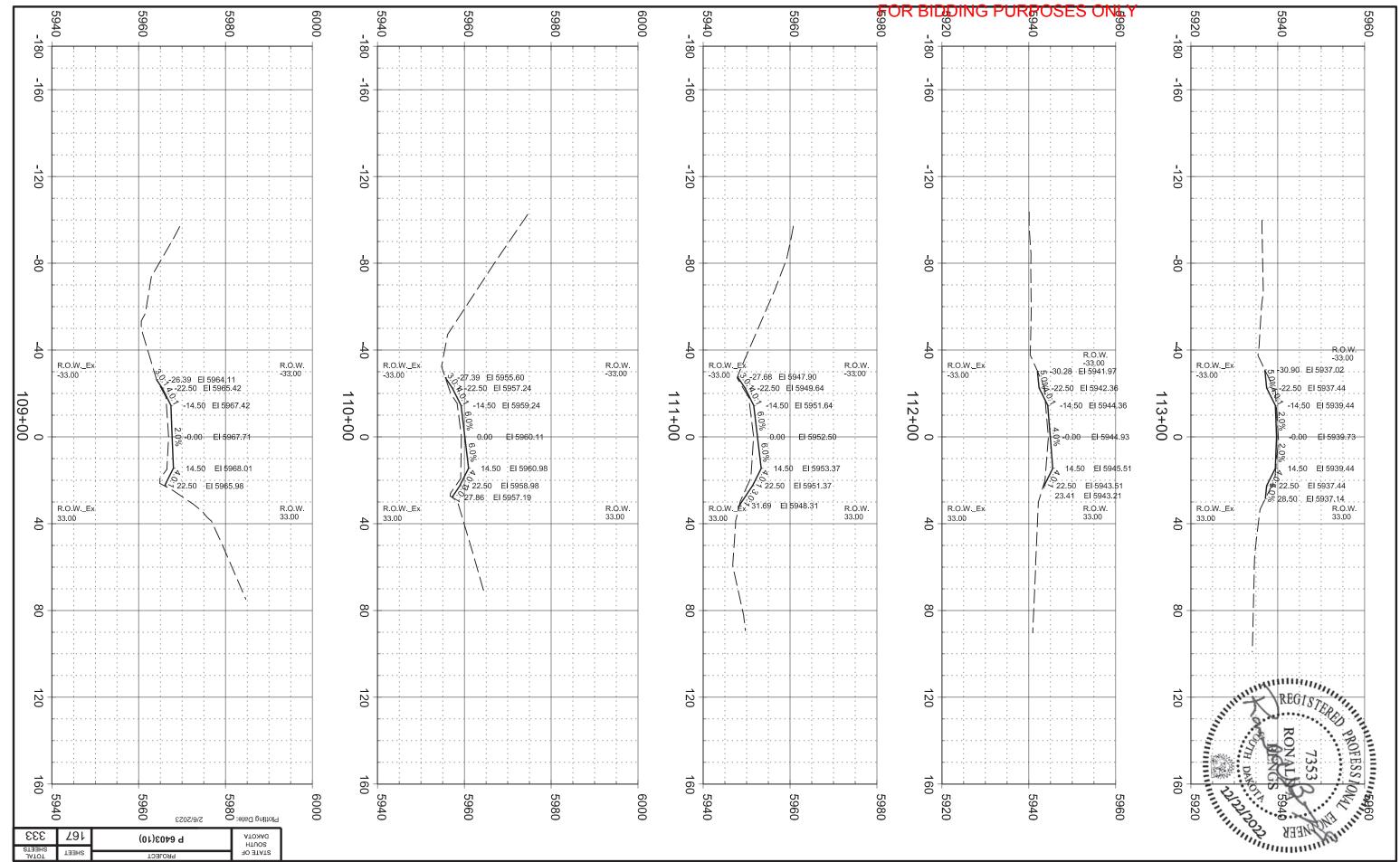


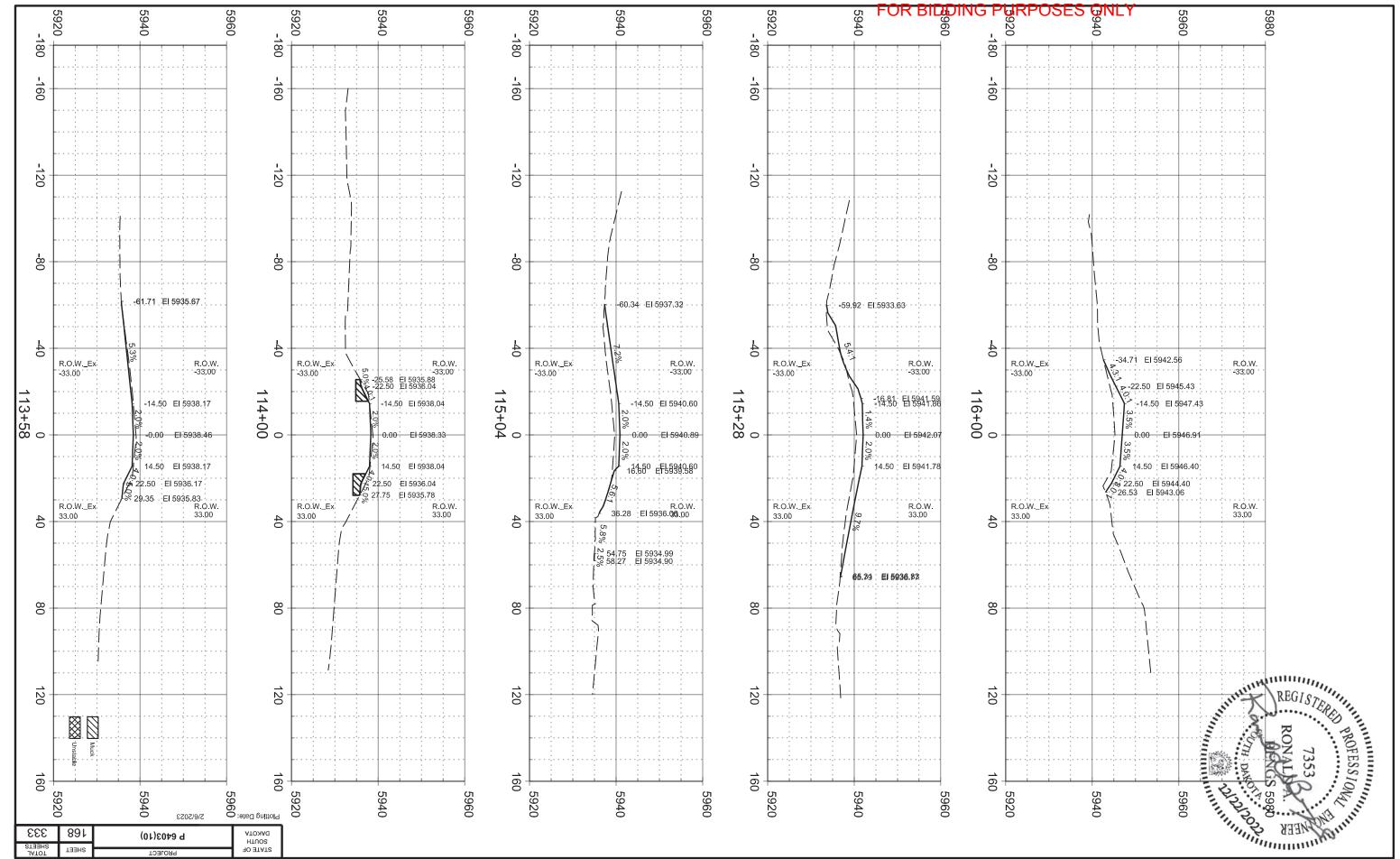


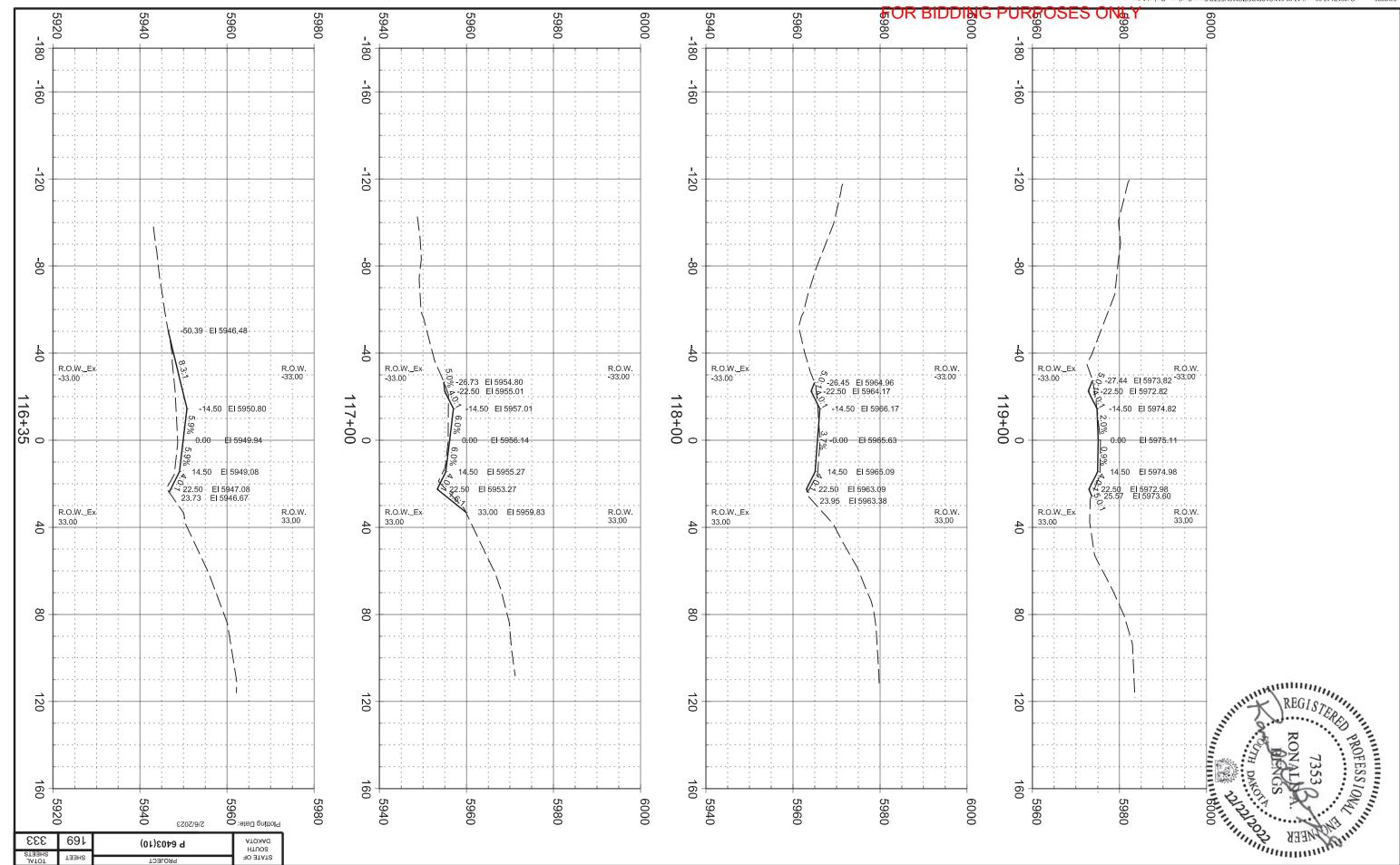


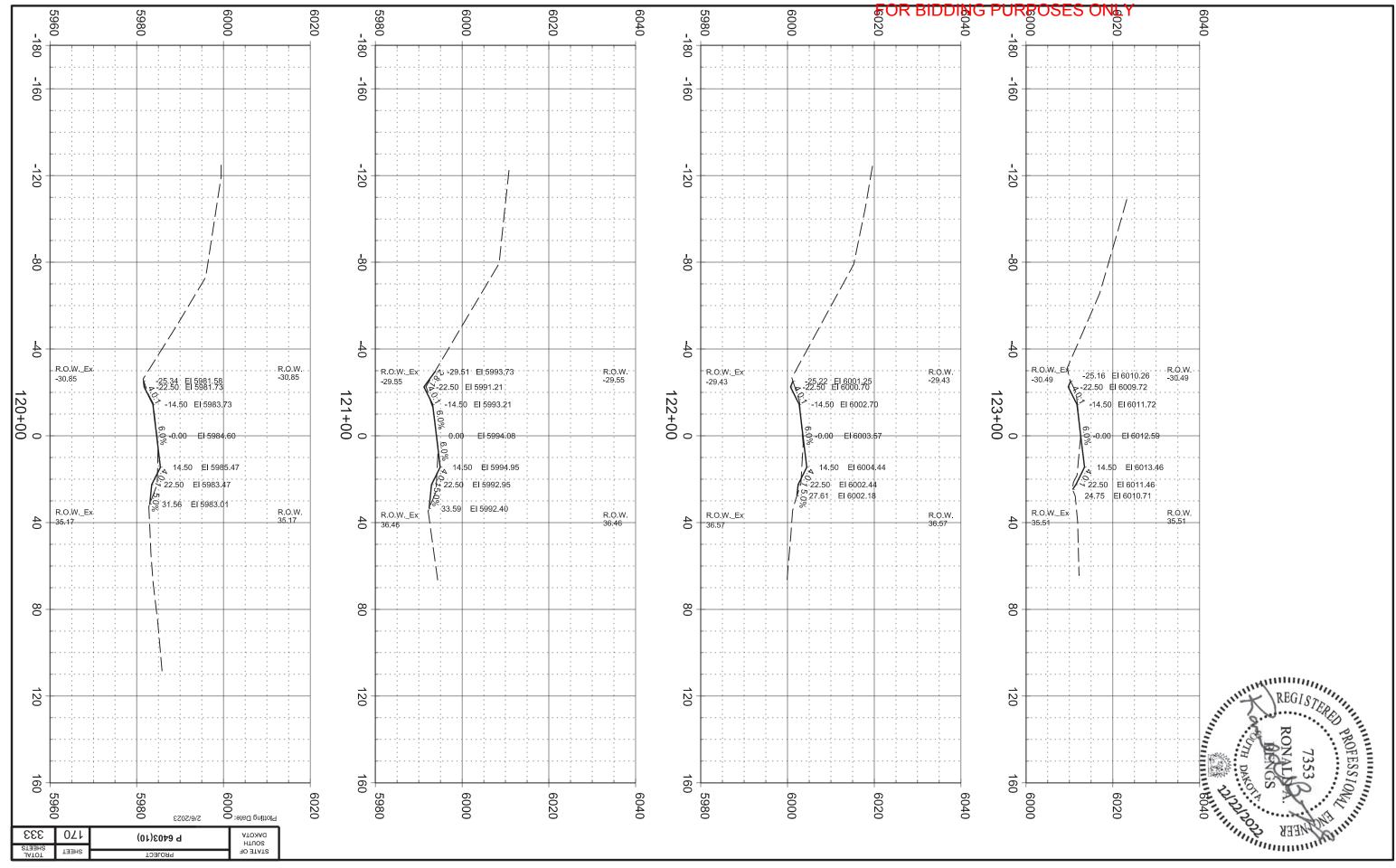


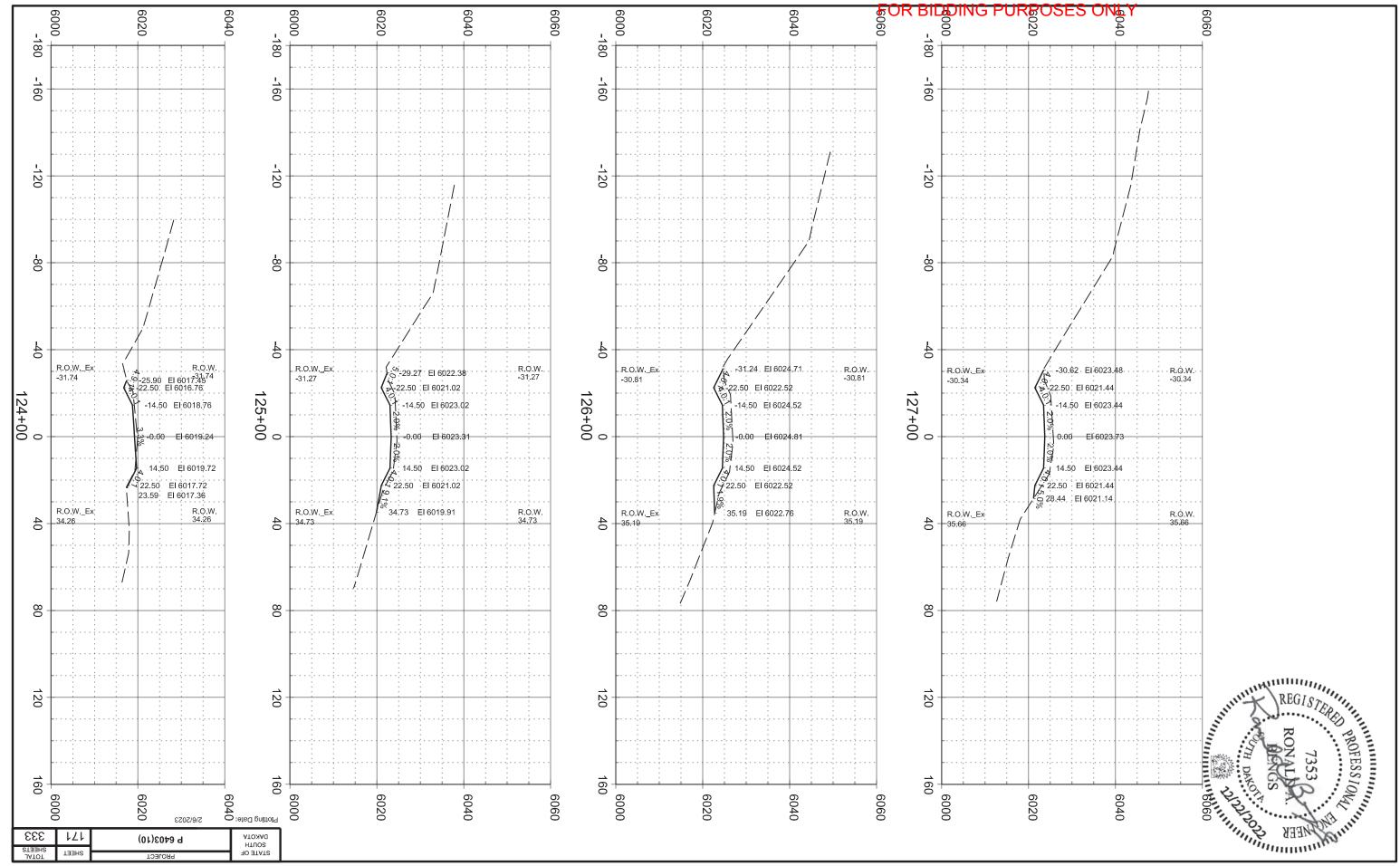


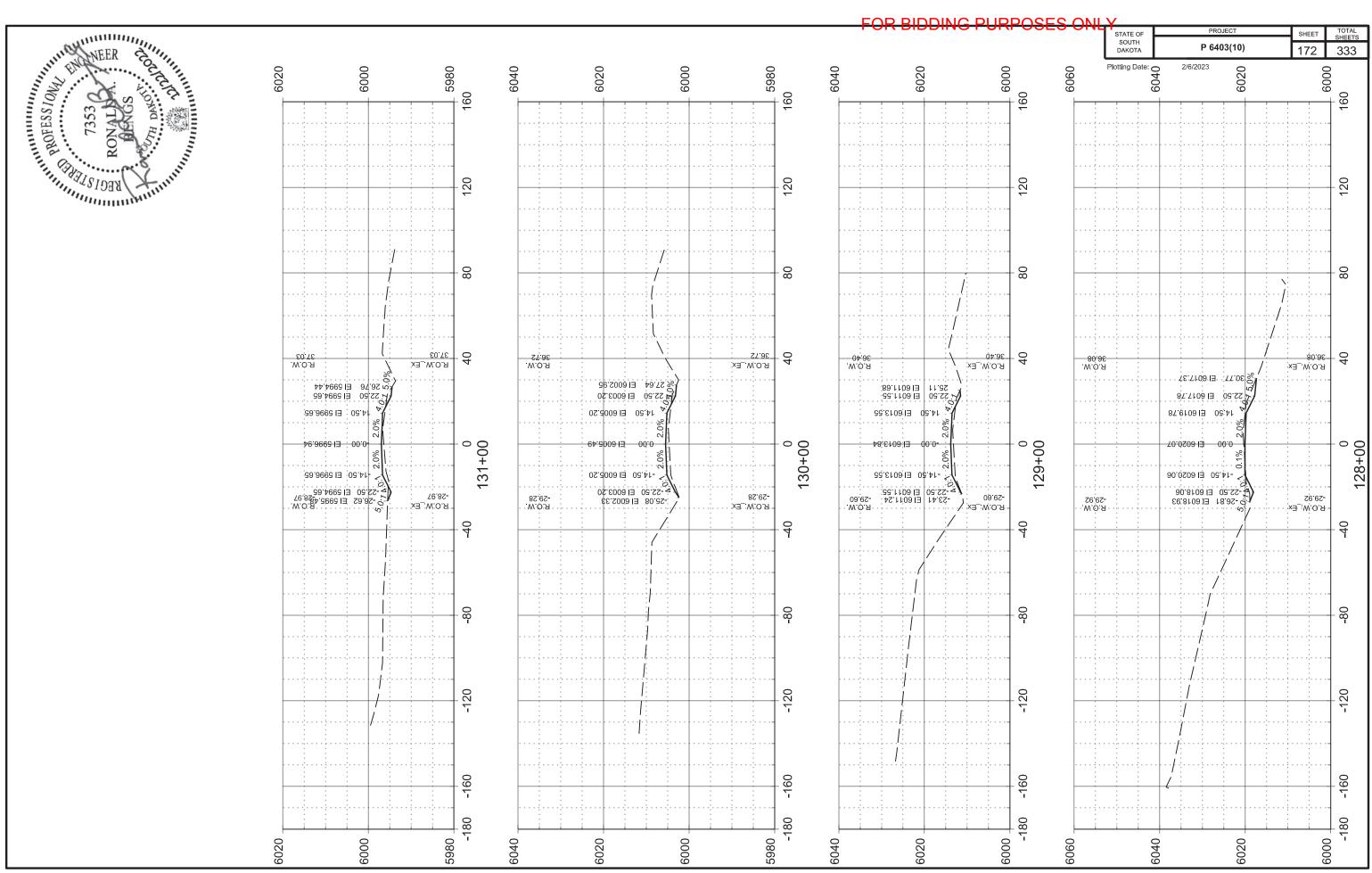


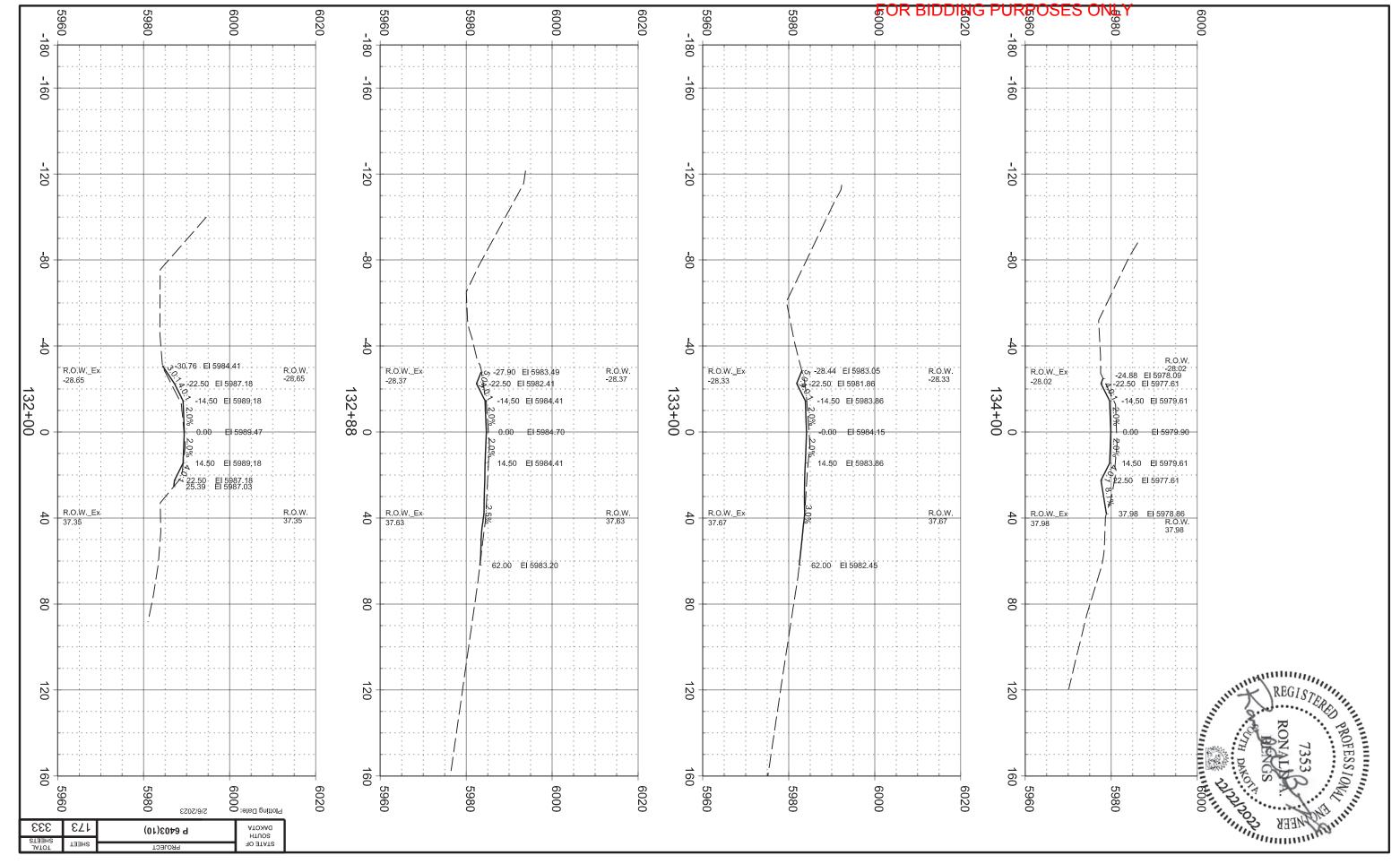


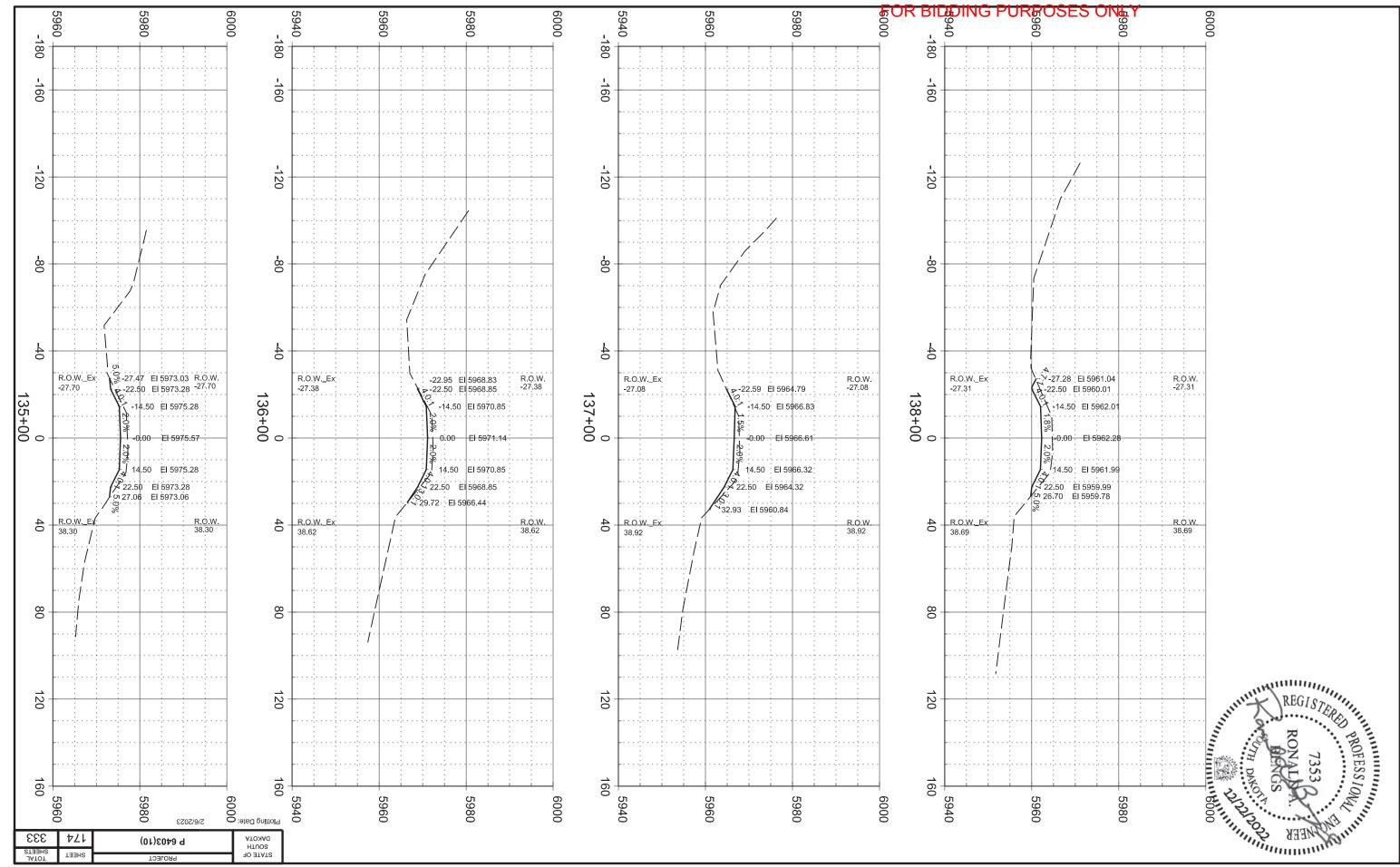


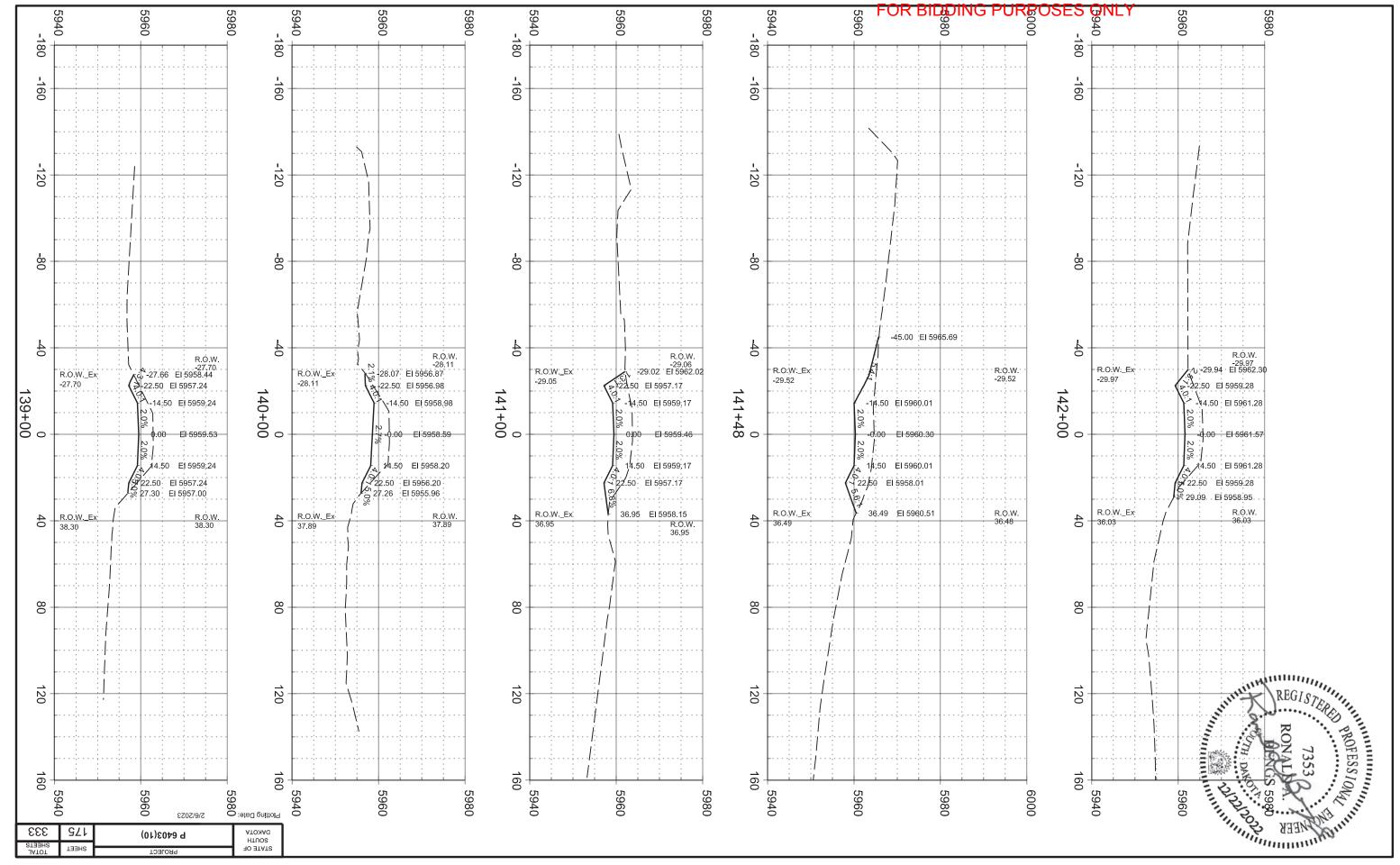


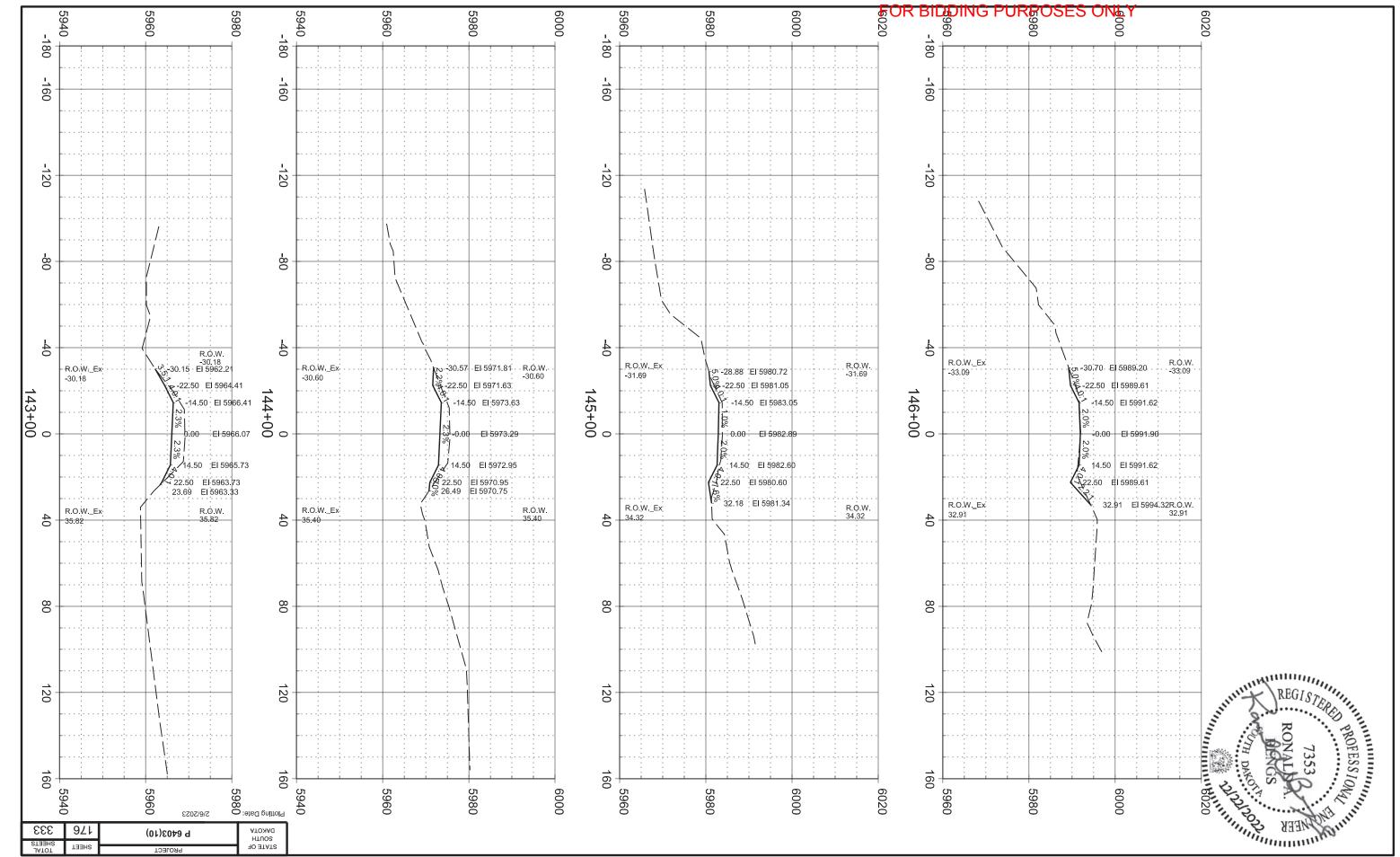


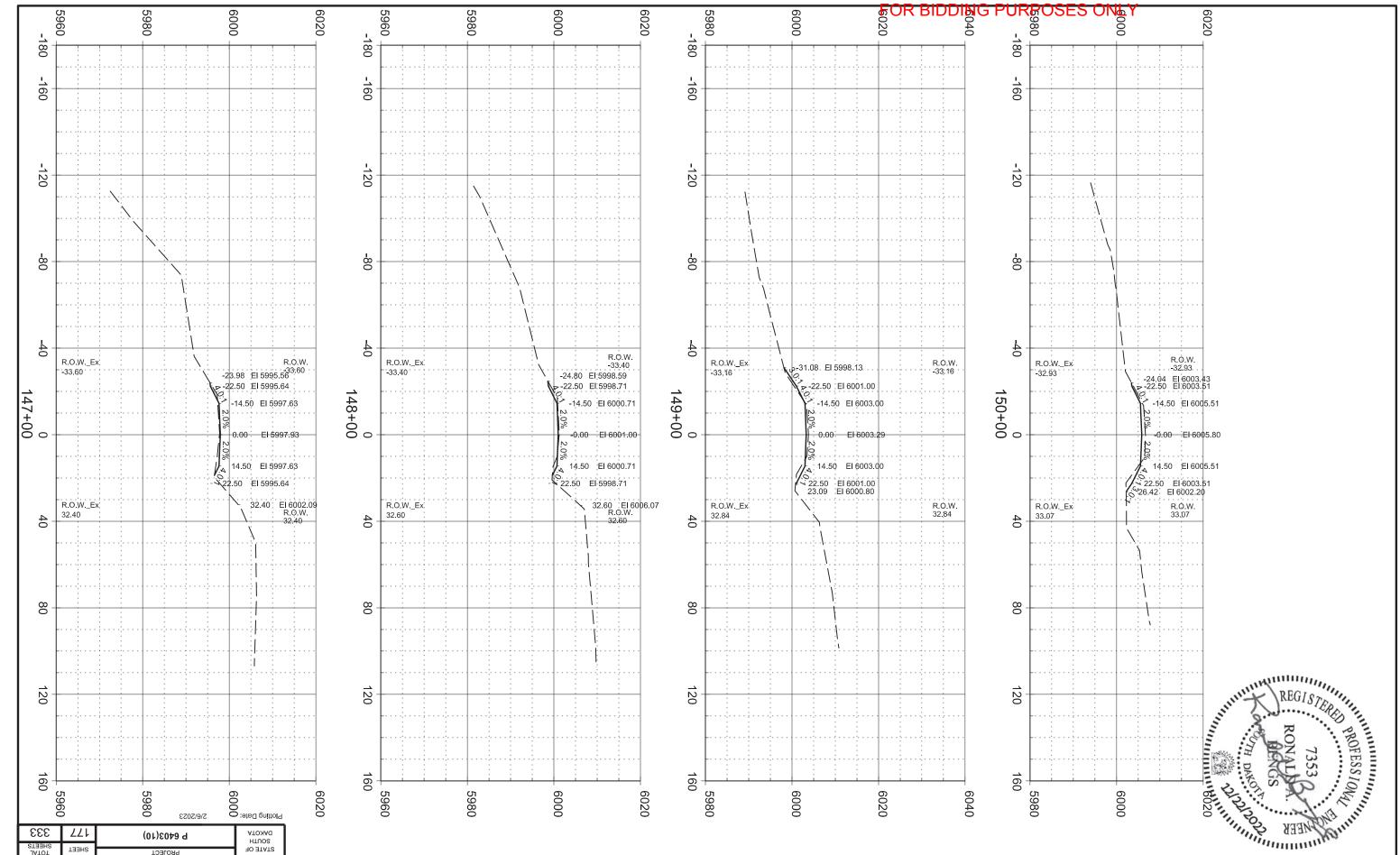




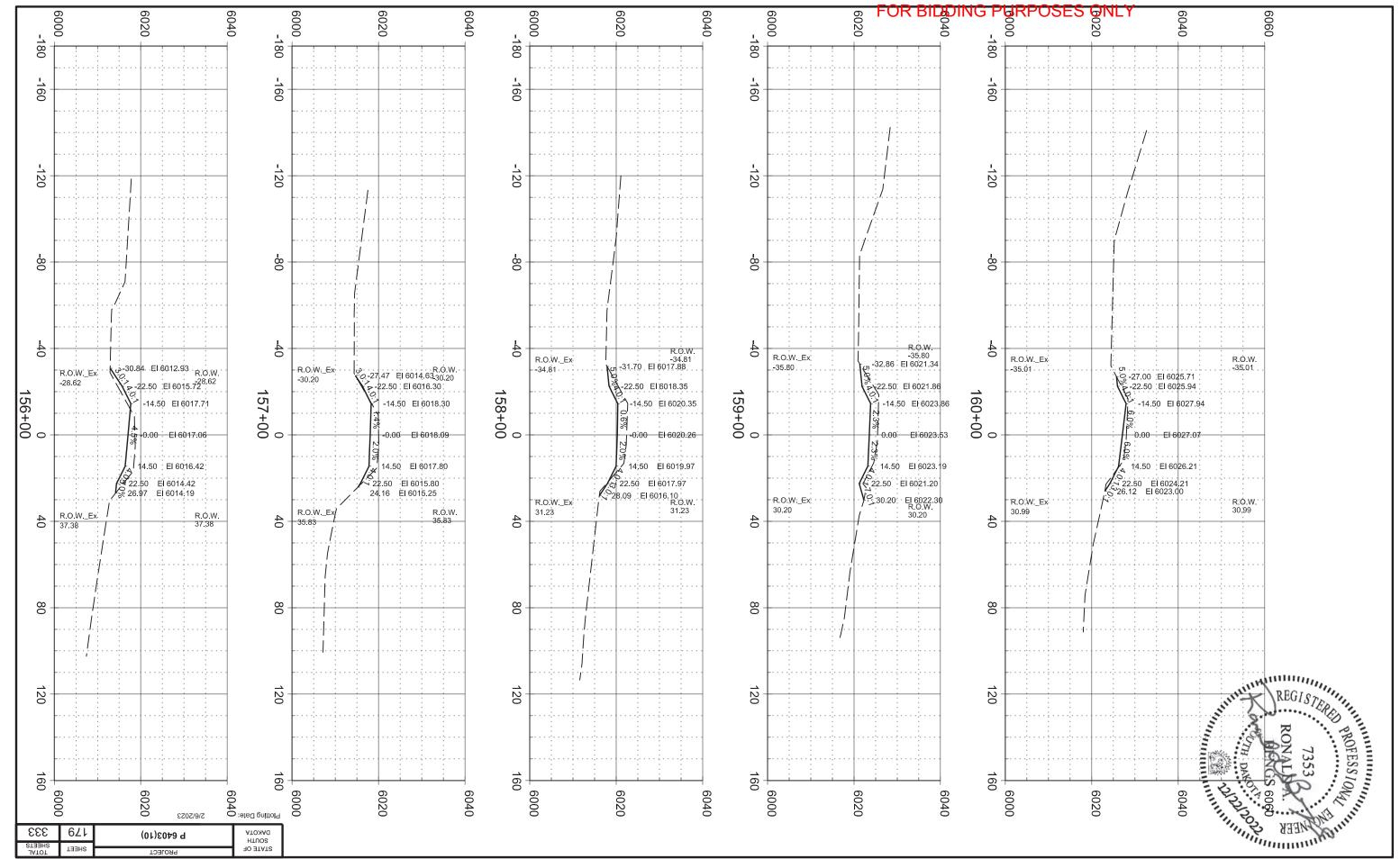


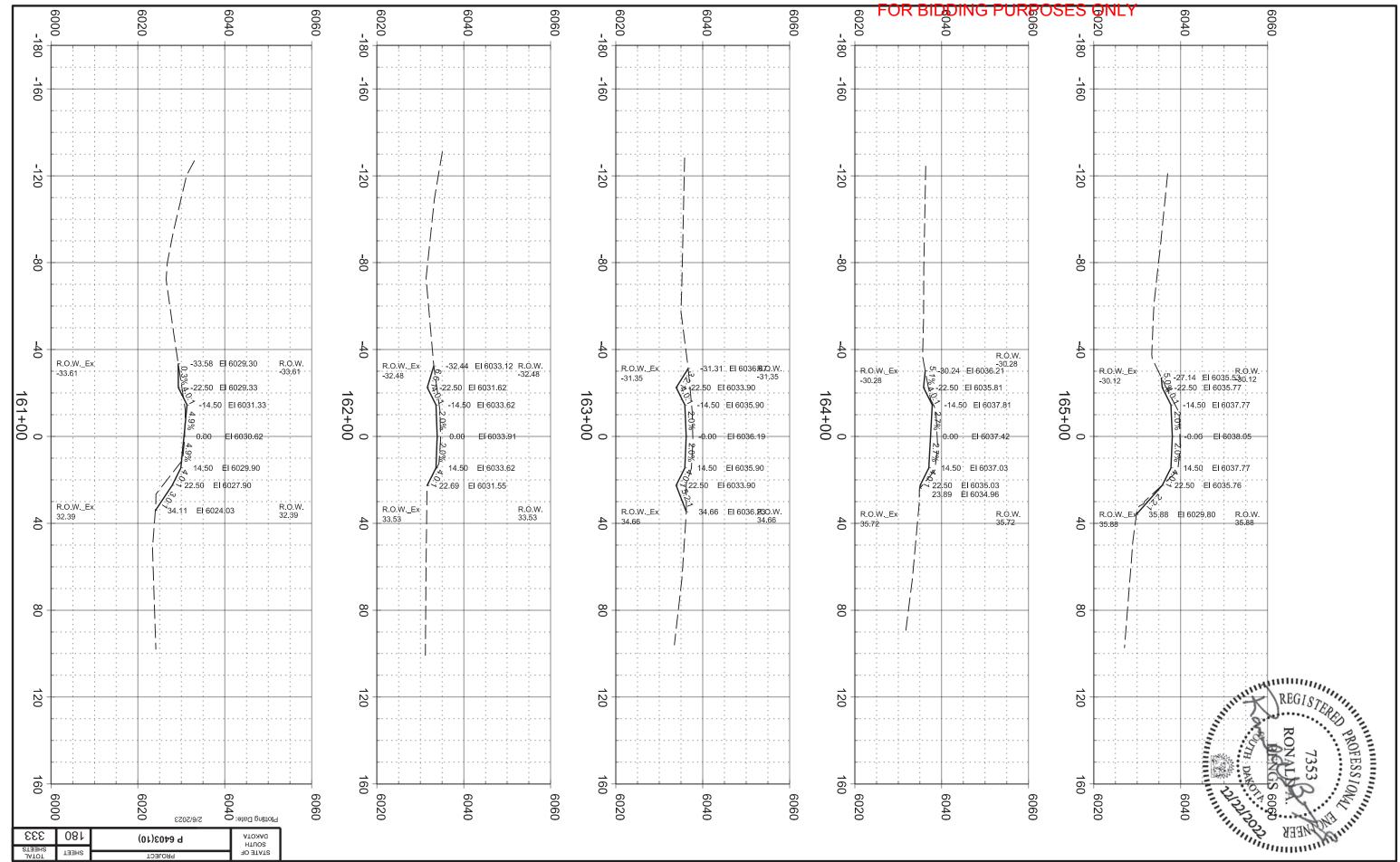


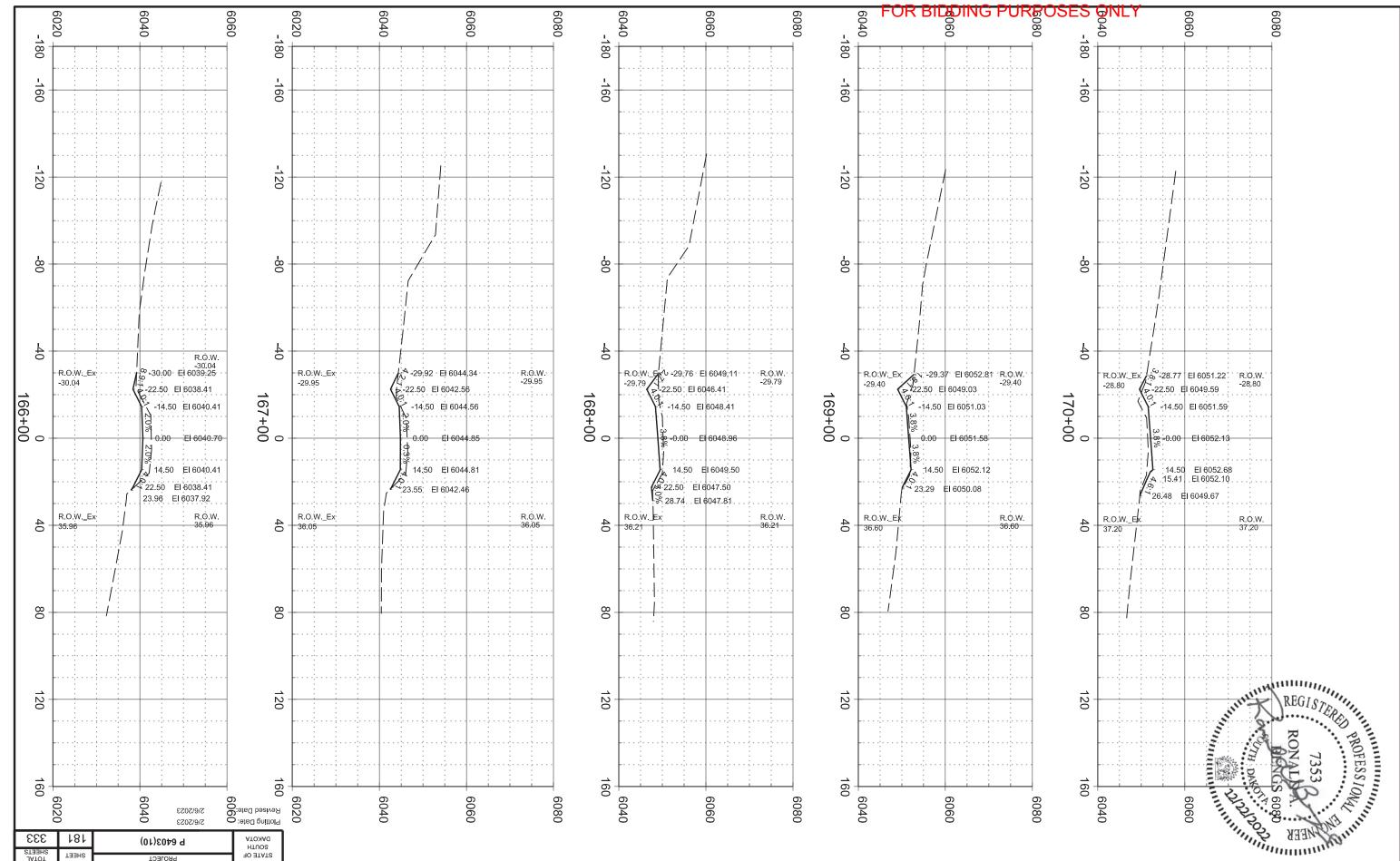


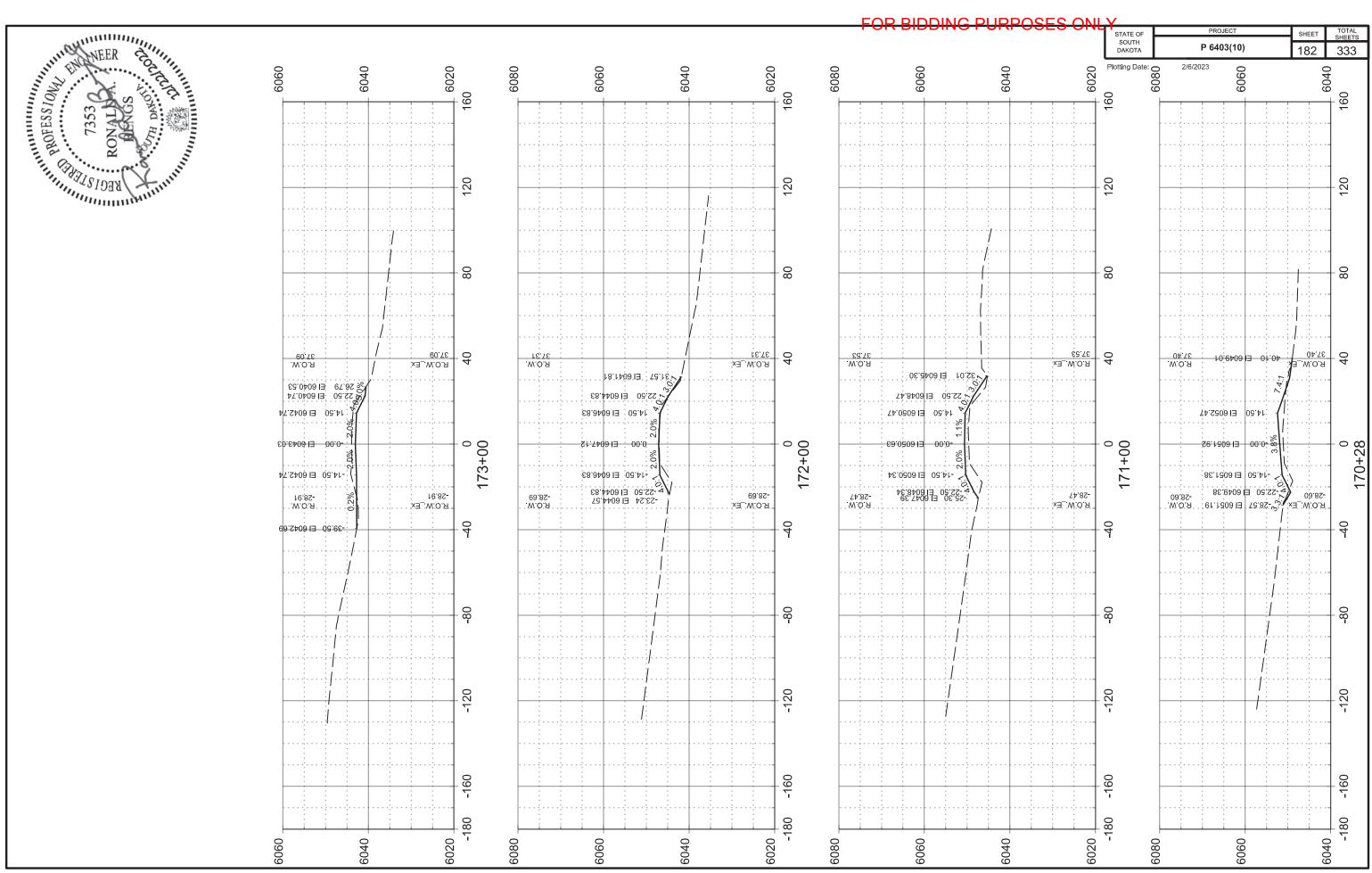


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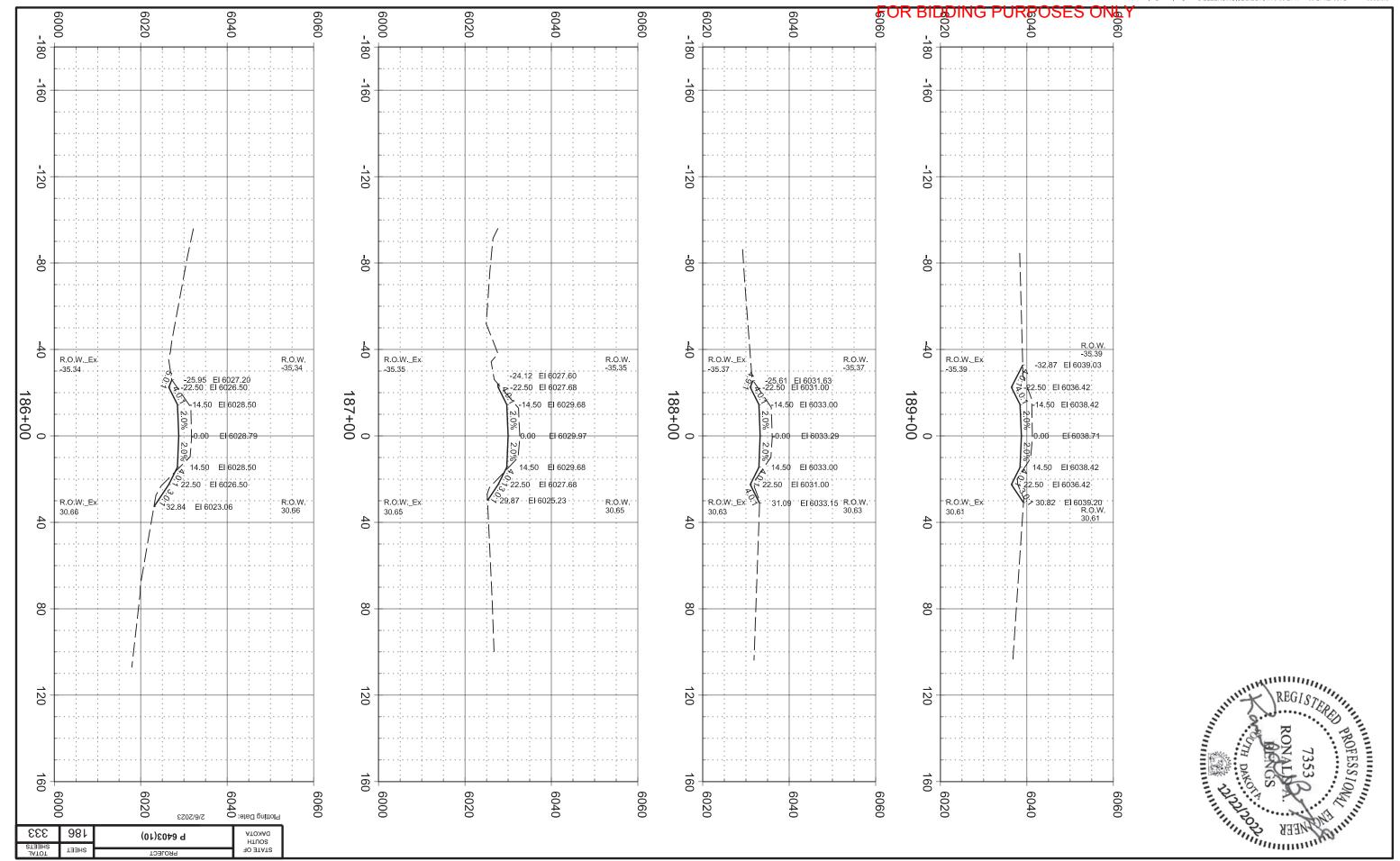


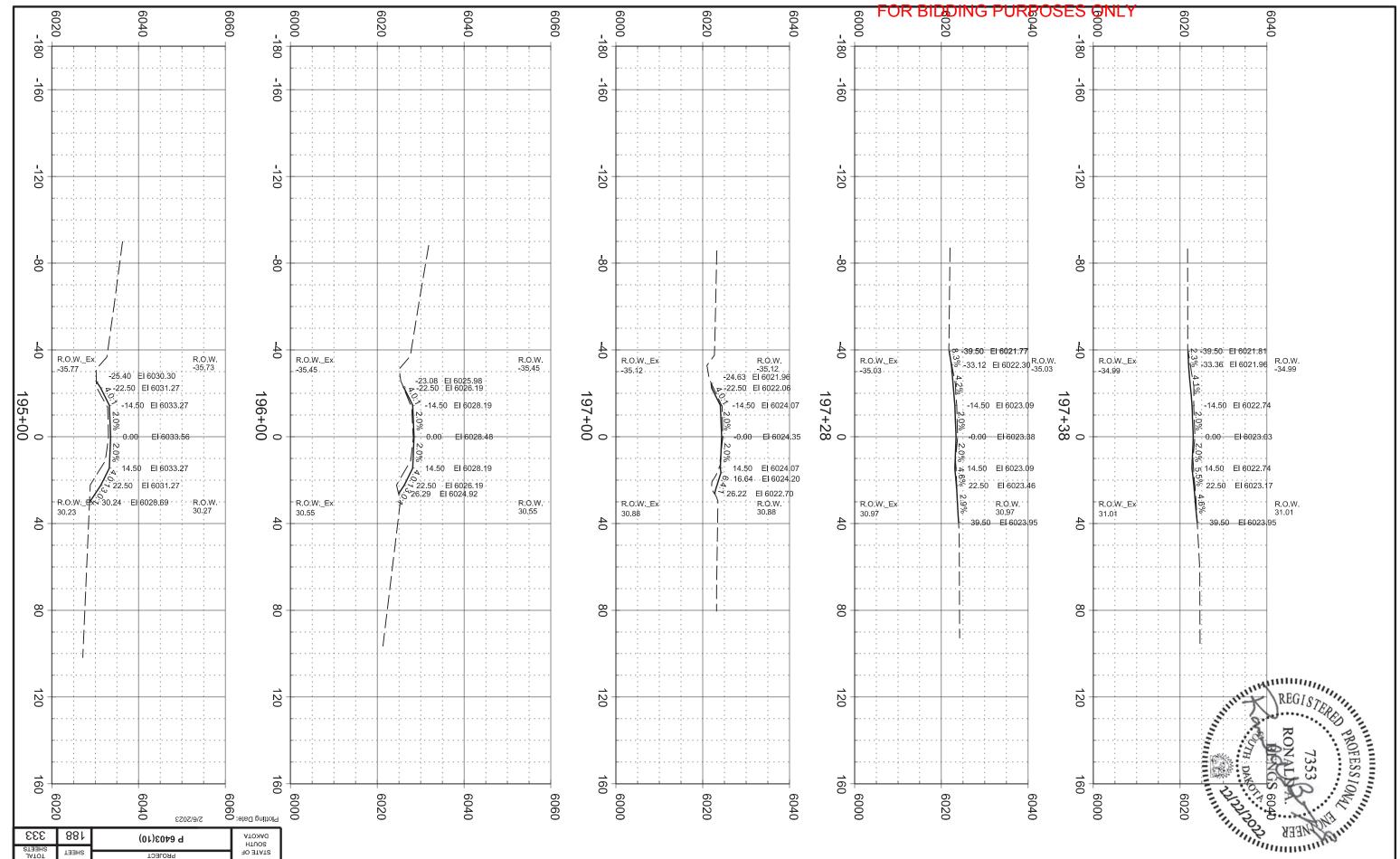


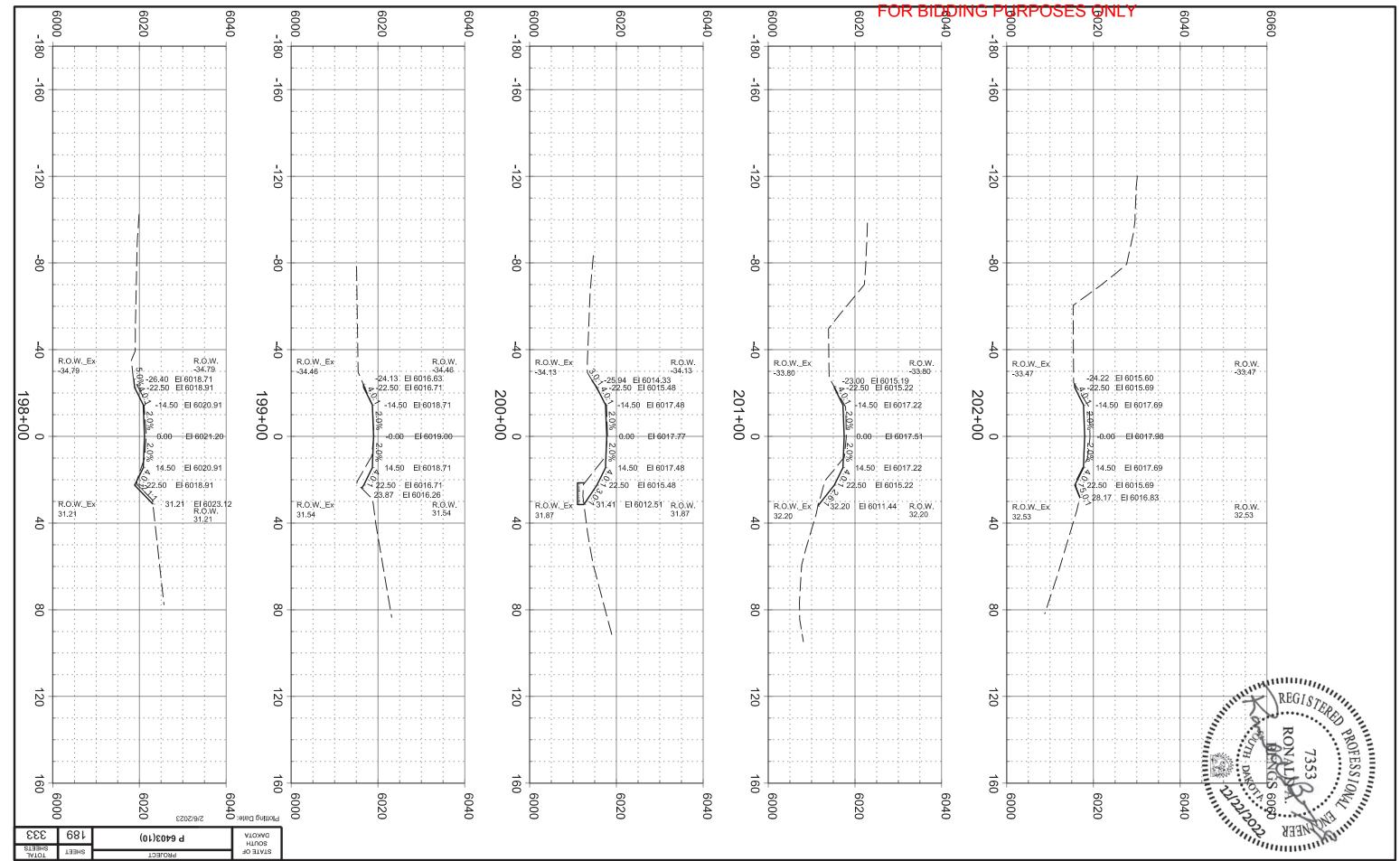


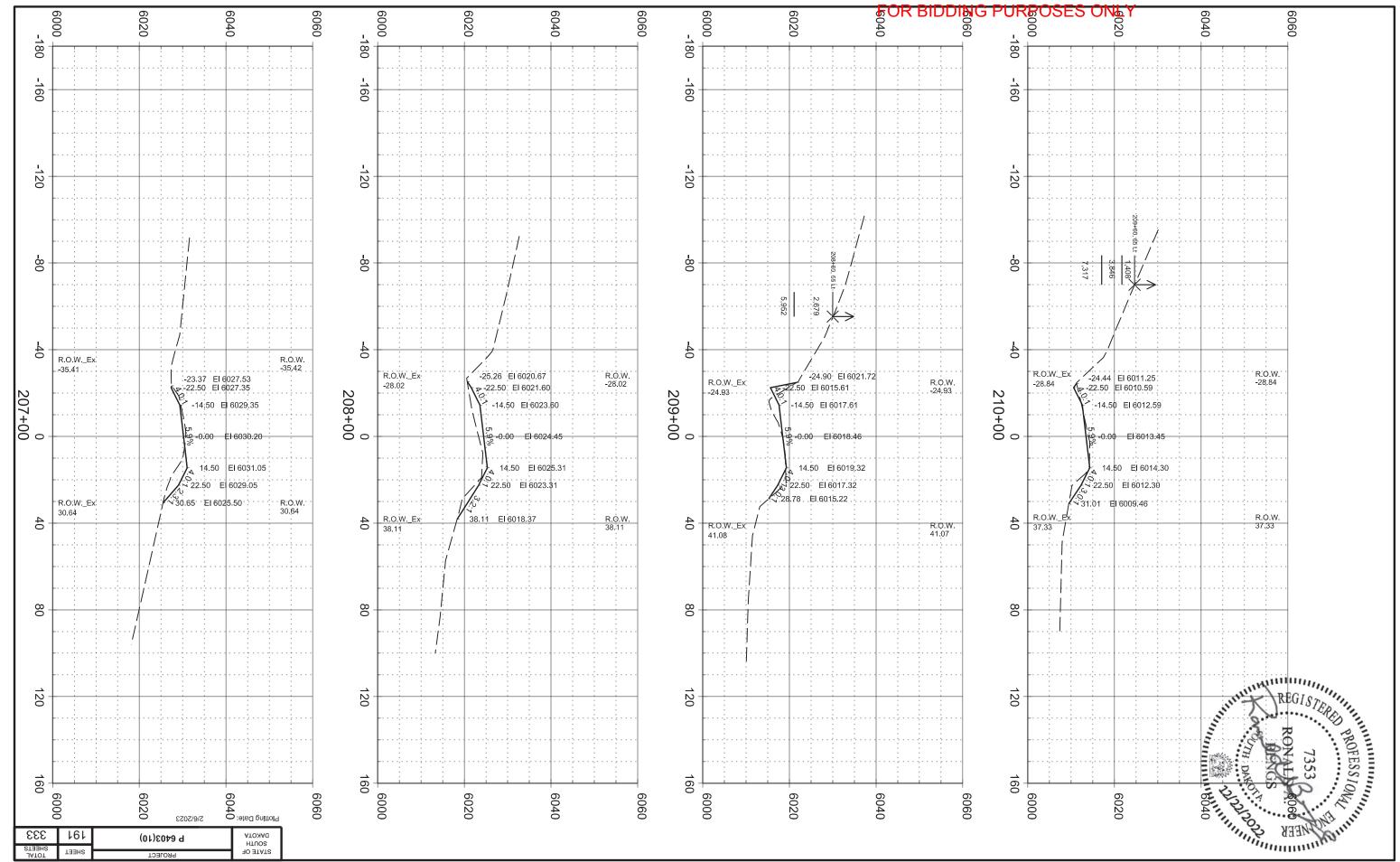


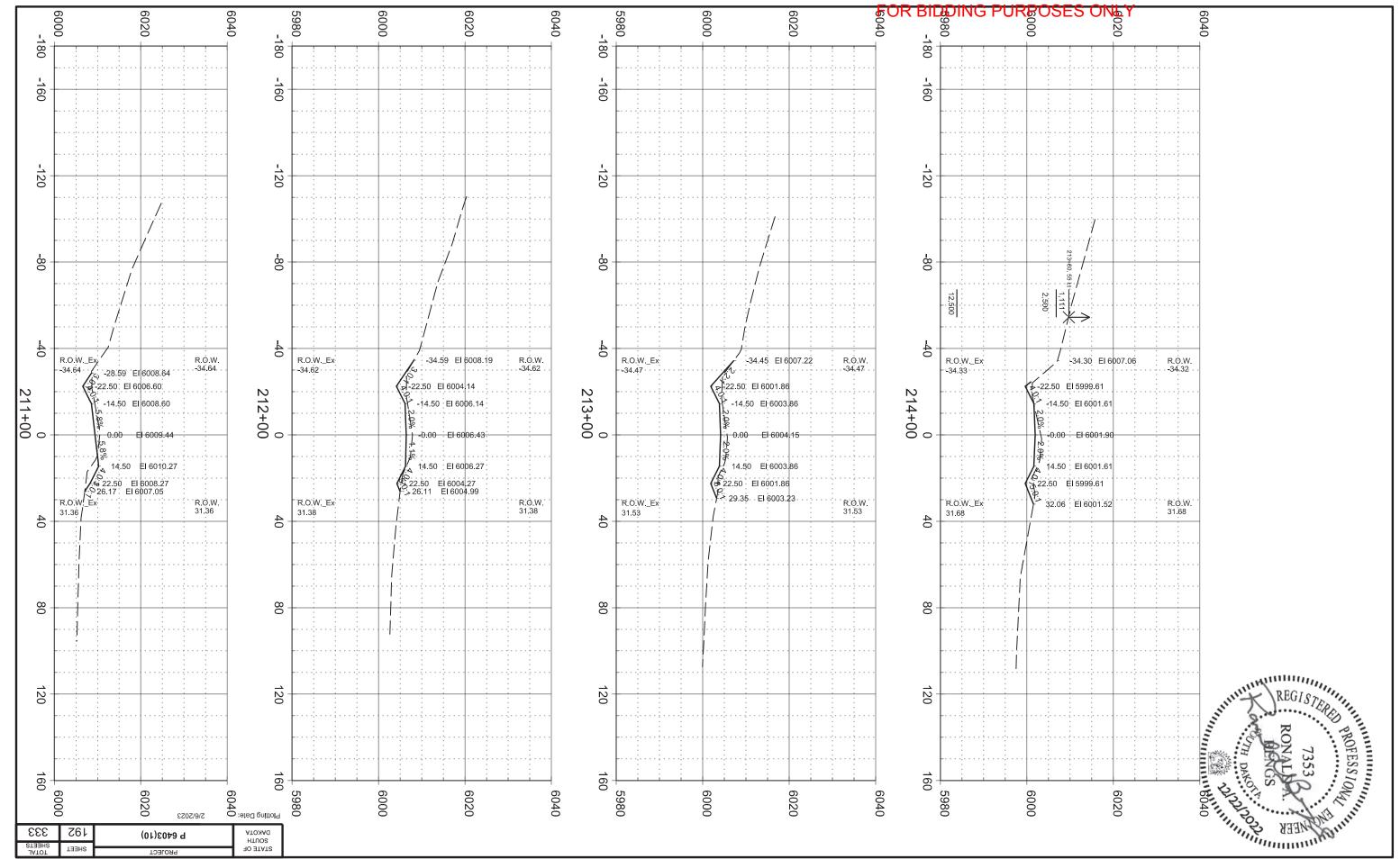
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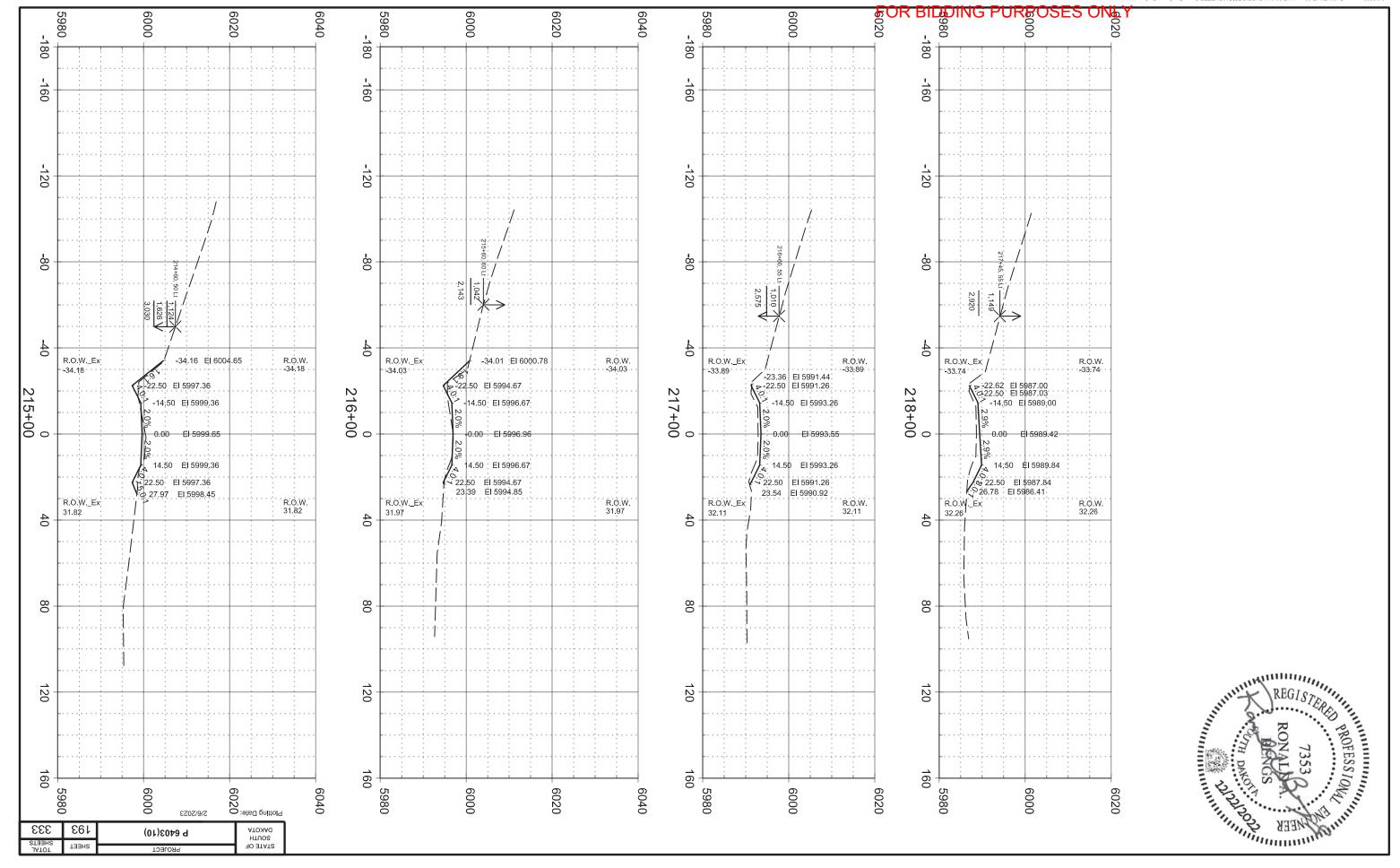


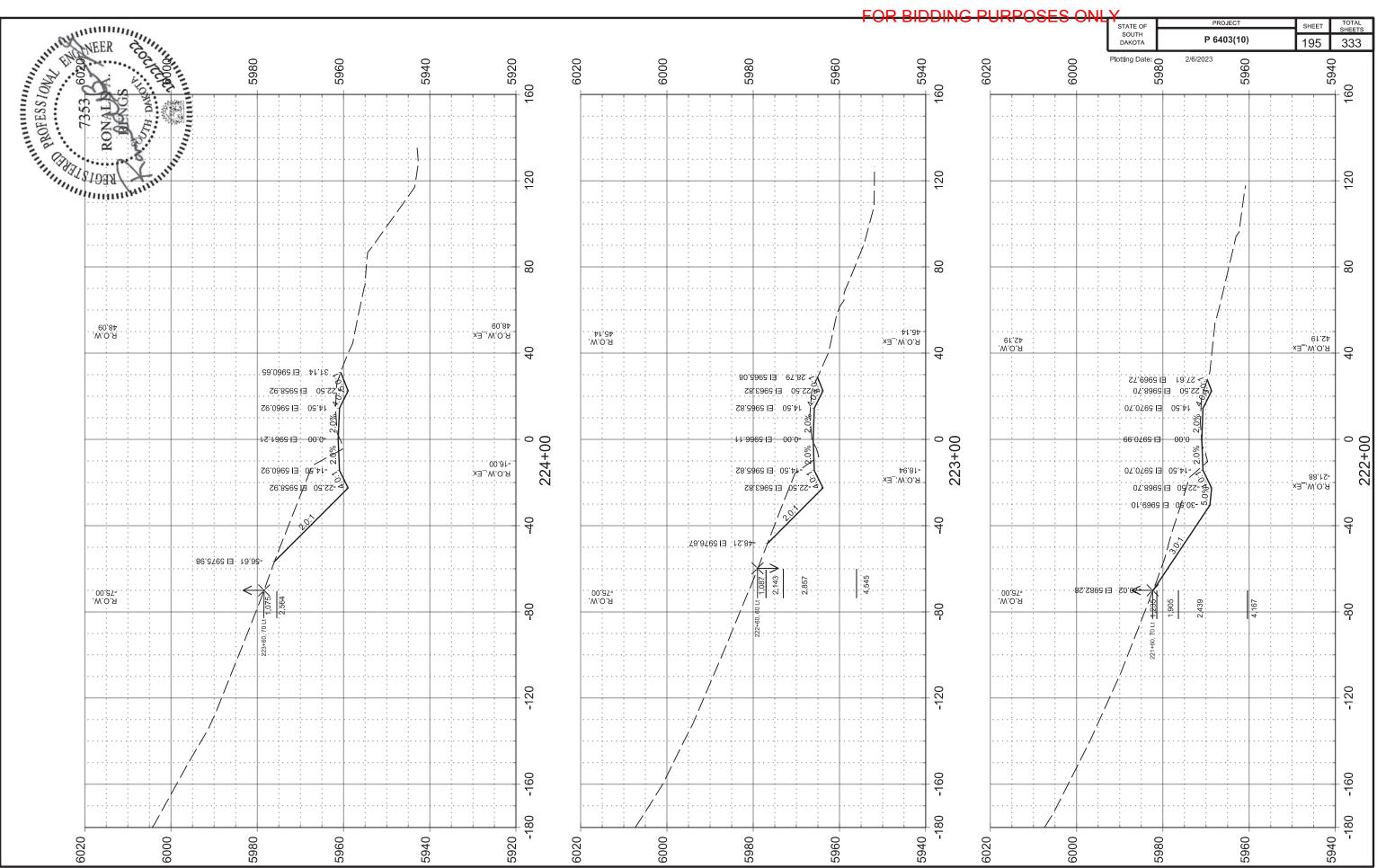


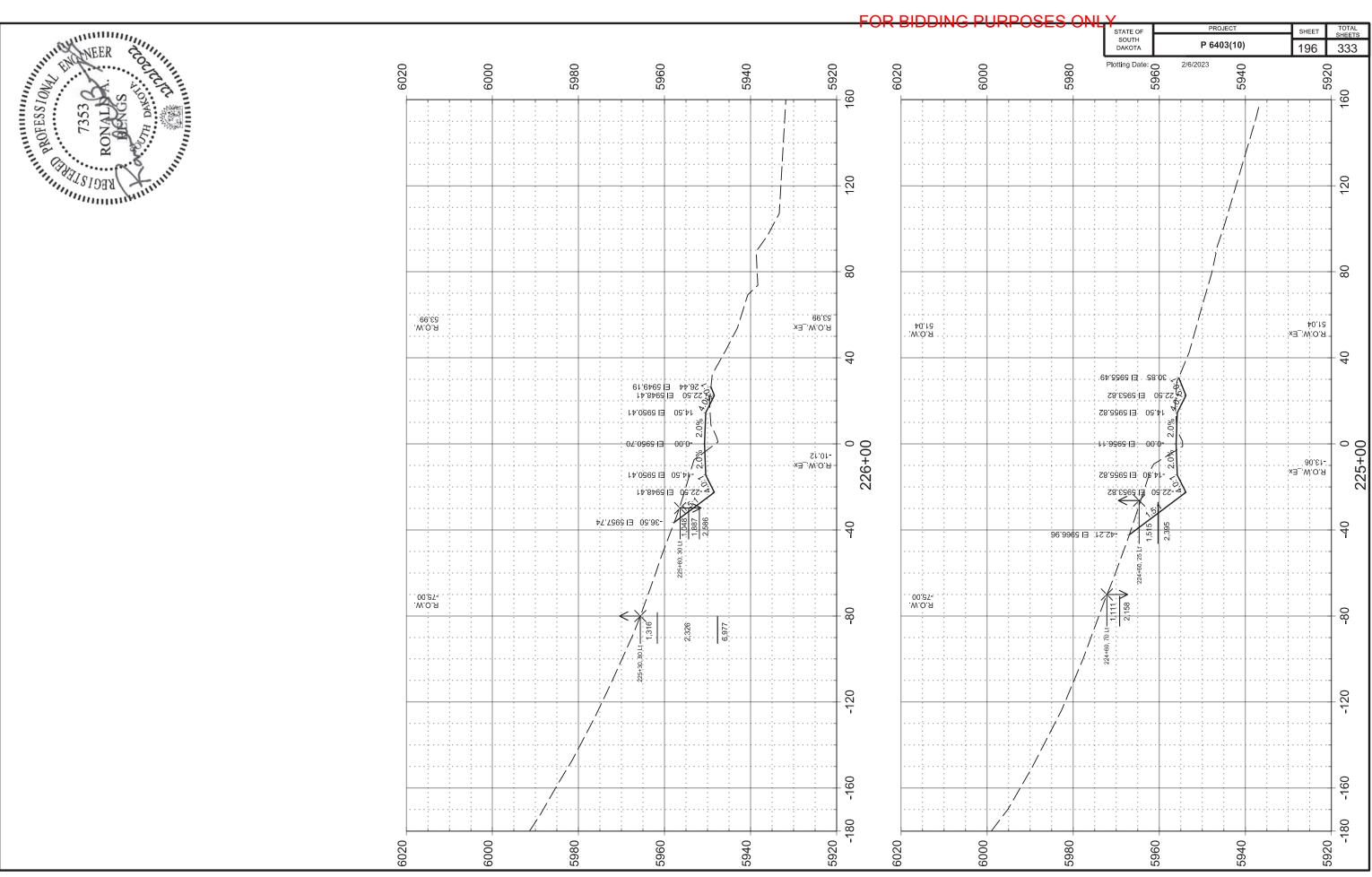


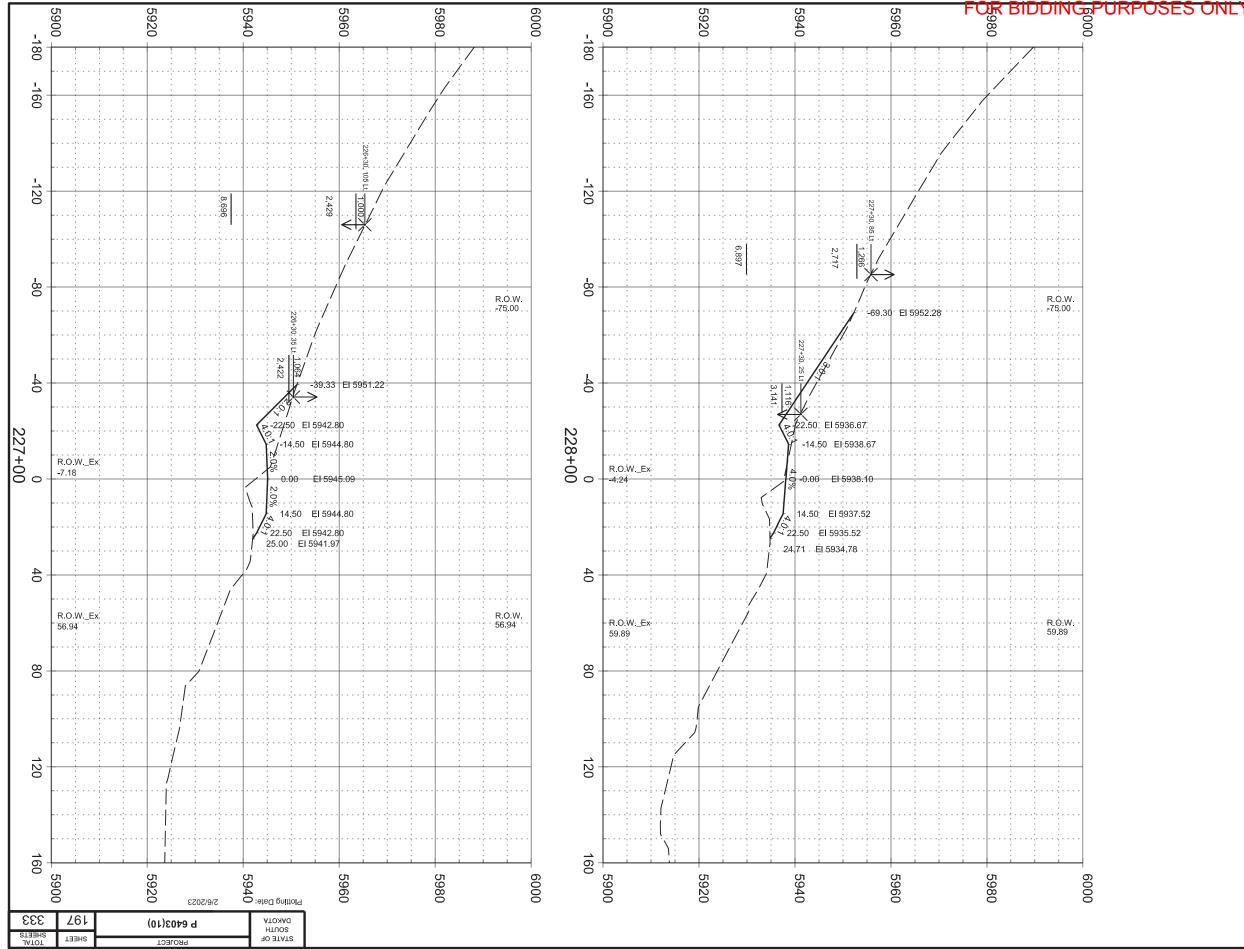




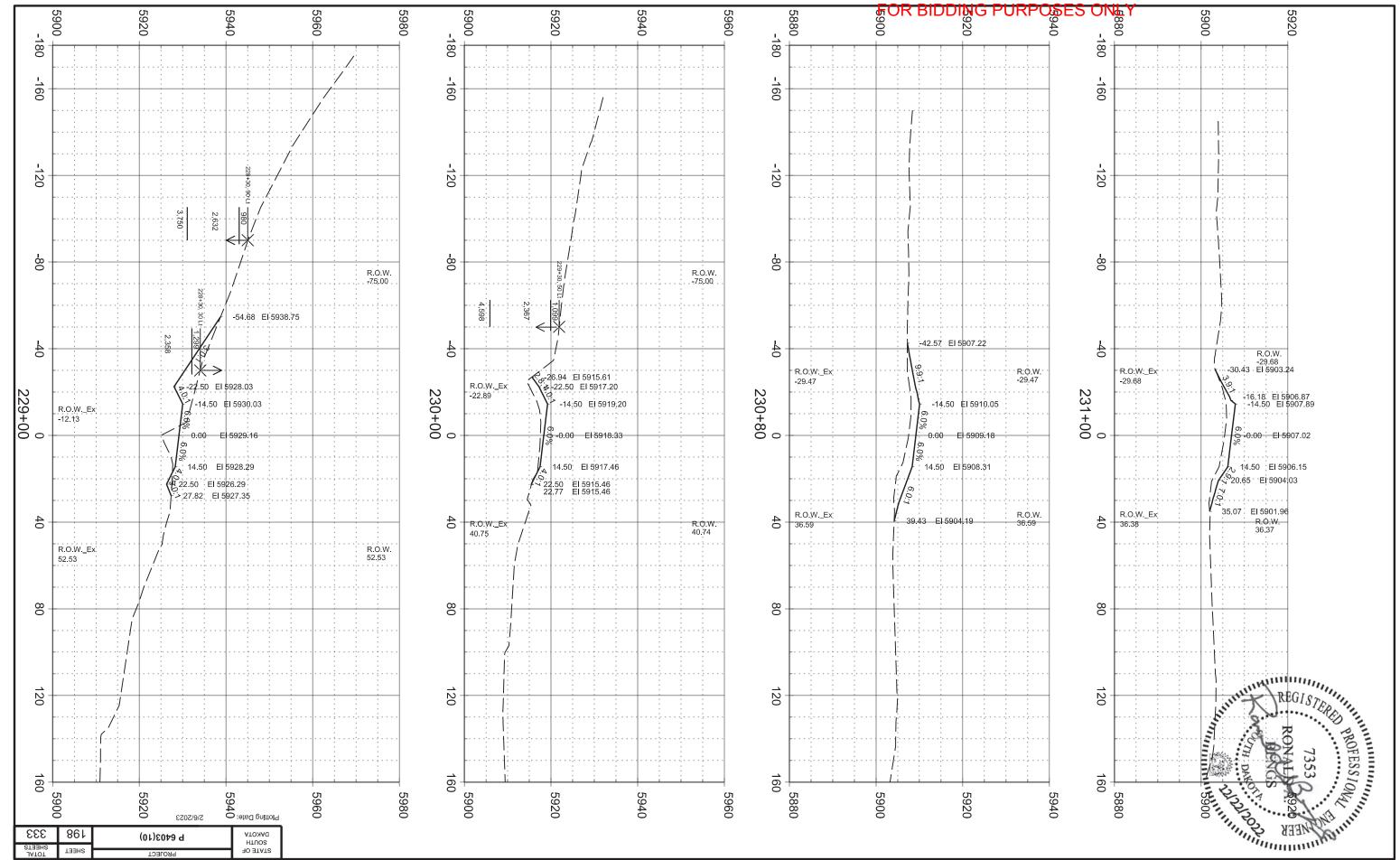


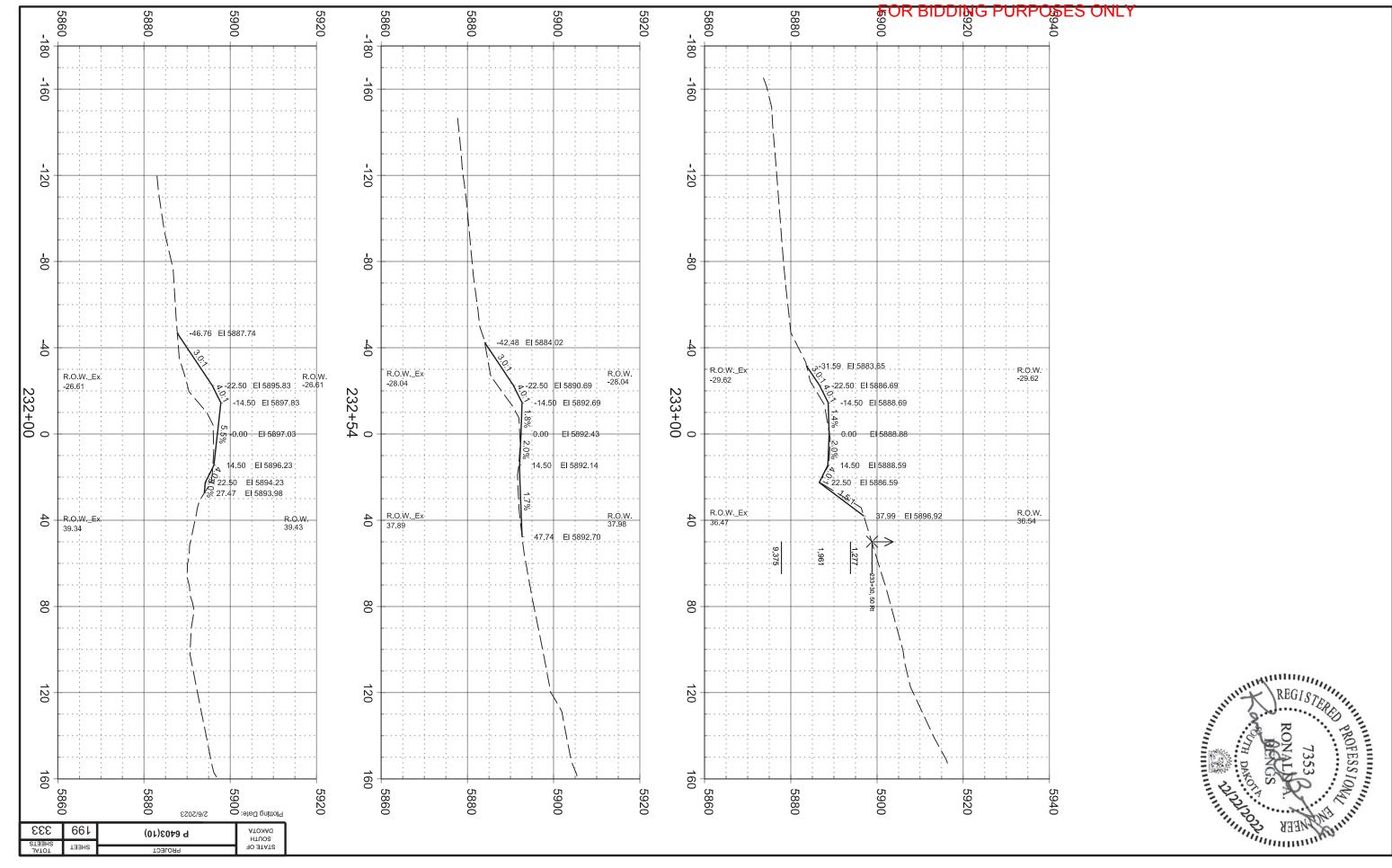


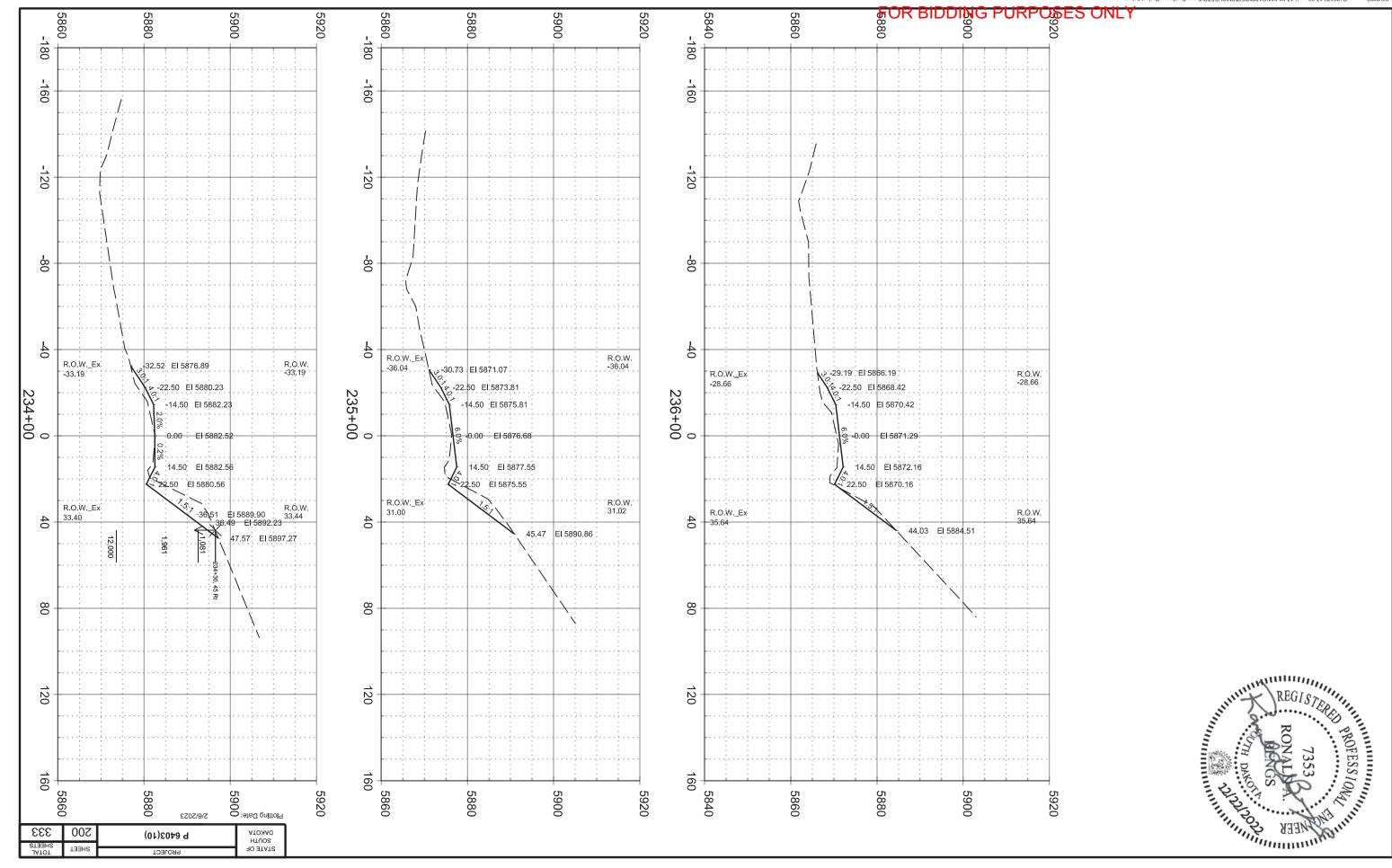


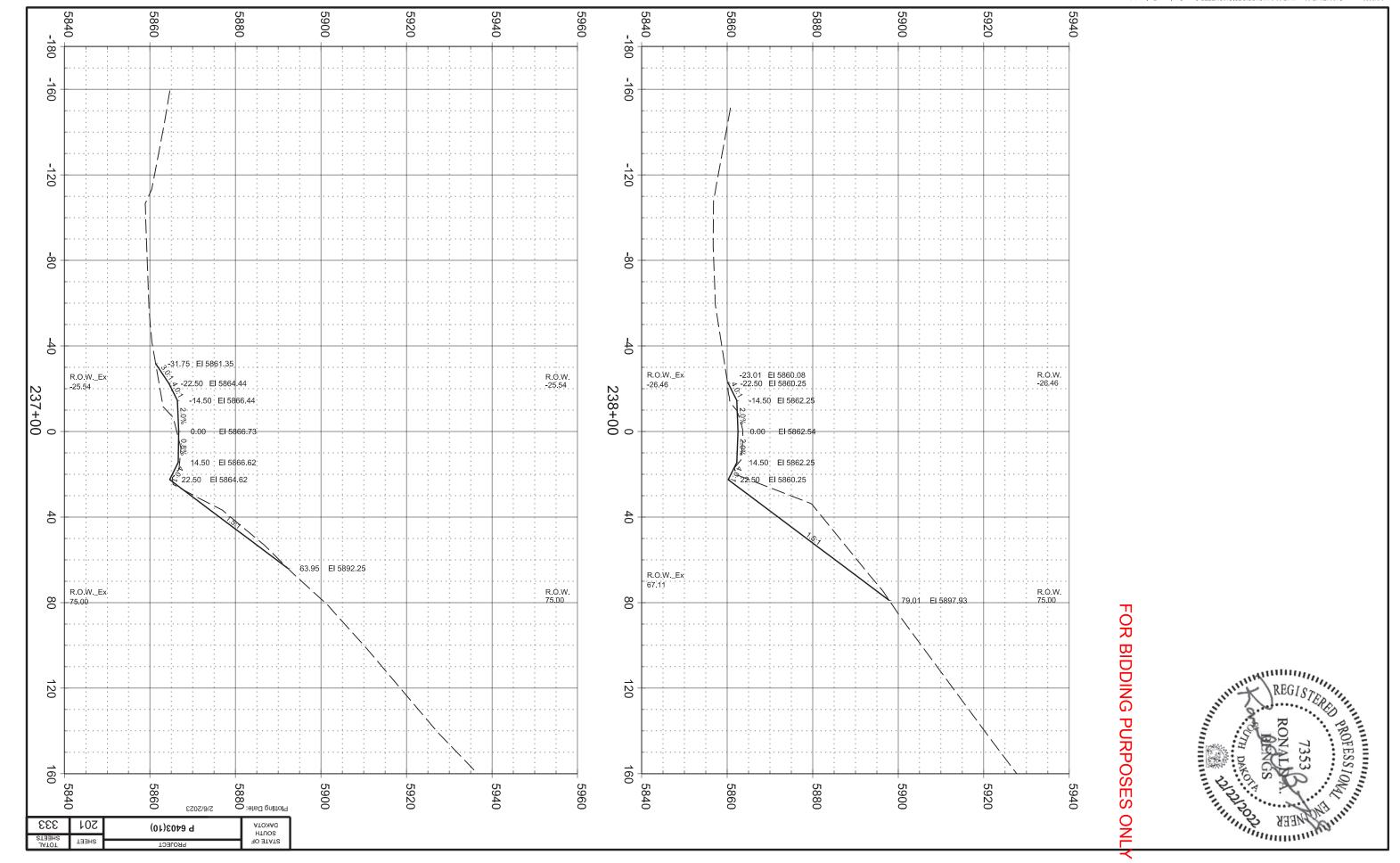


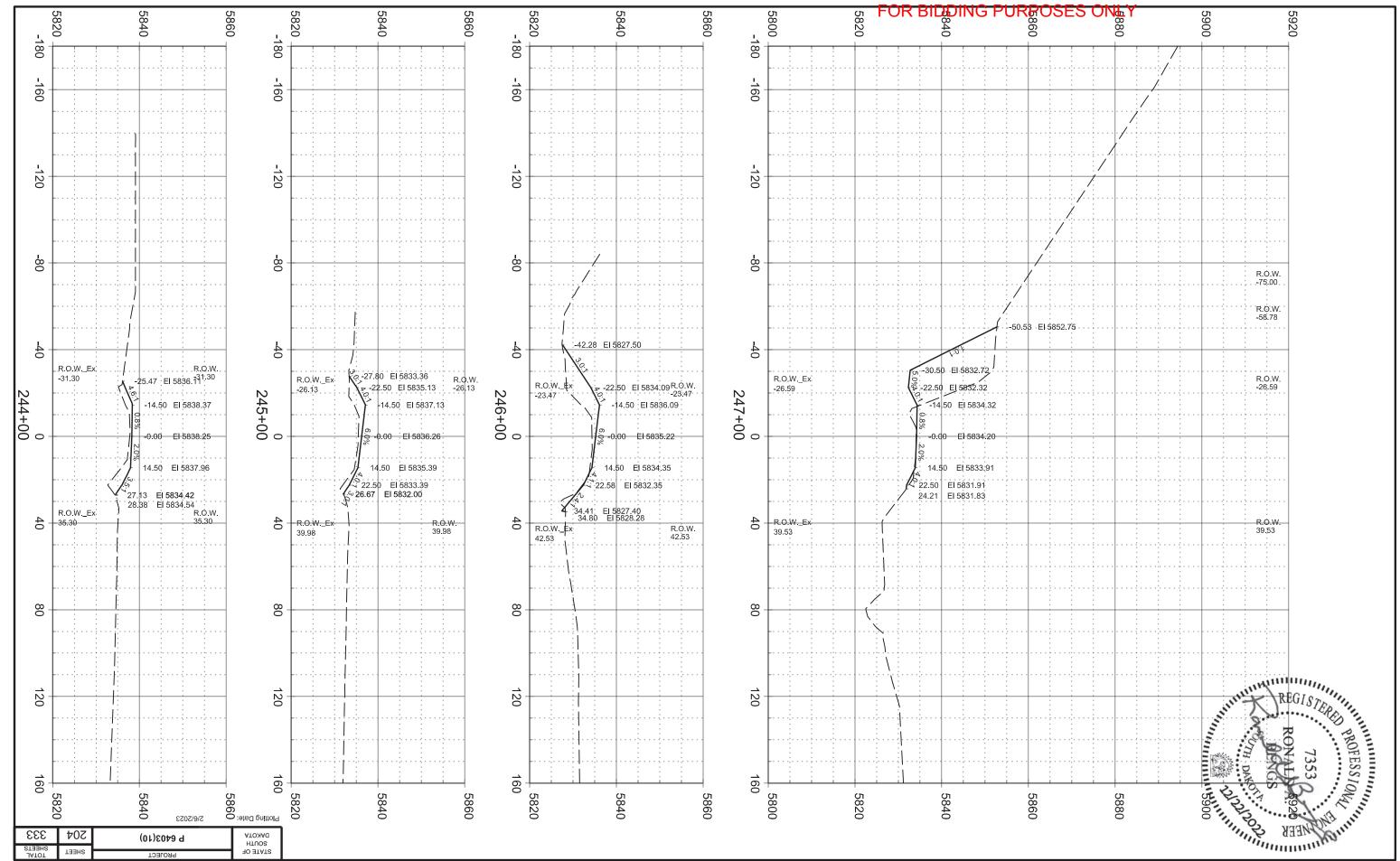


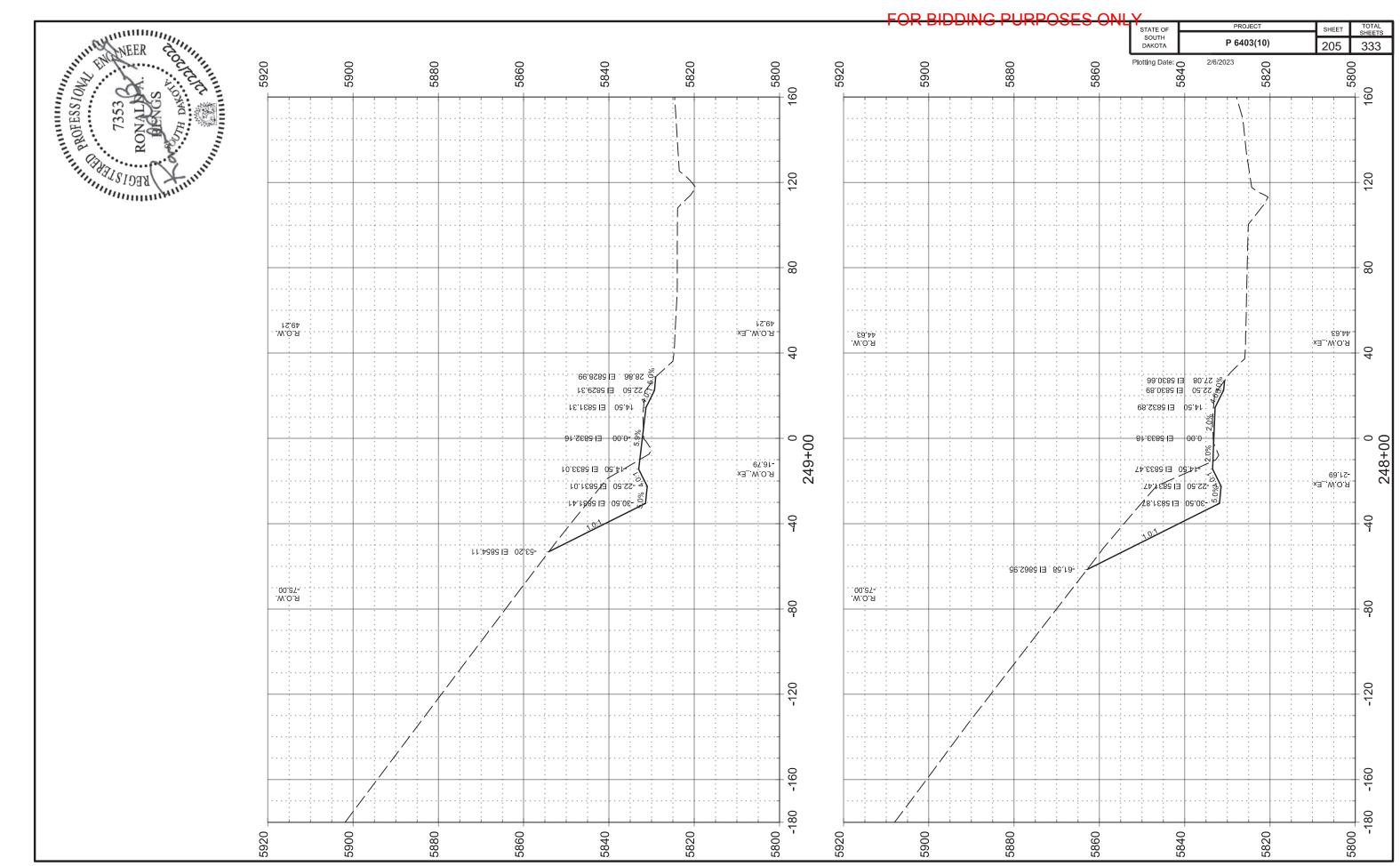






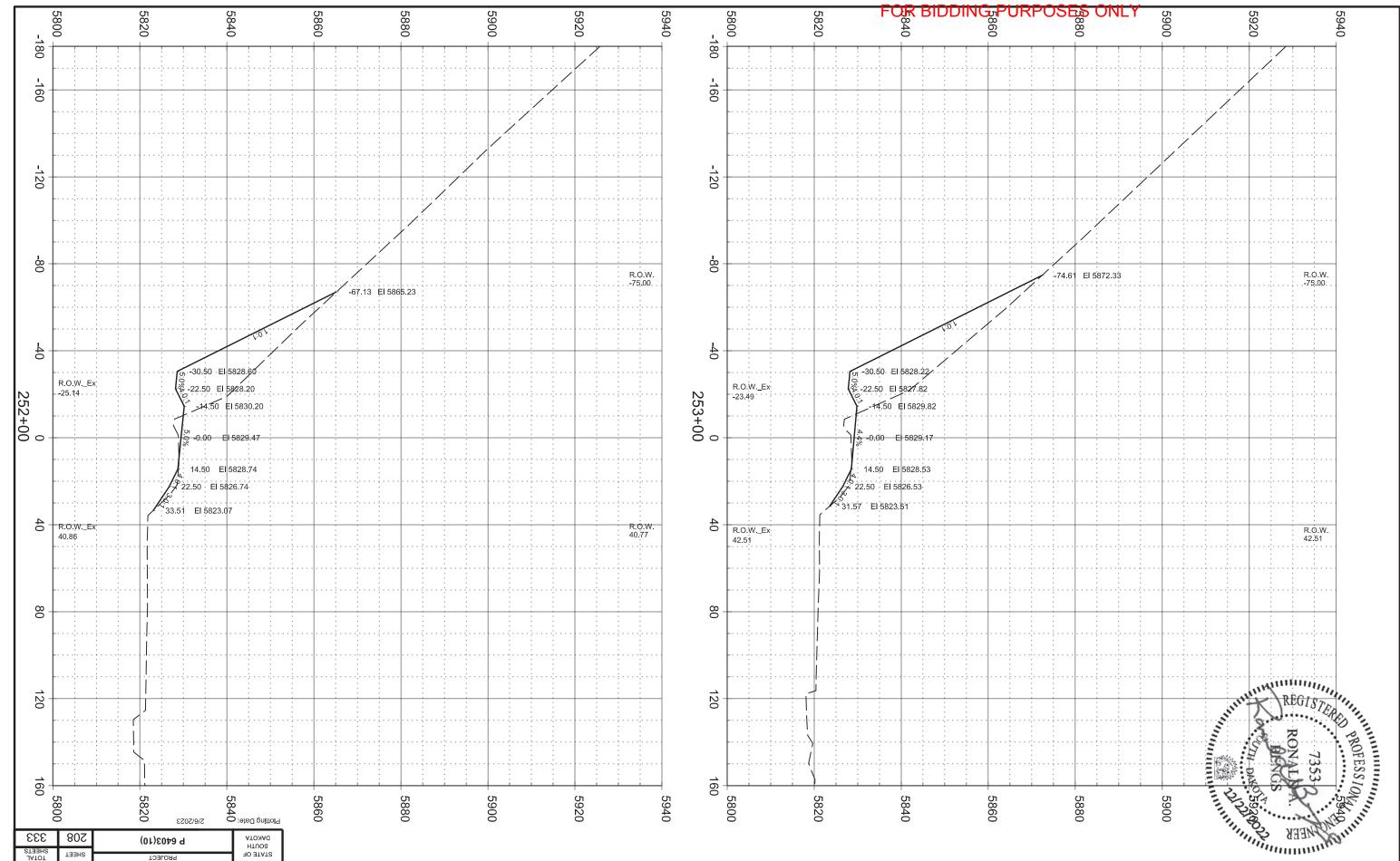




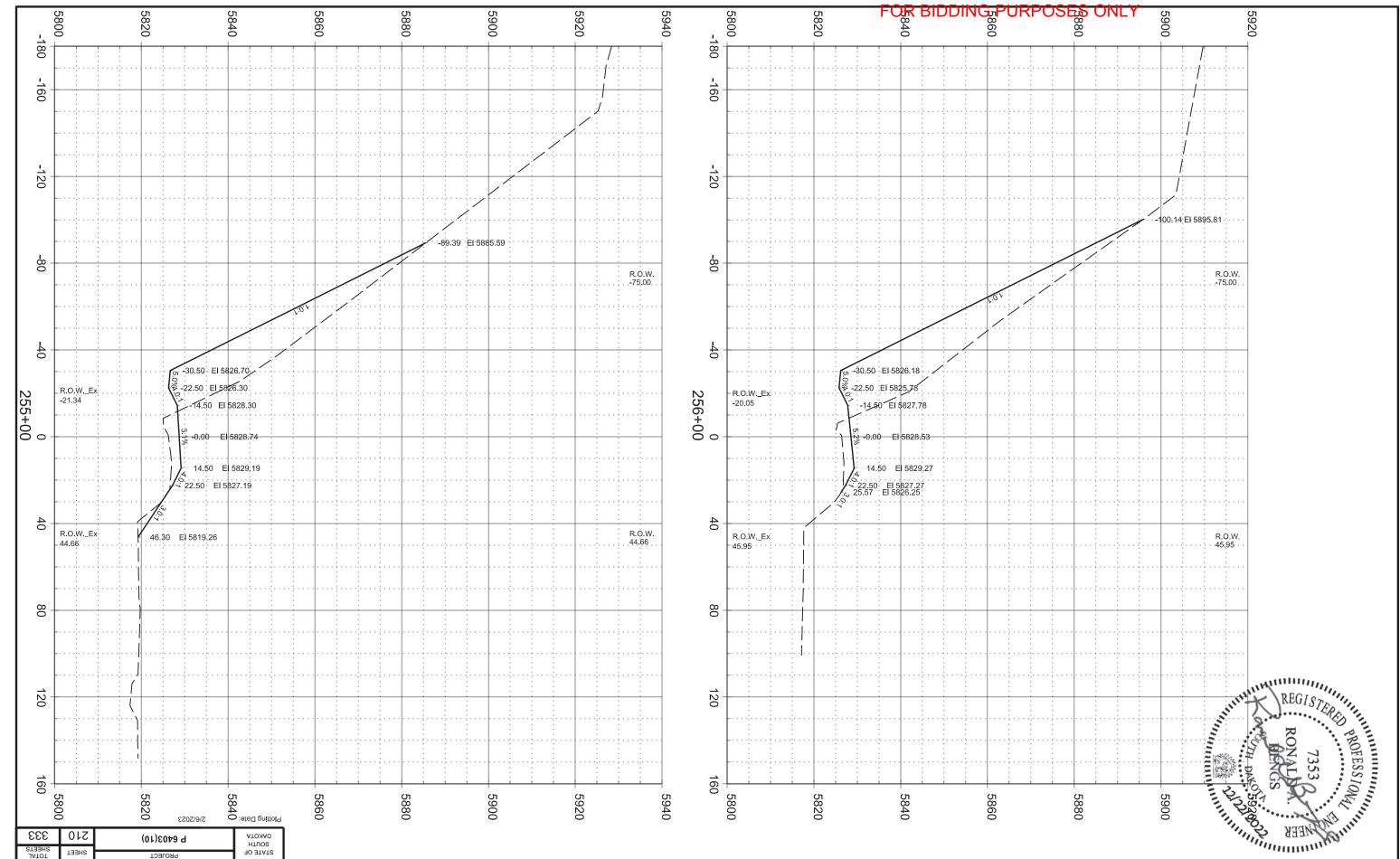


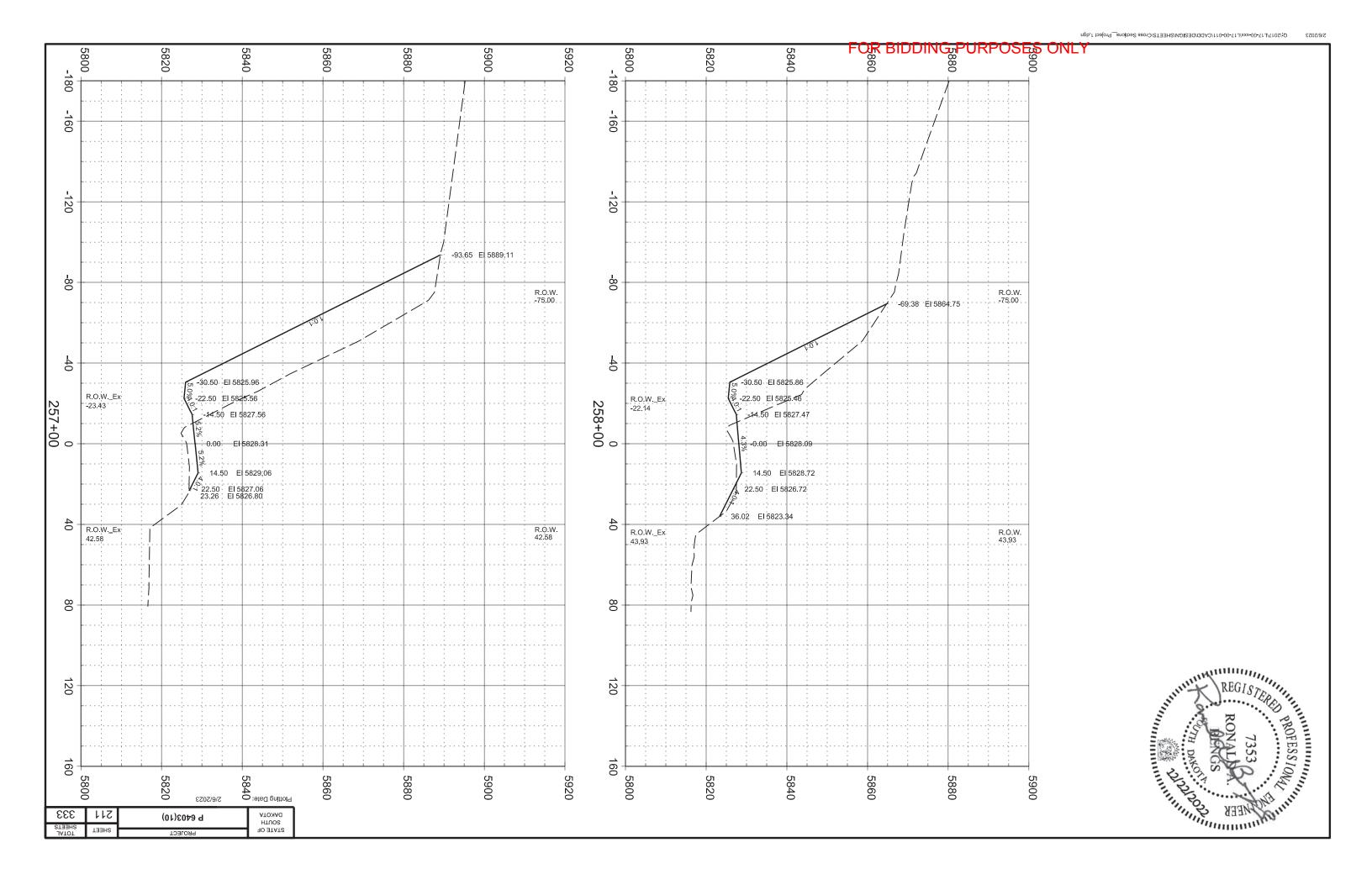
FOR RIDDING PURPOSES ONLY RONAL DAYOUT STATE OF SHEET SOUTH P 6403(10) 206 333 Plotting Date: 048 2/6/2023 5940 5880 5820 5800 5900 5860 160 120 80 F O W $\mathsf{FOW}_{\mathsf{EX}}$ 40 26.75 El 5828.40 22.50 EI 5828.45 14.50 EI 5830.45 0 250+00 +1 12831 11 00 0 -14 50 El 5831 82 -20.36 \$ -22.50 El 5829.82 -x∃_-W-O-Я-ZZ 0889 I∃ 09 06- 1 40 .00.27-96.1788 IE 8871.36 R.O.W. -80 -160 -180 5800 5920 5900 5880 5860 5840 5820

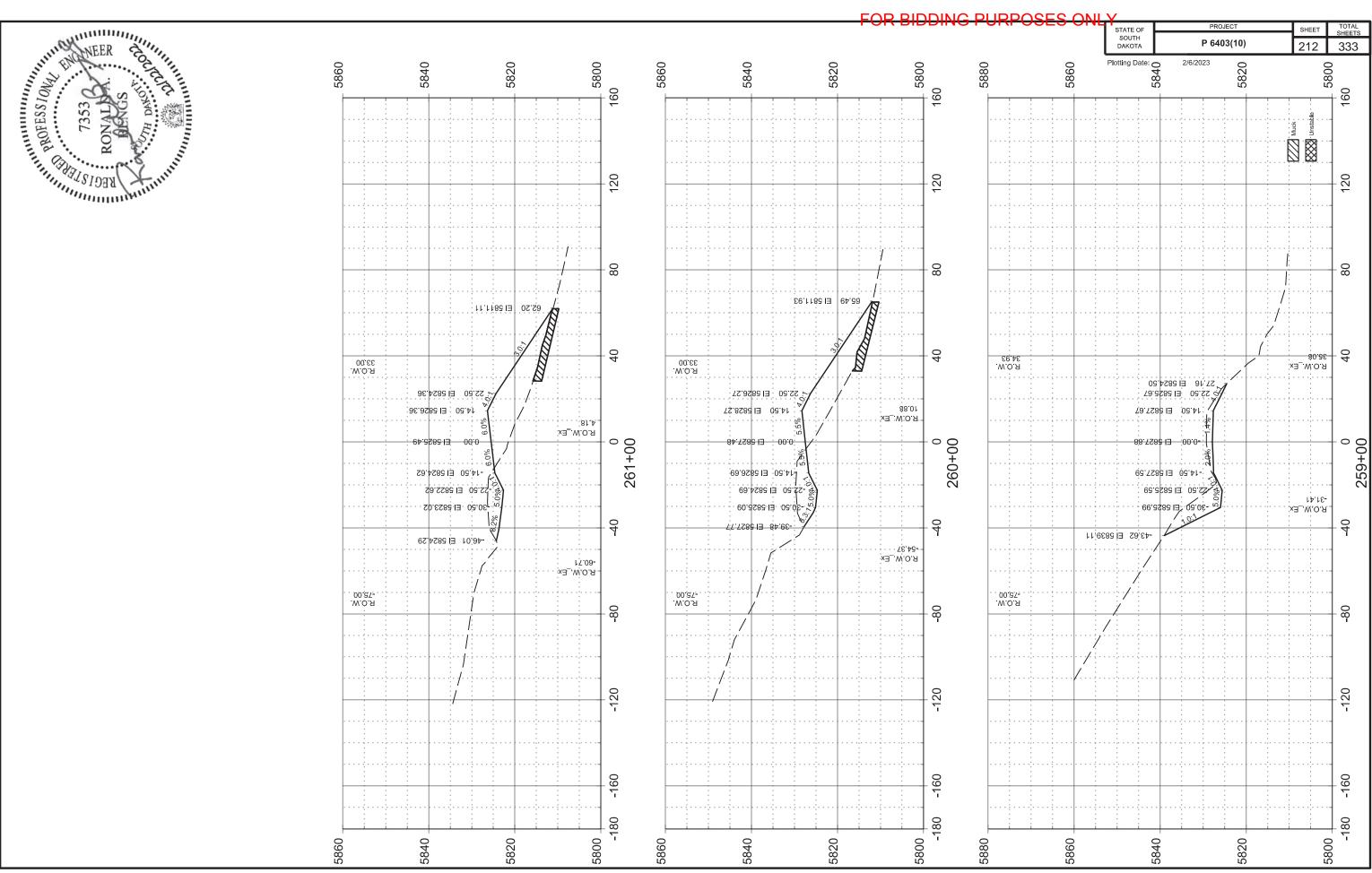
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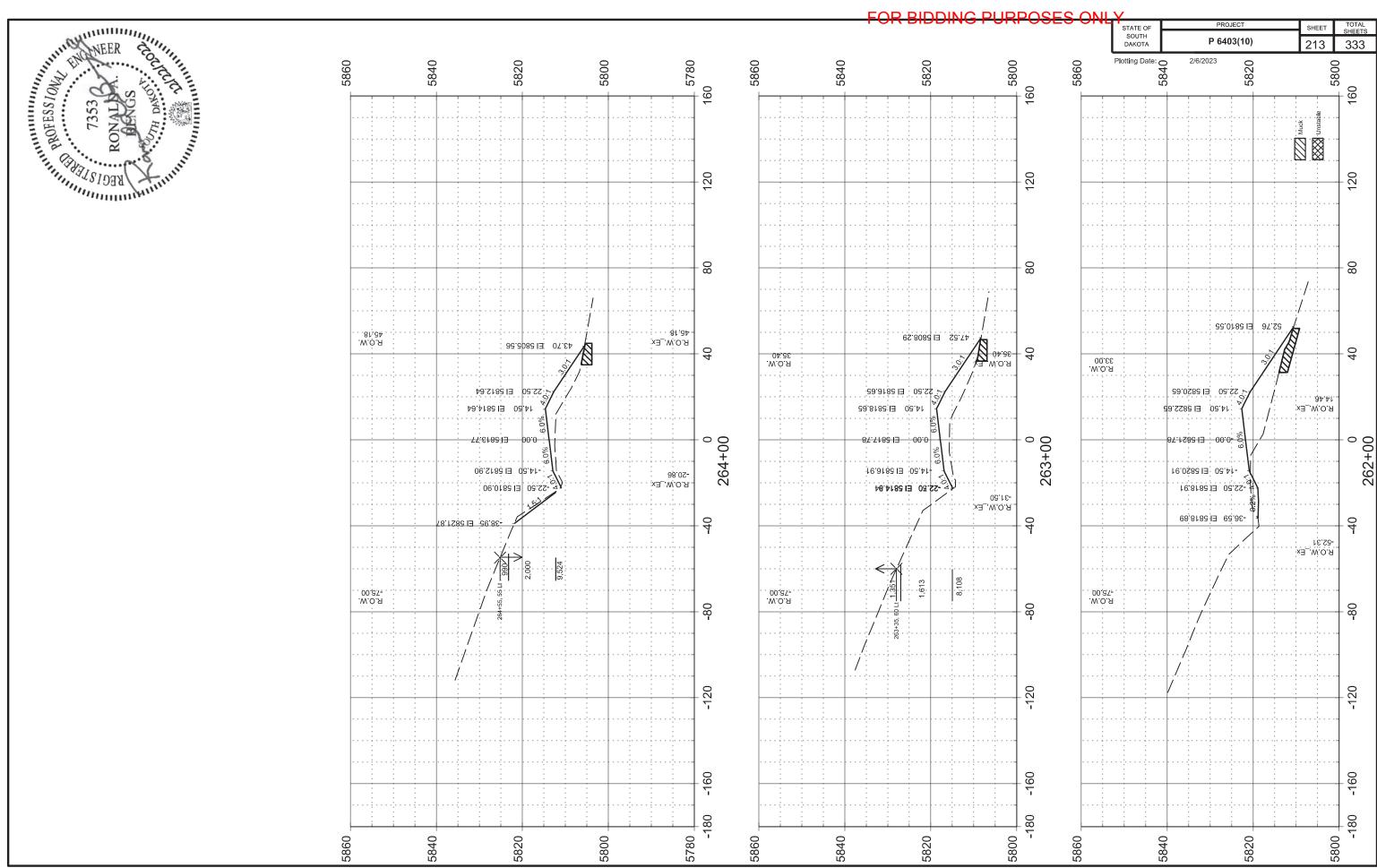


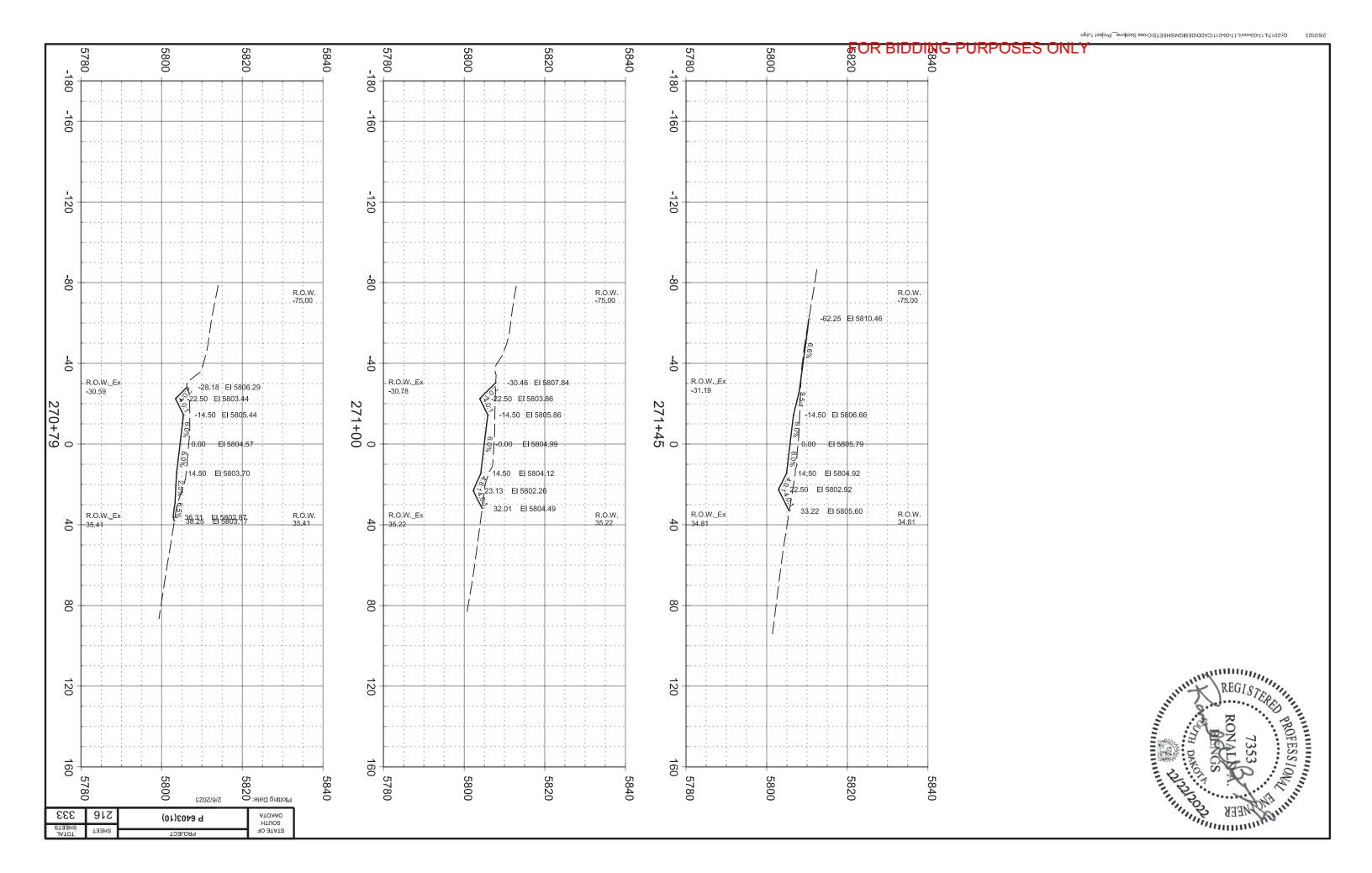
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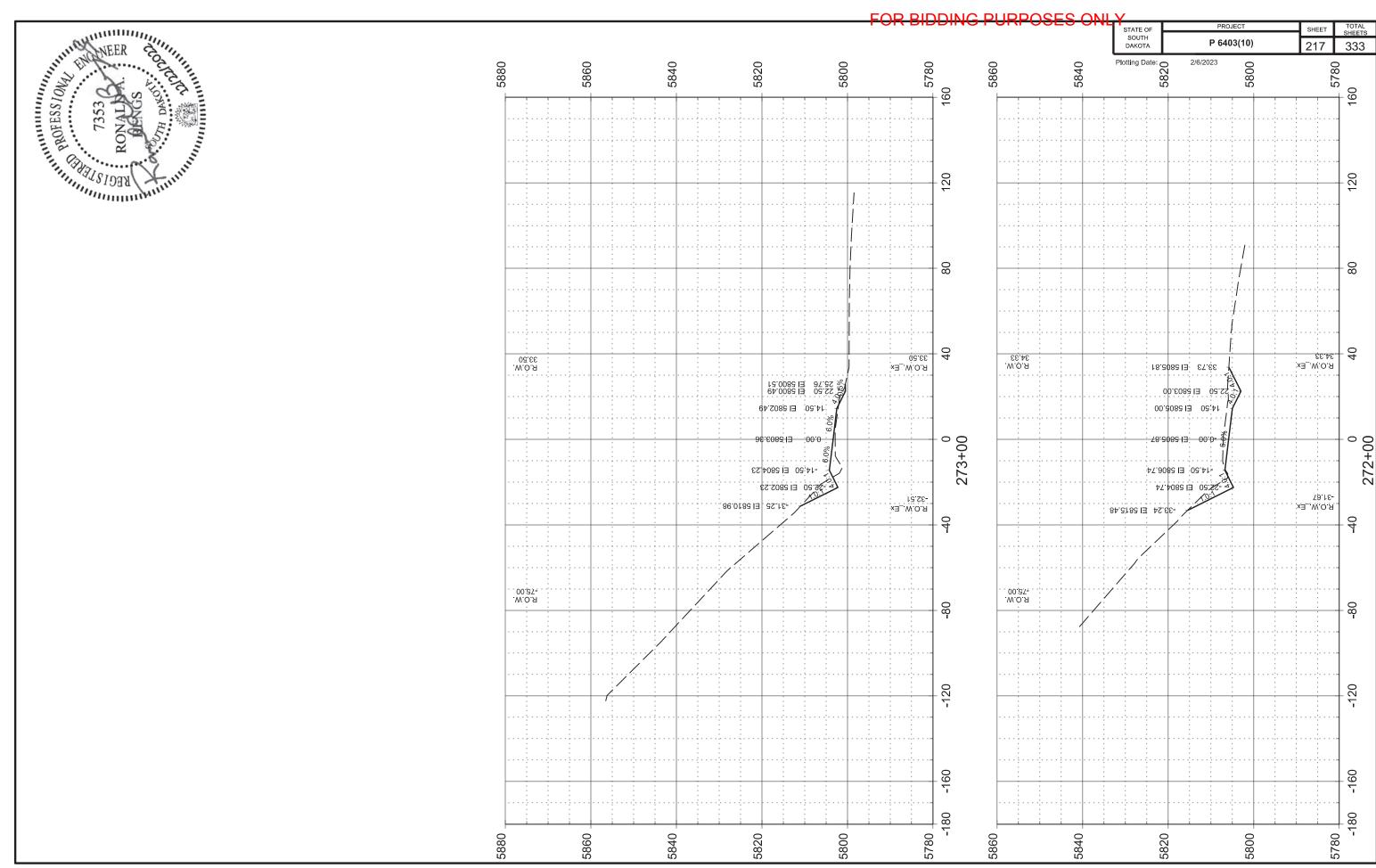


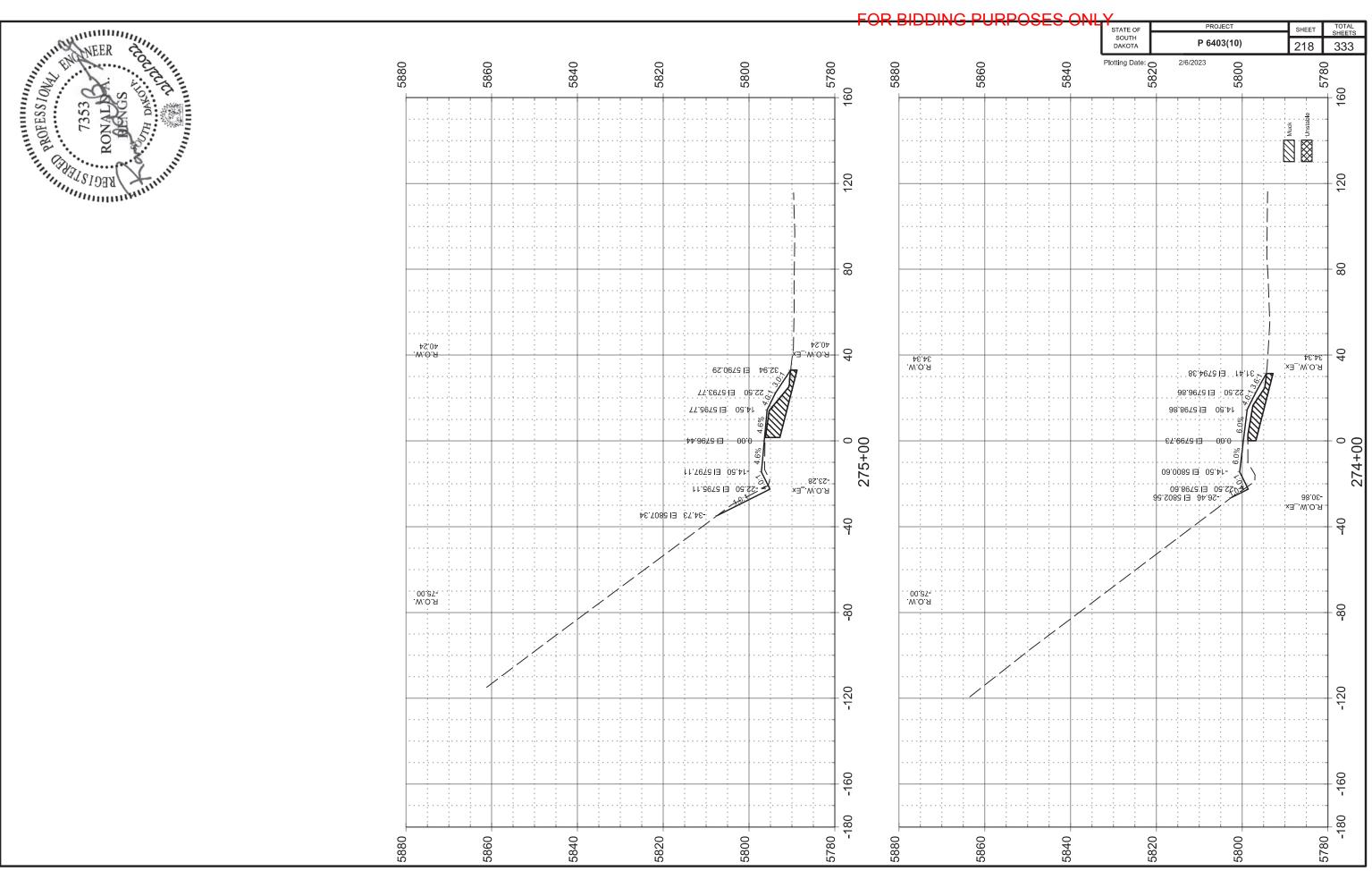








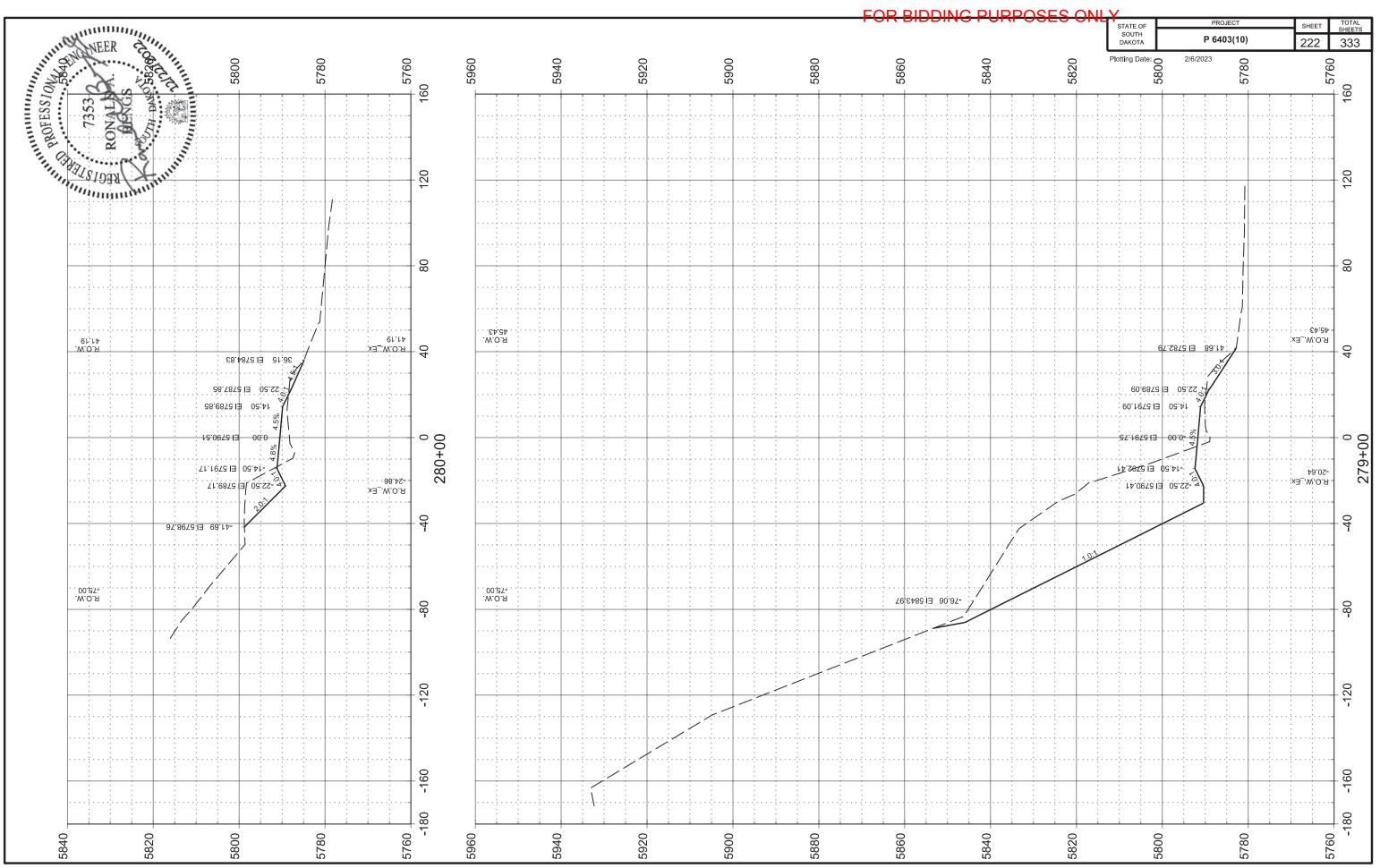


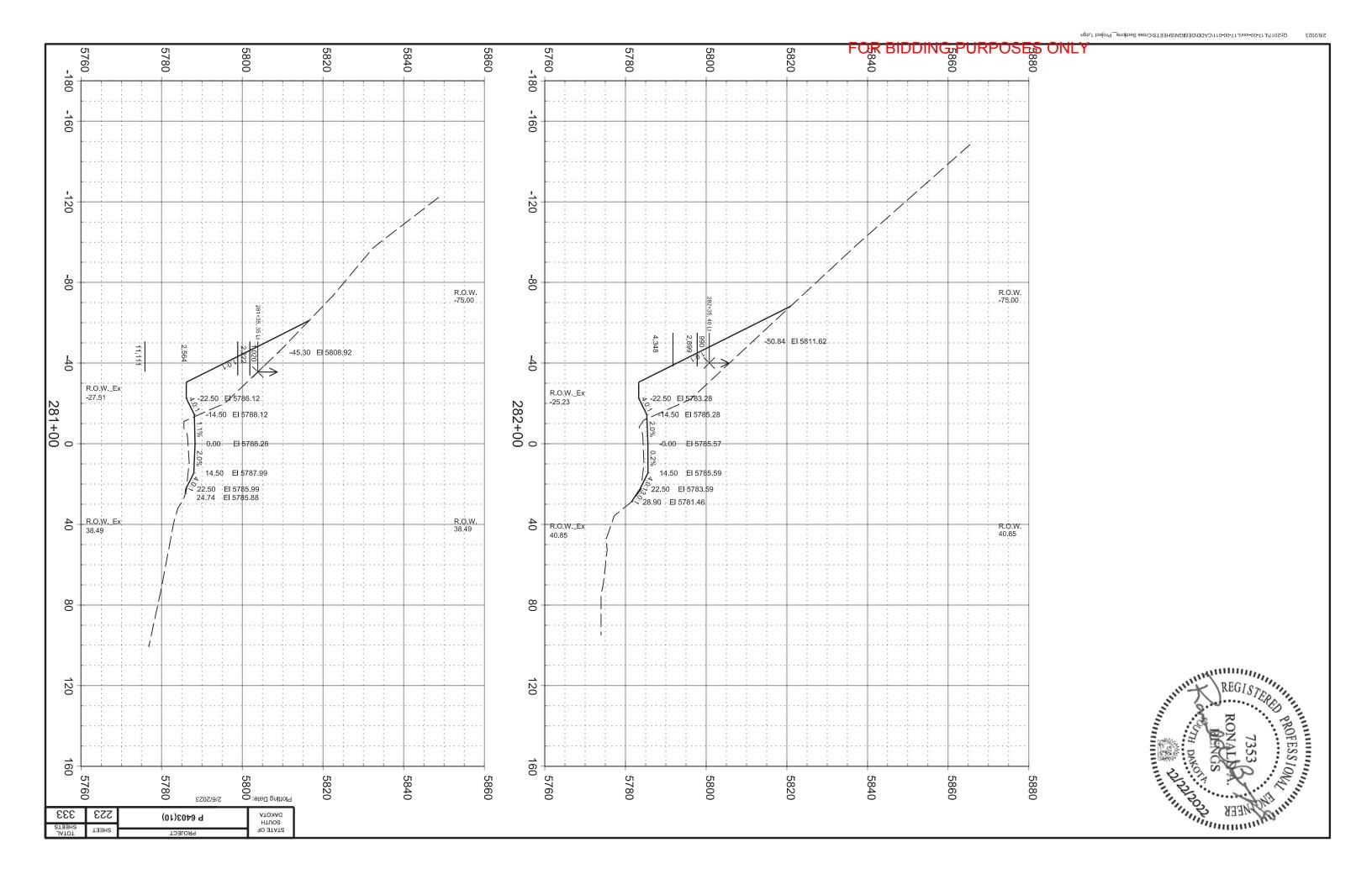


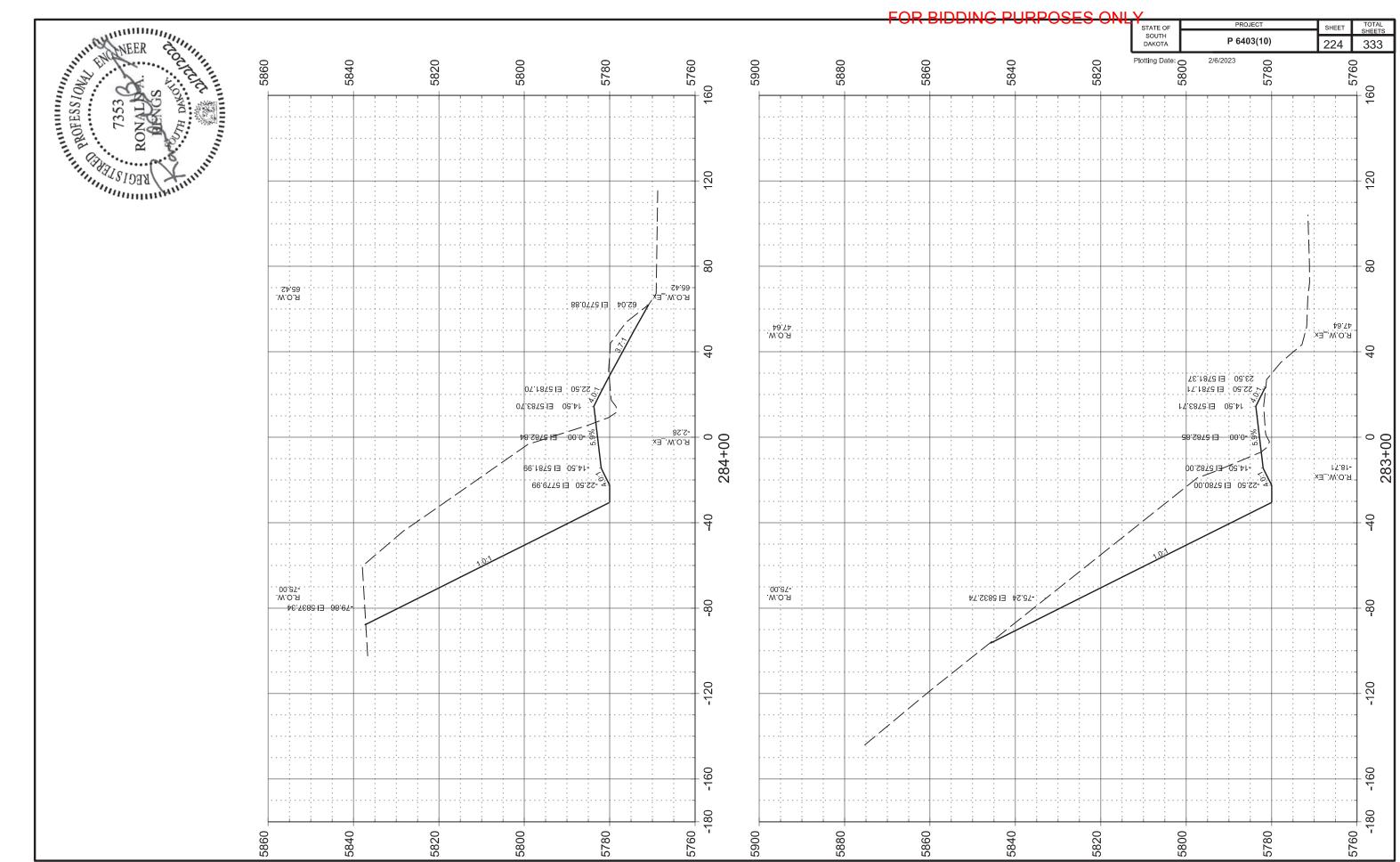
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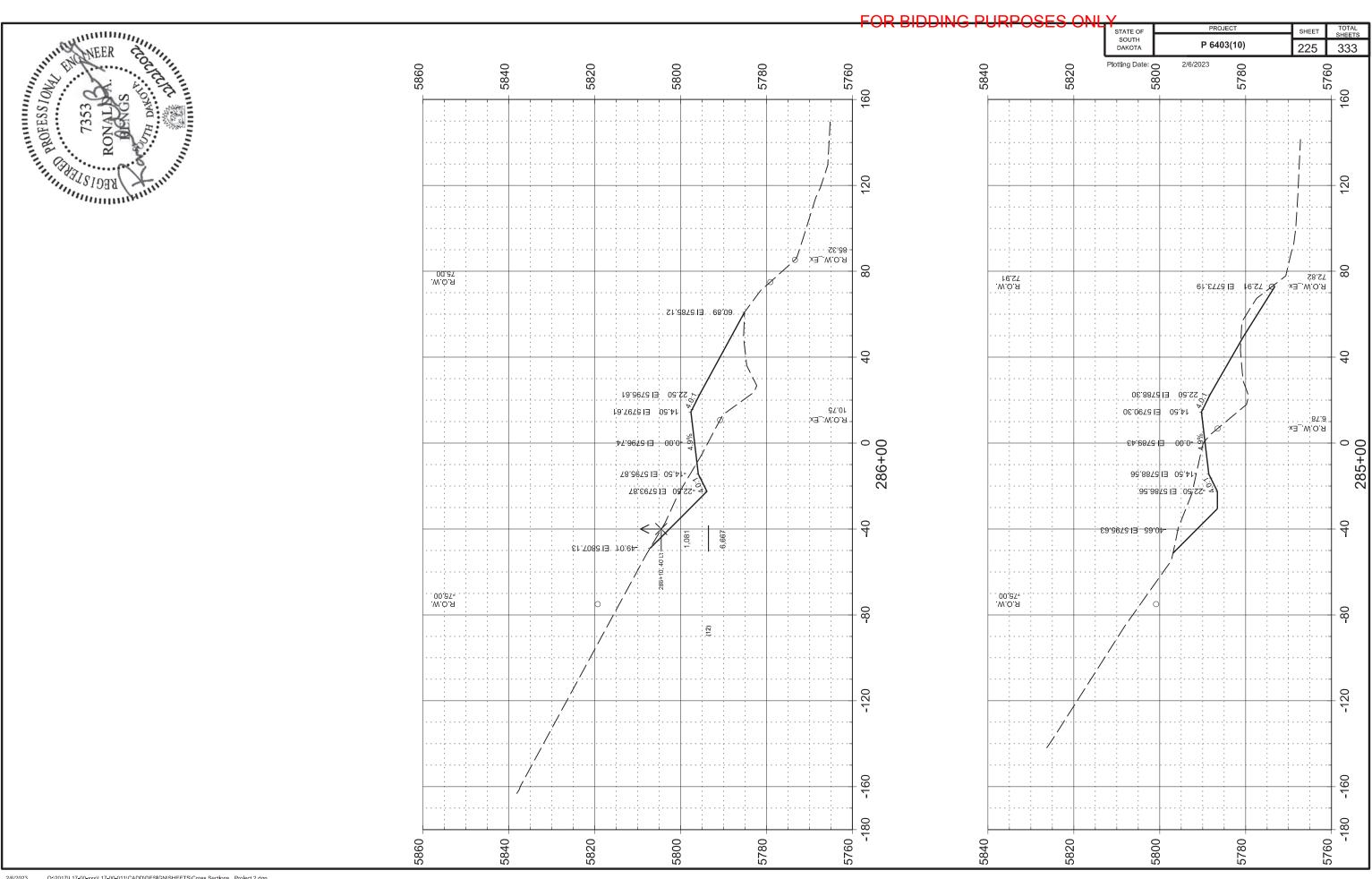
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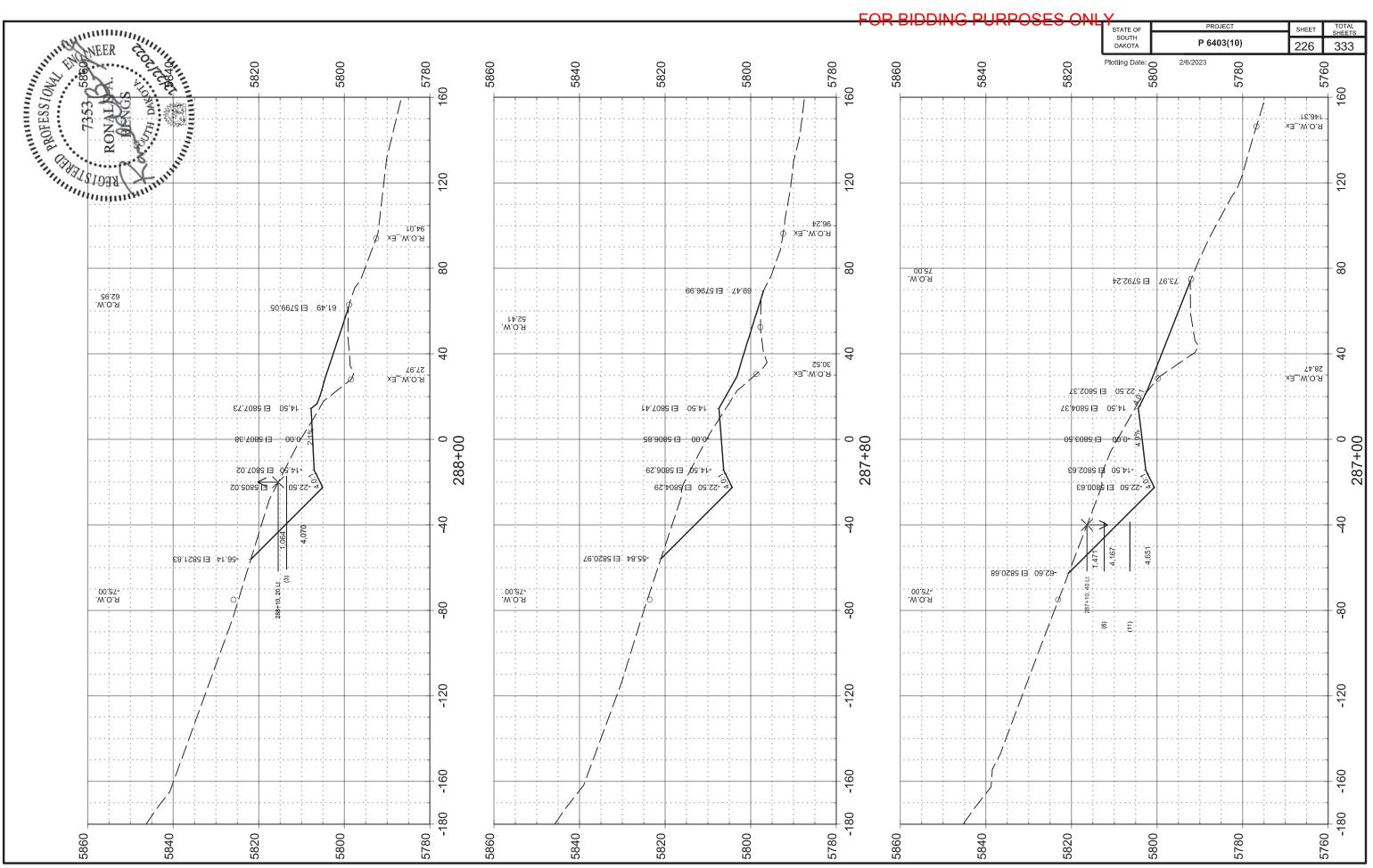
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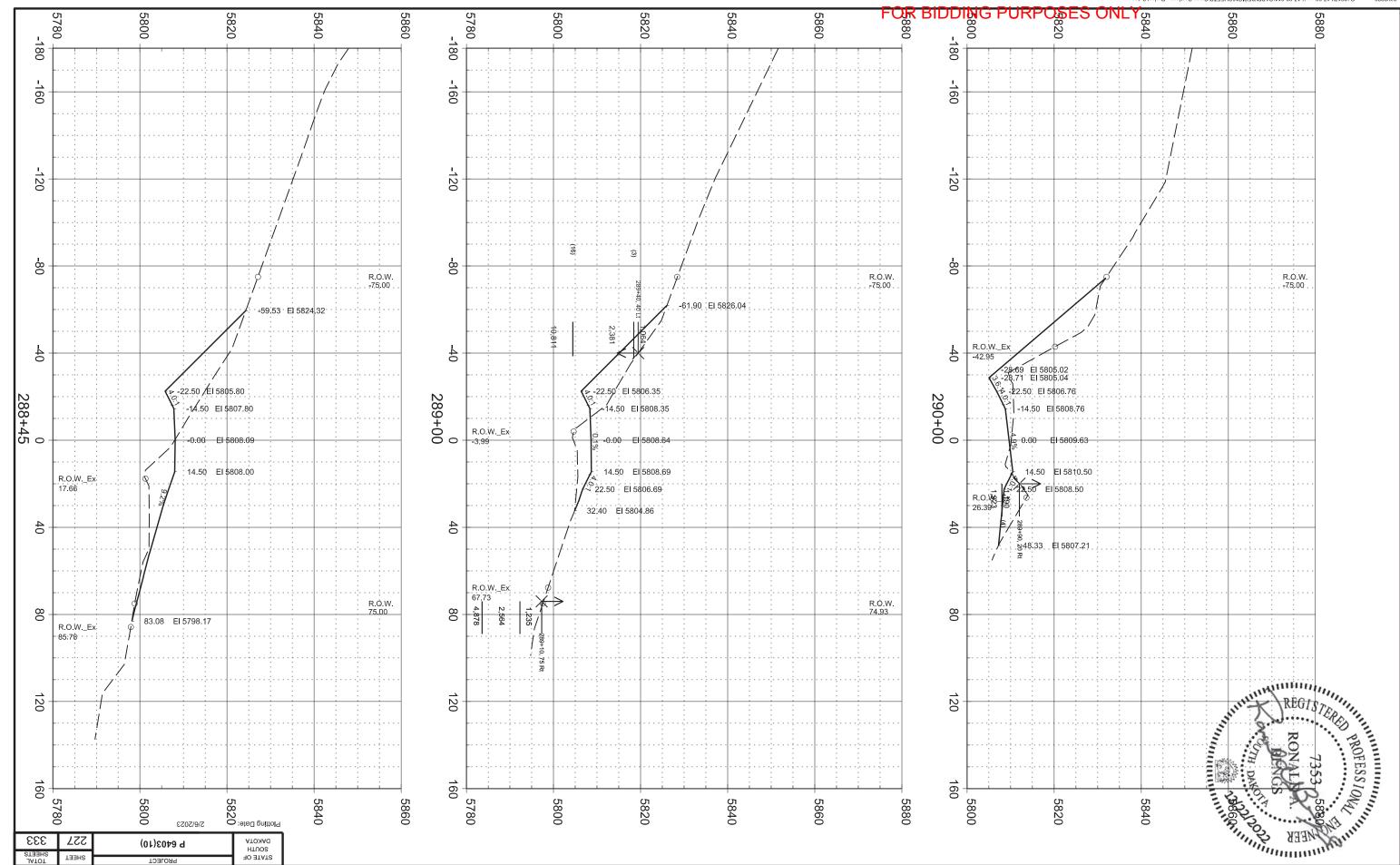




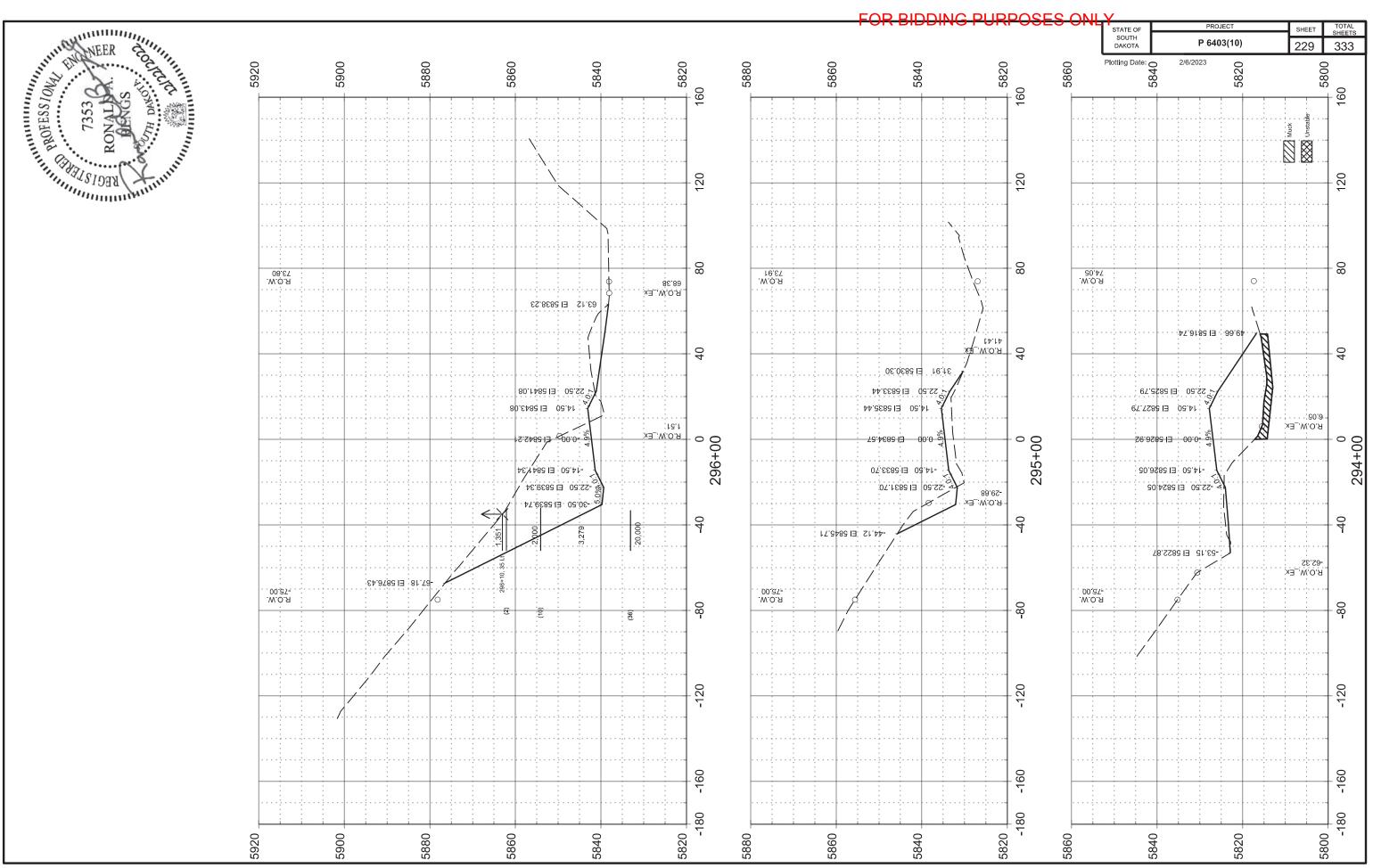


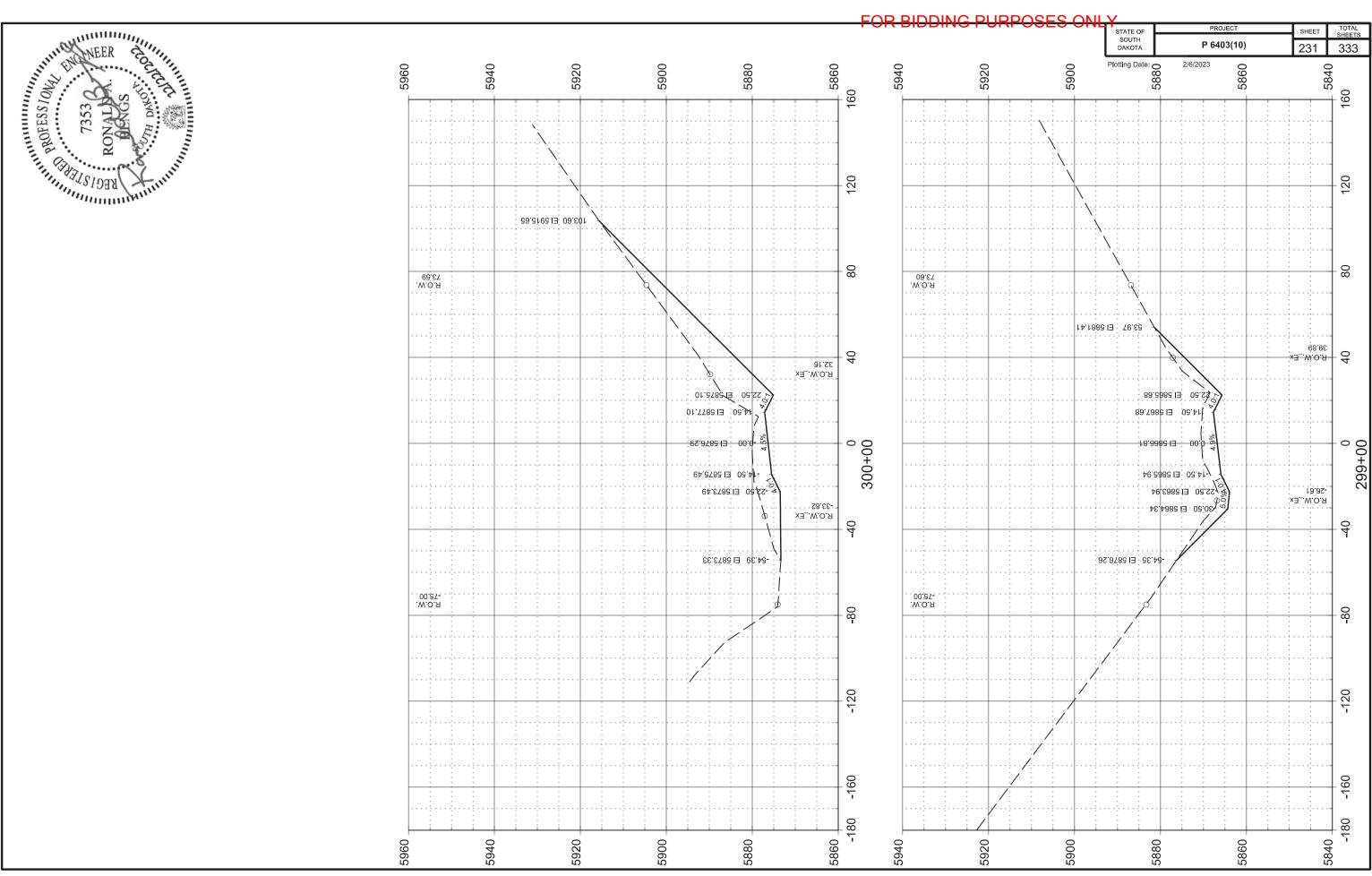


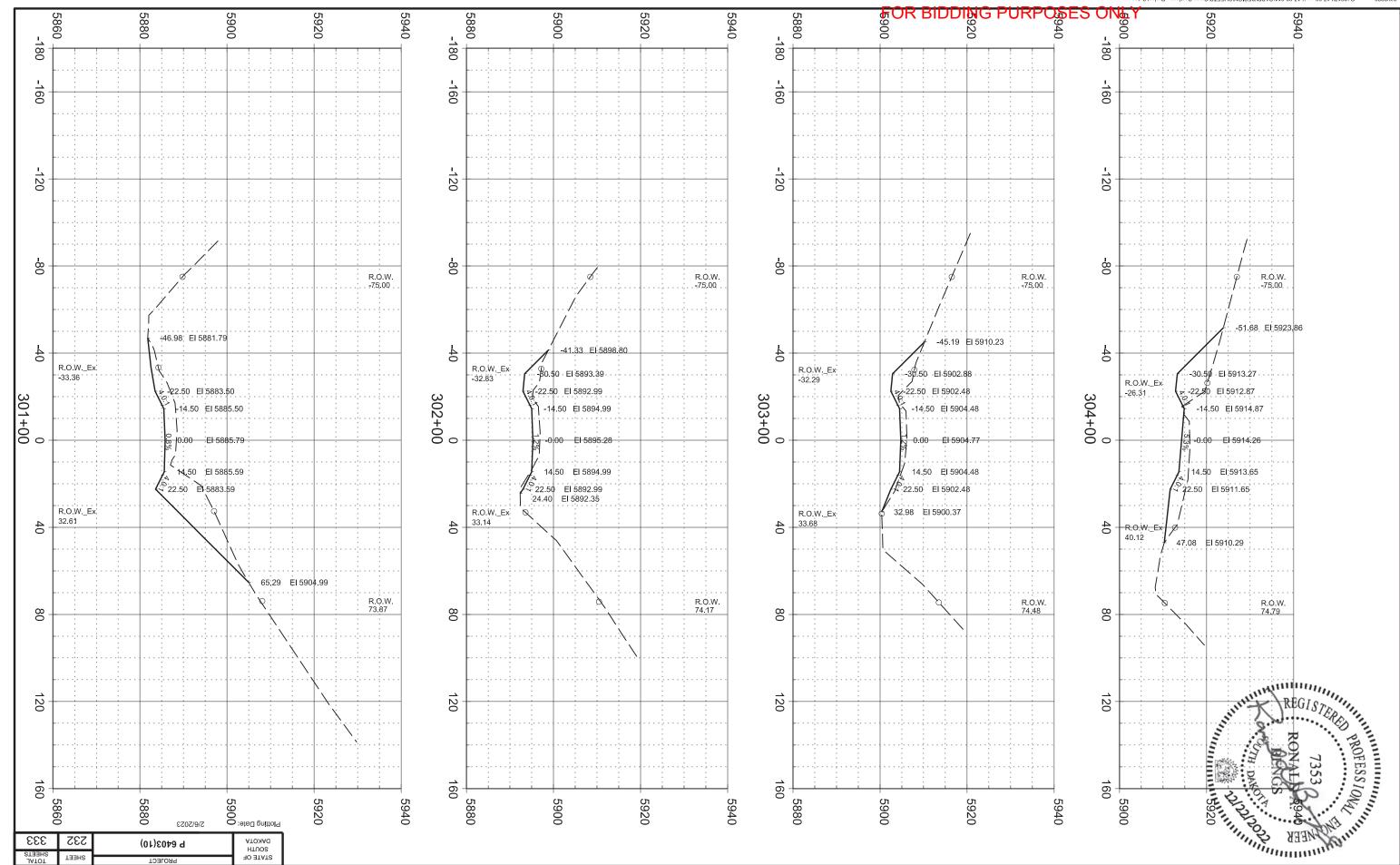


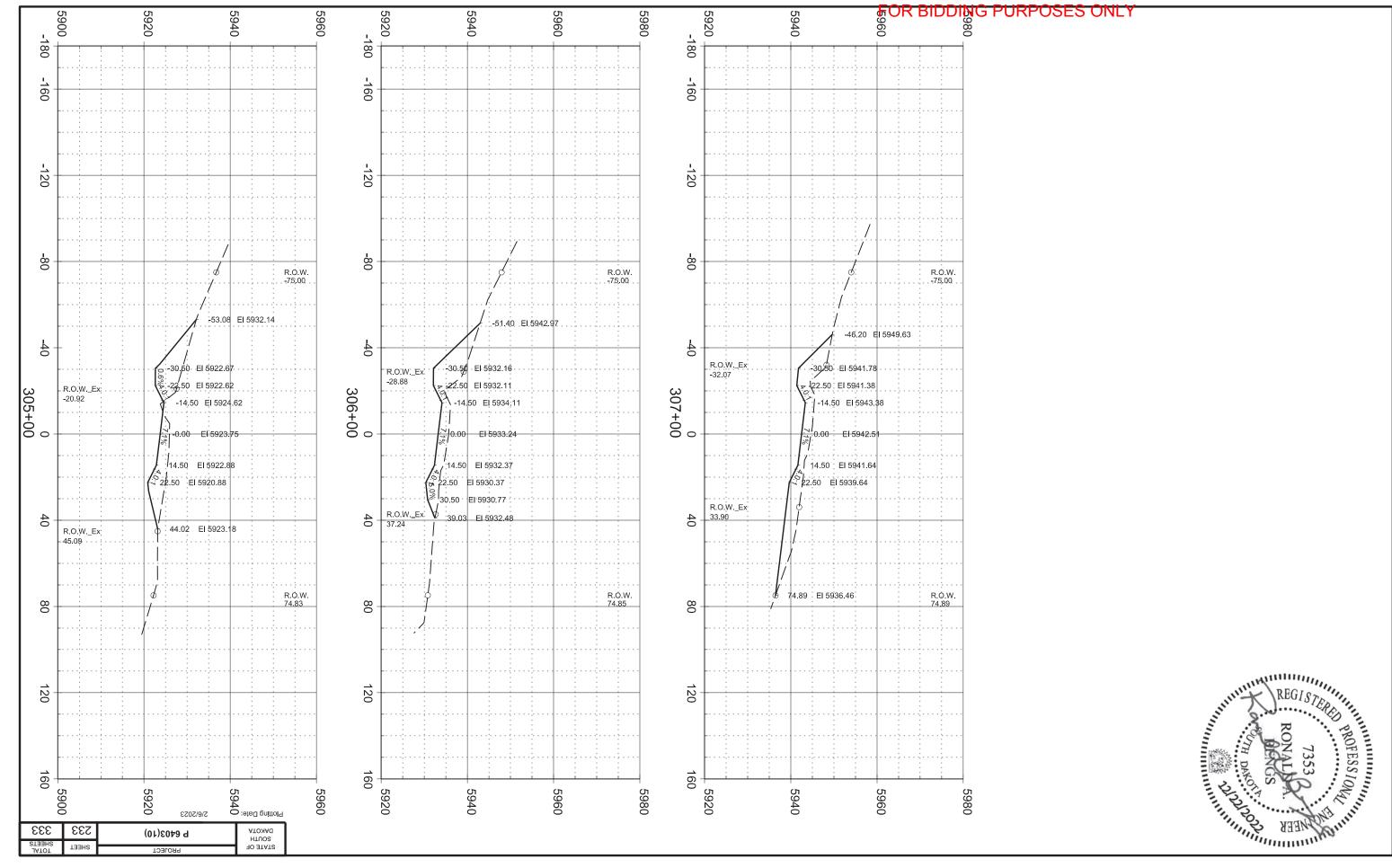


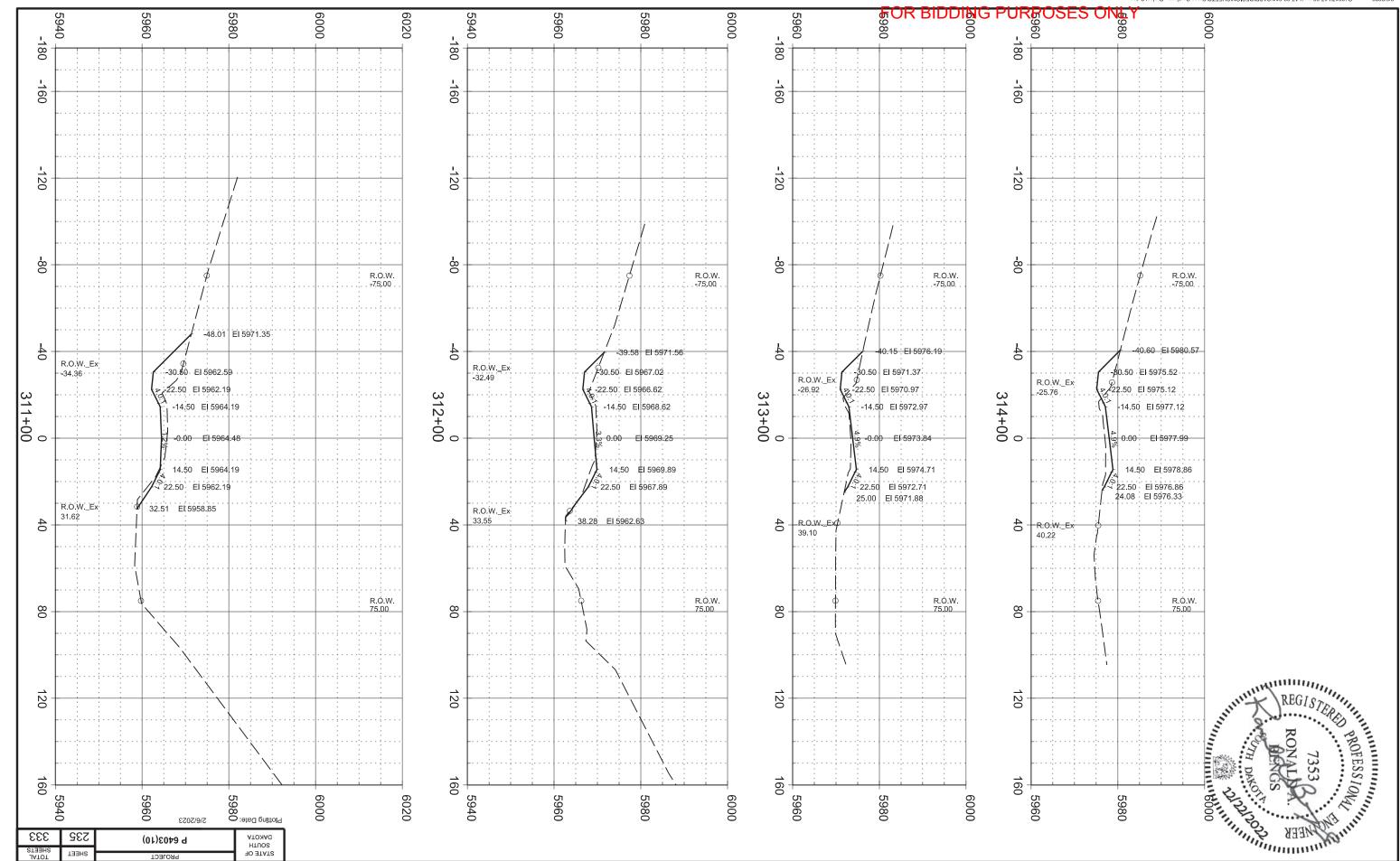
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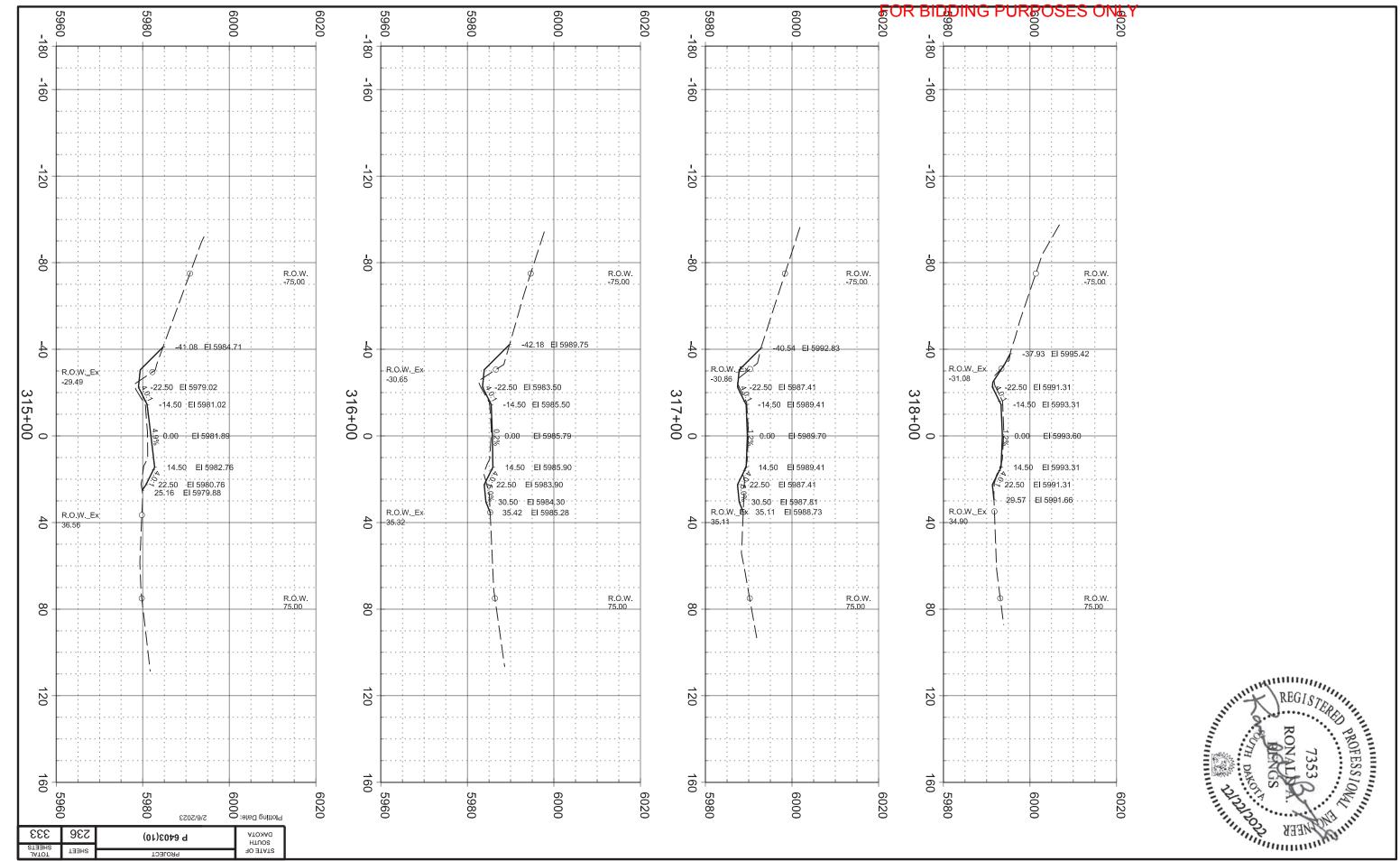


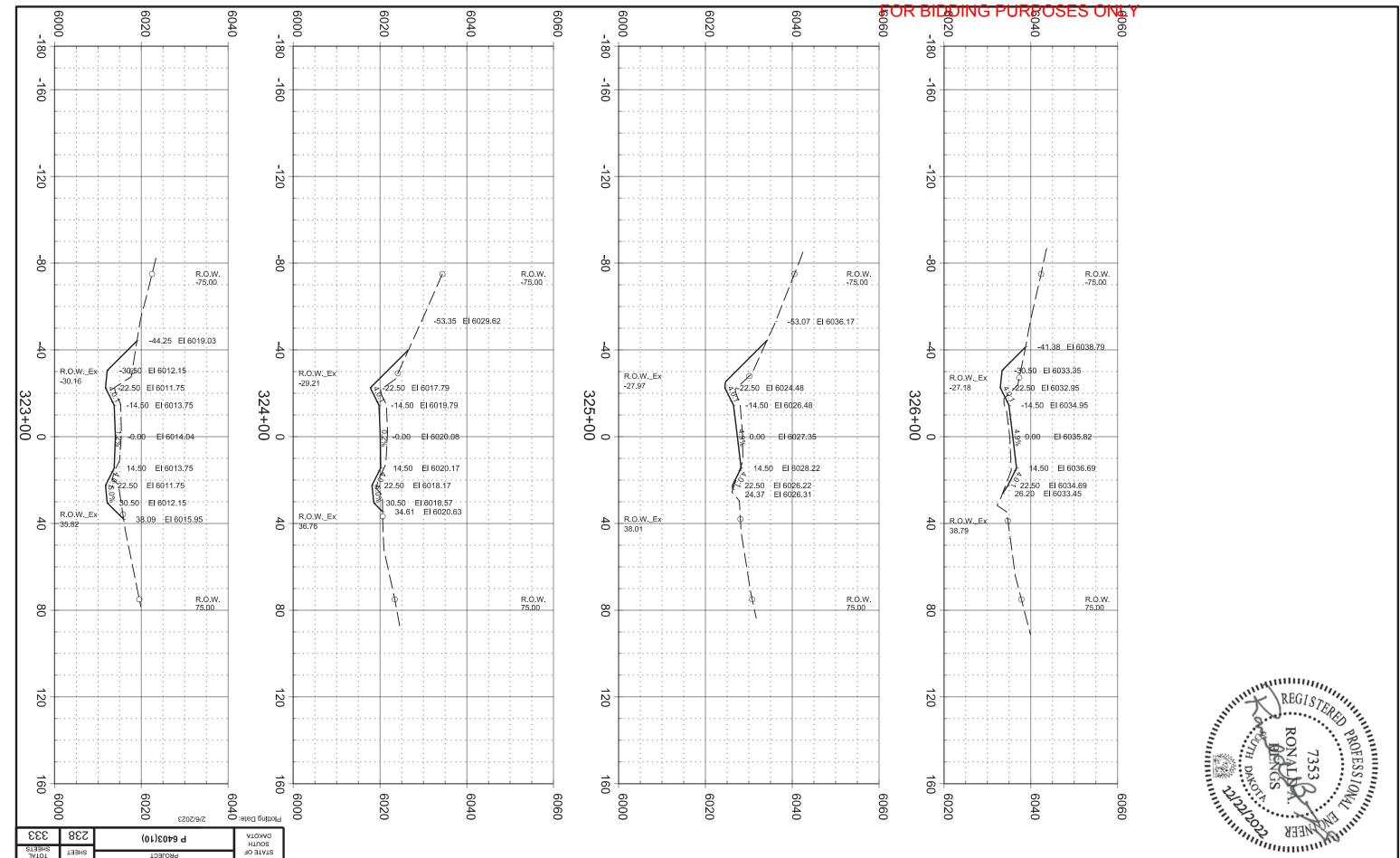


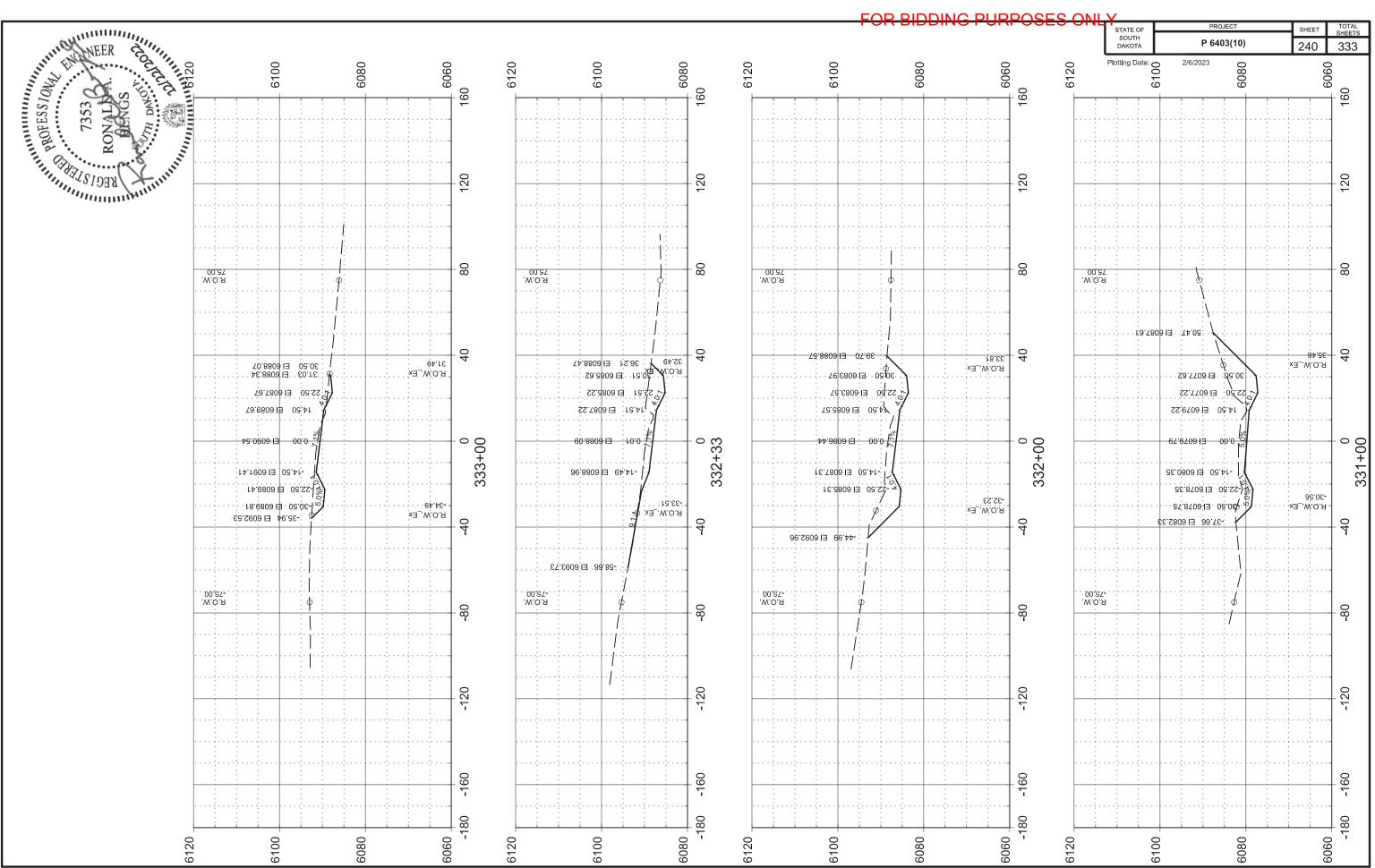


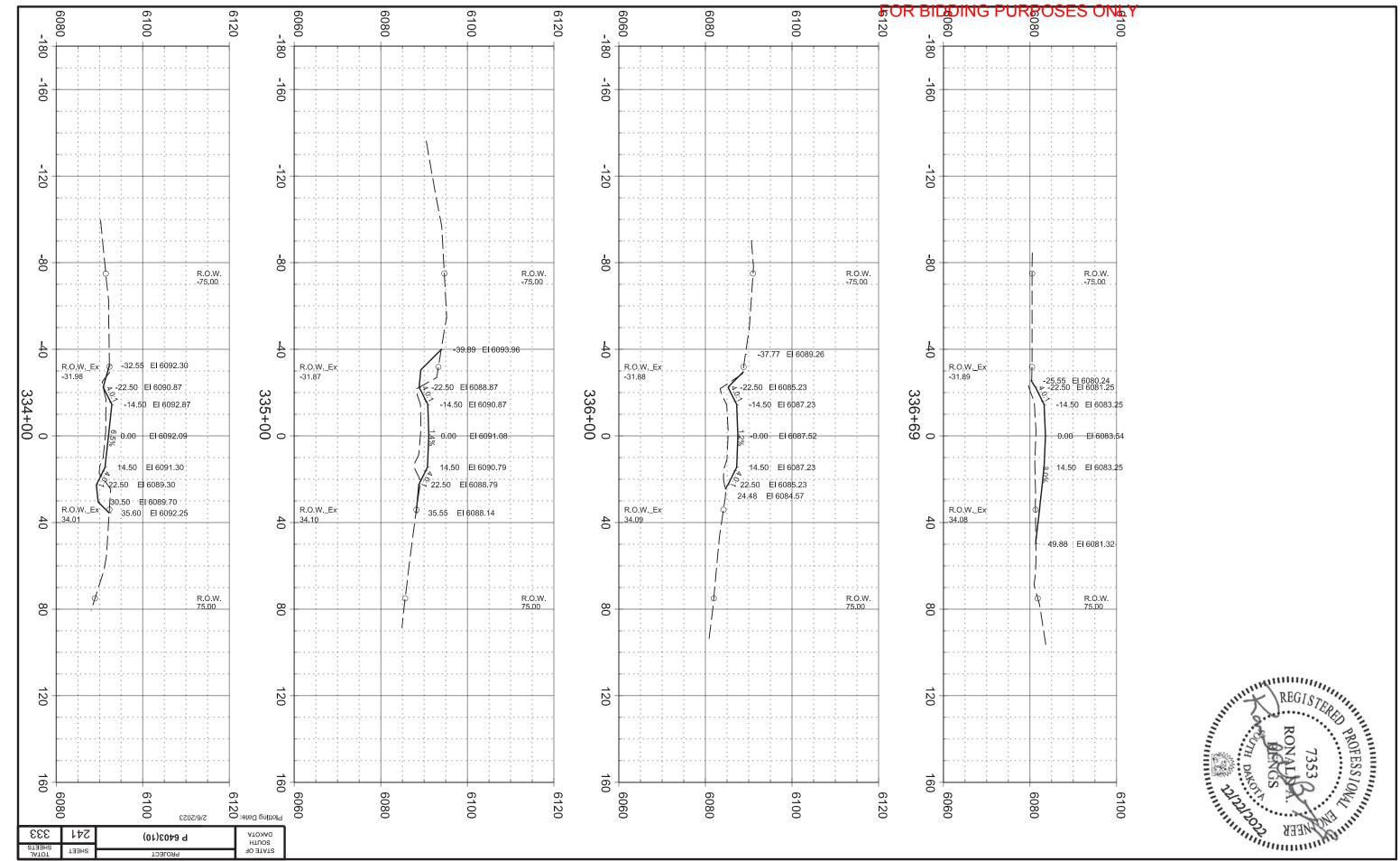


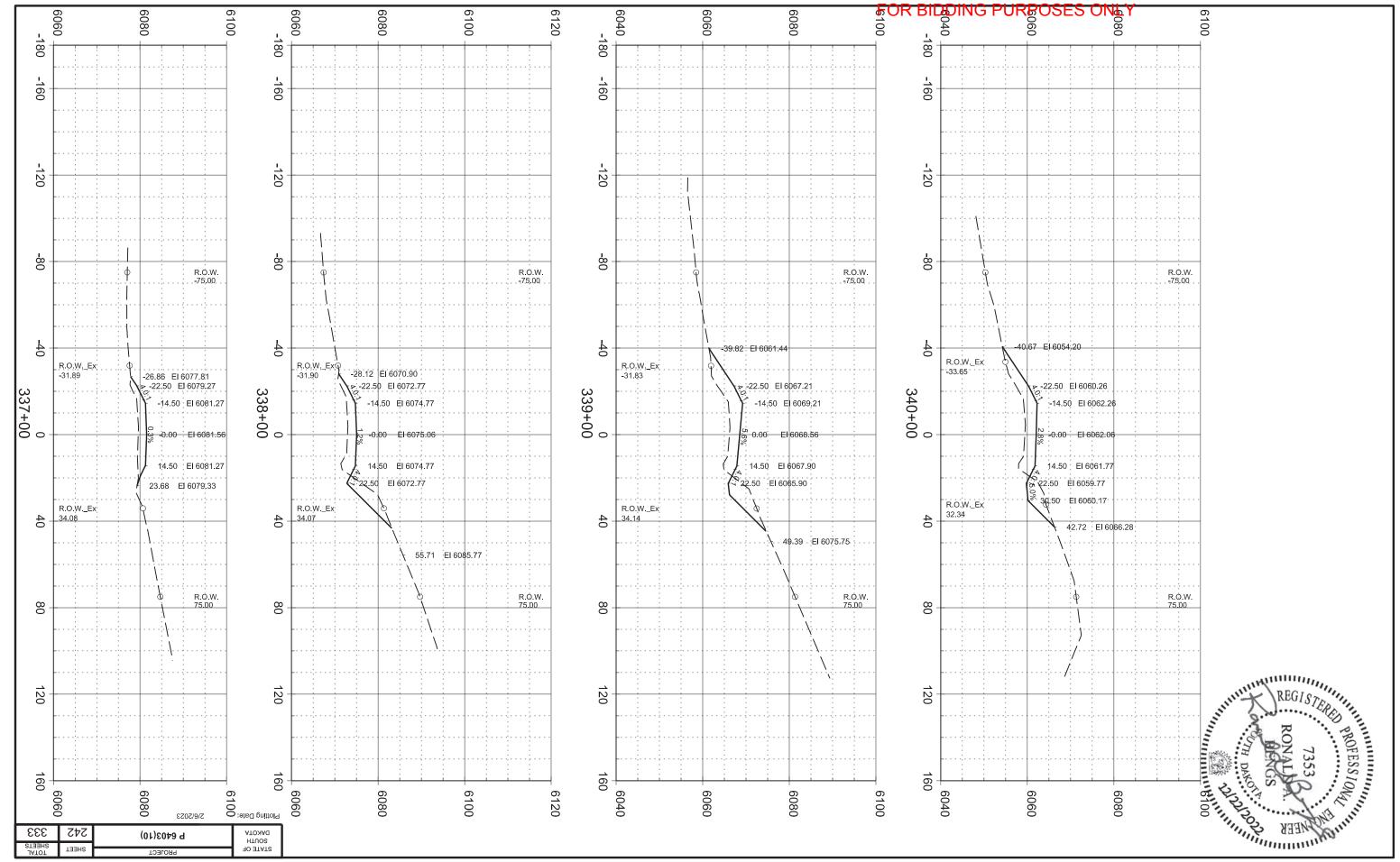


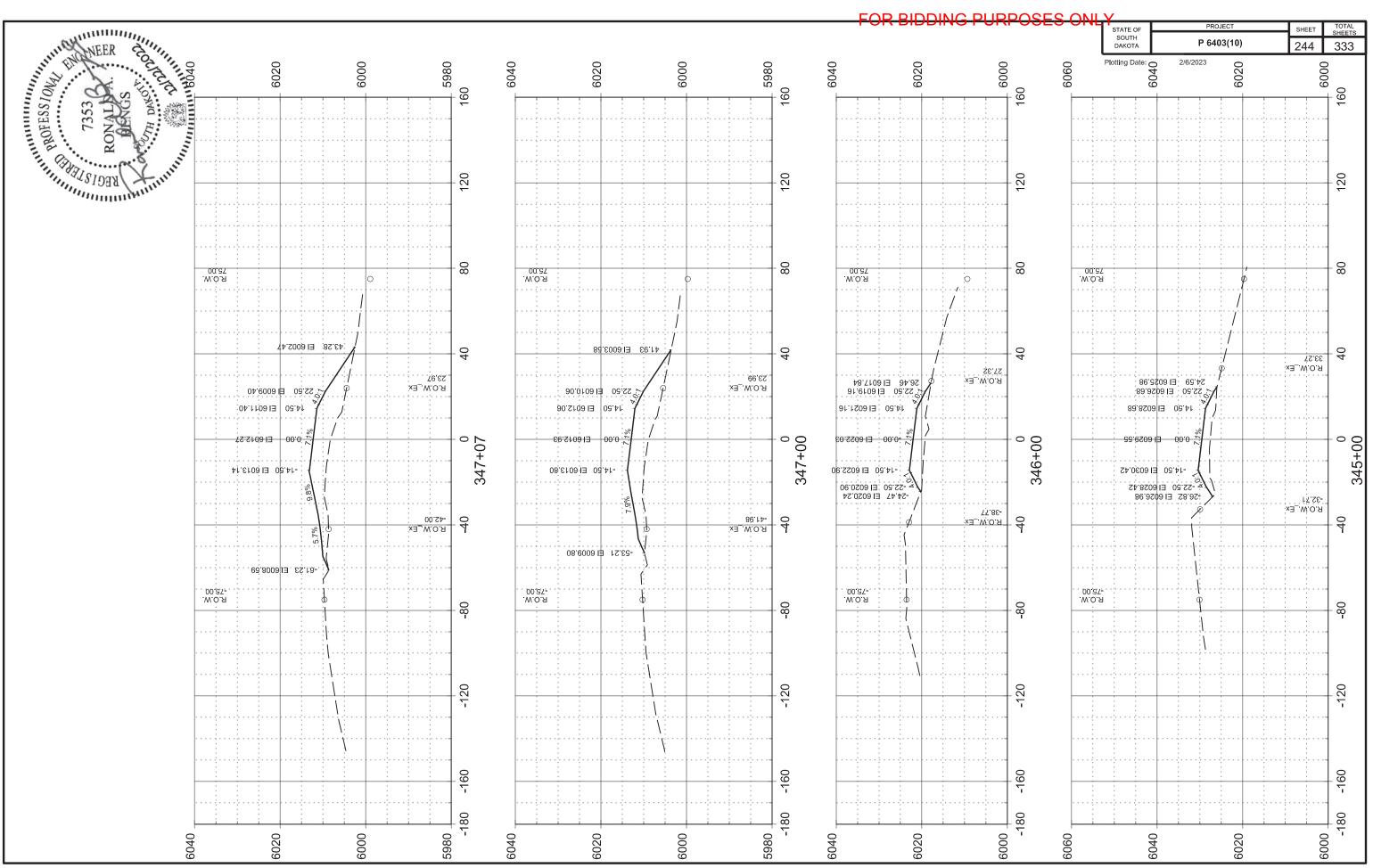


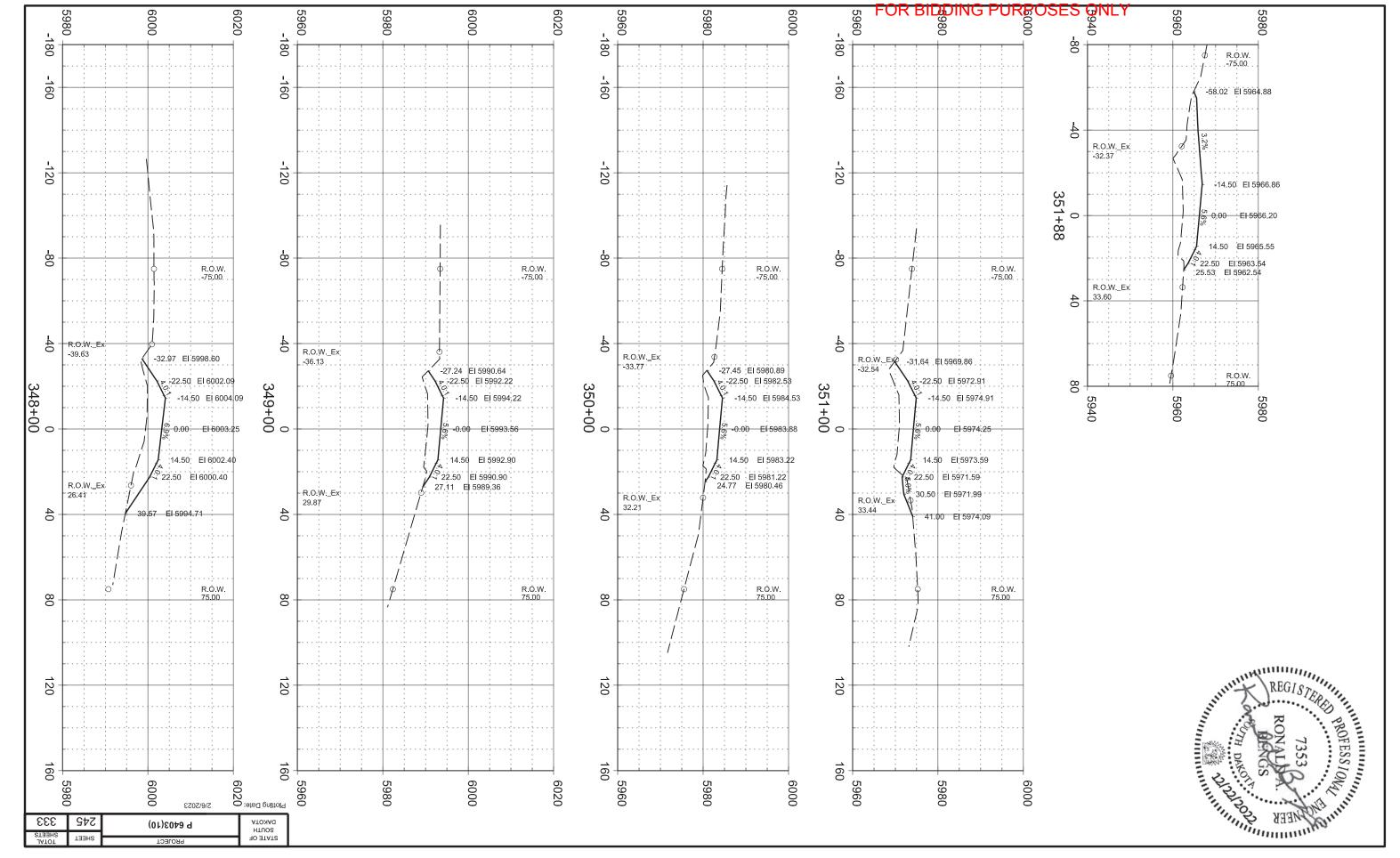


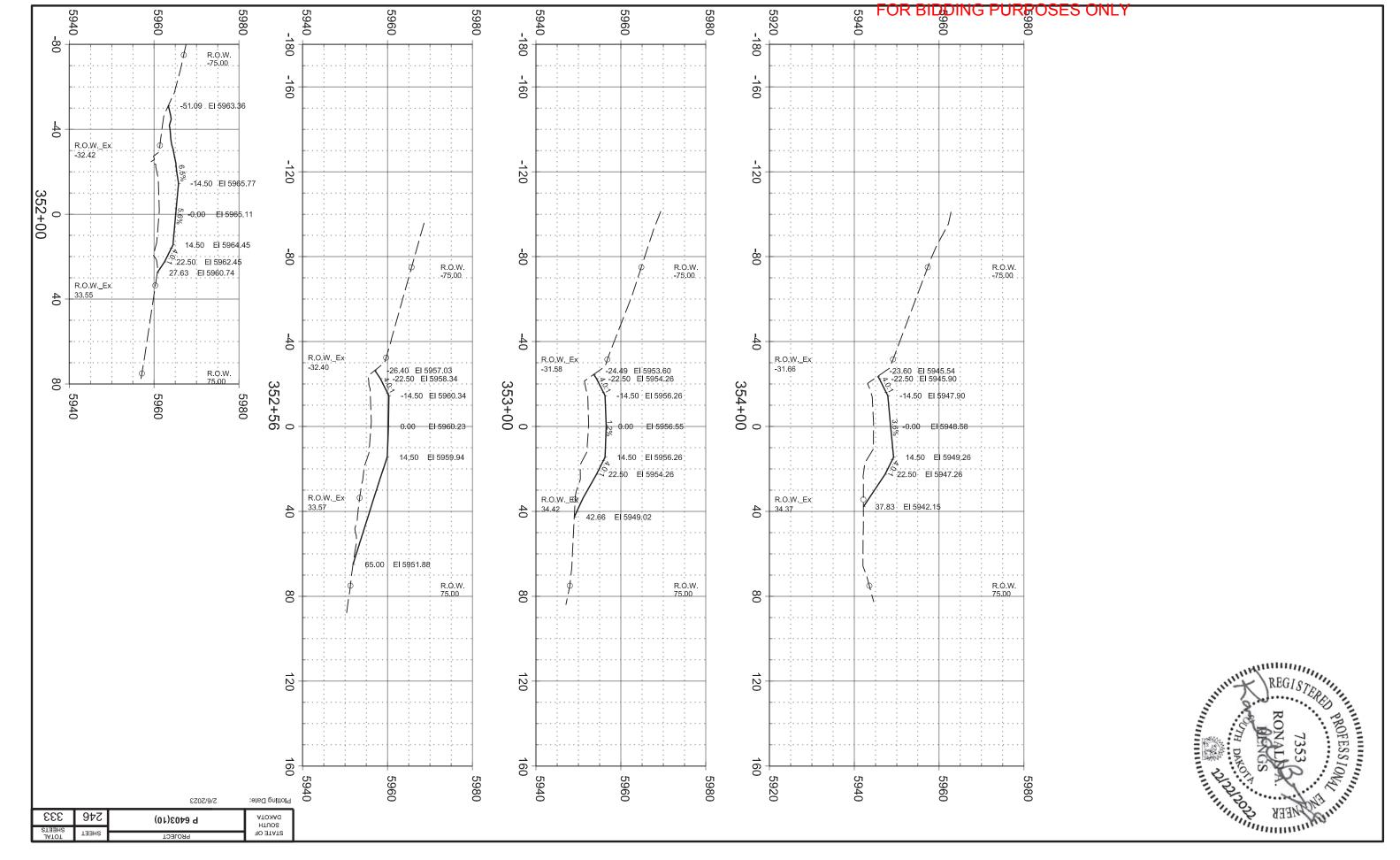


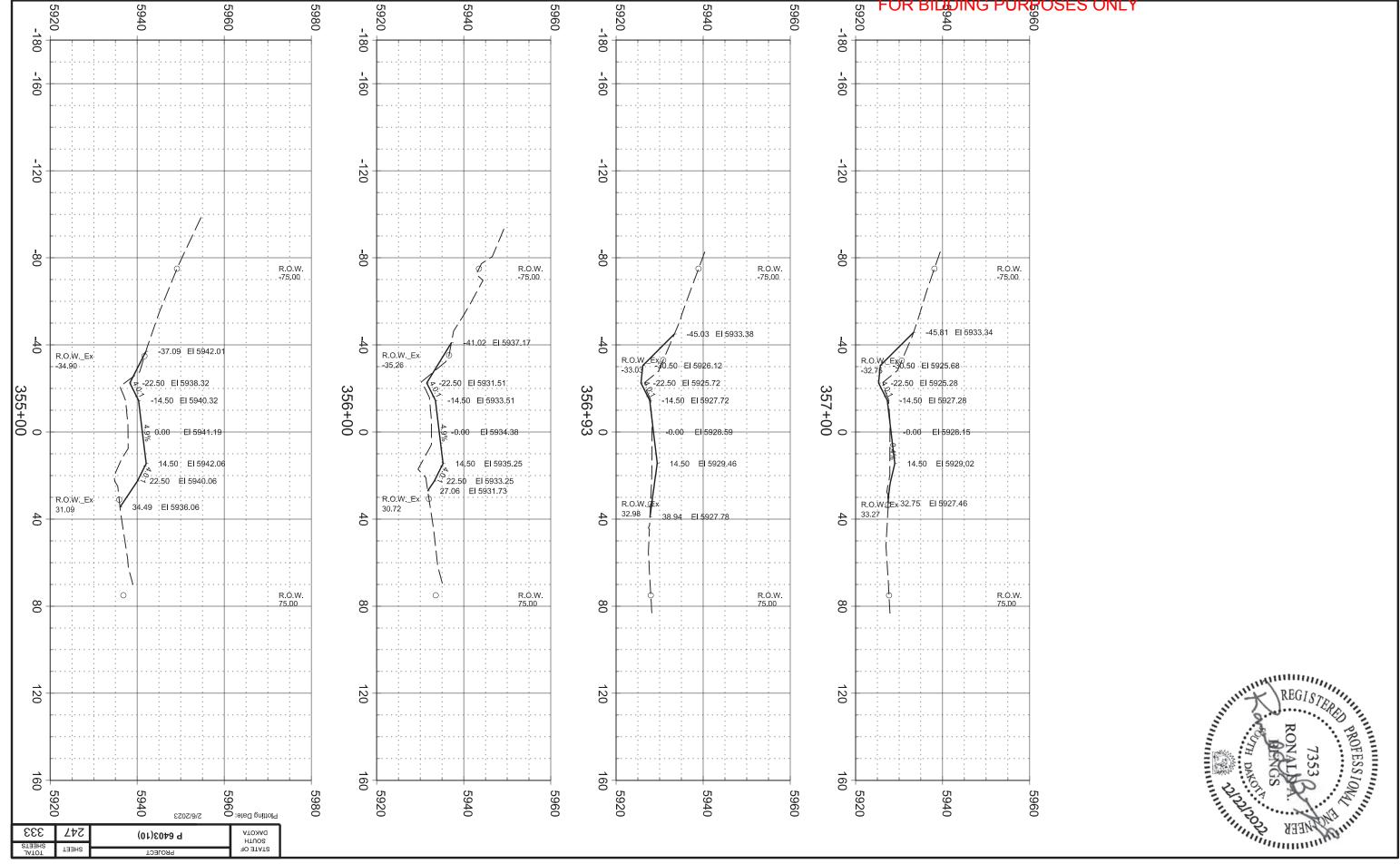


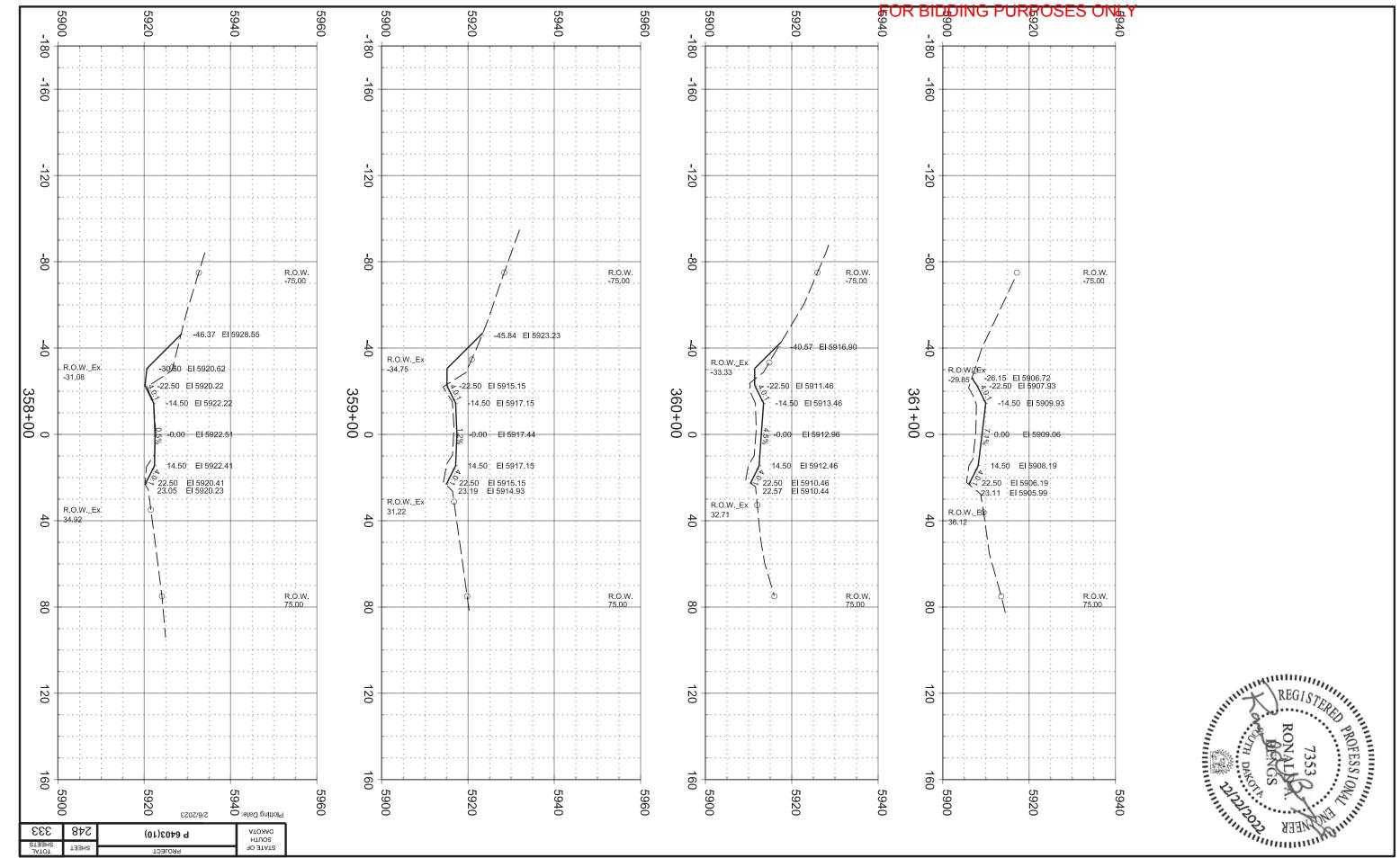


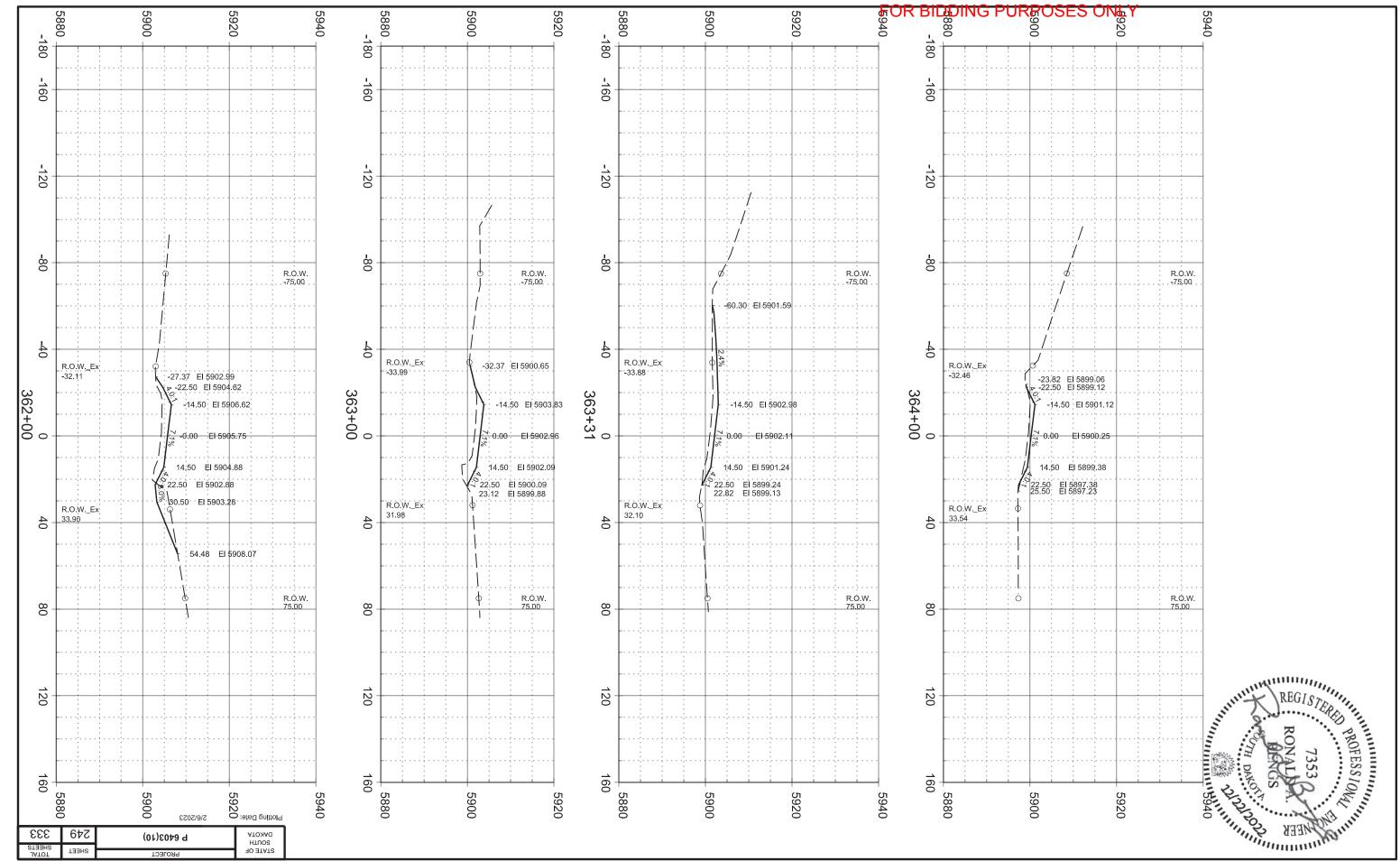


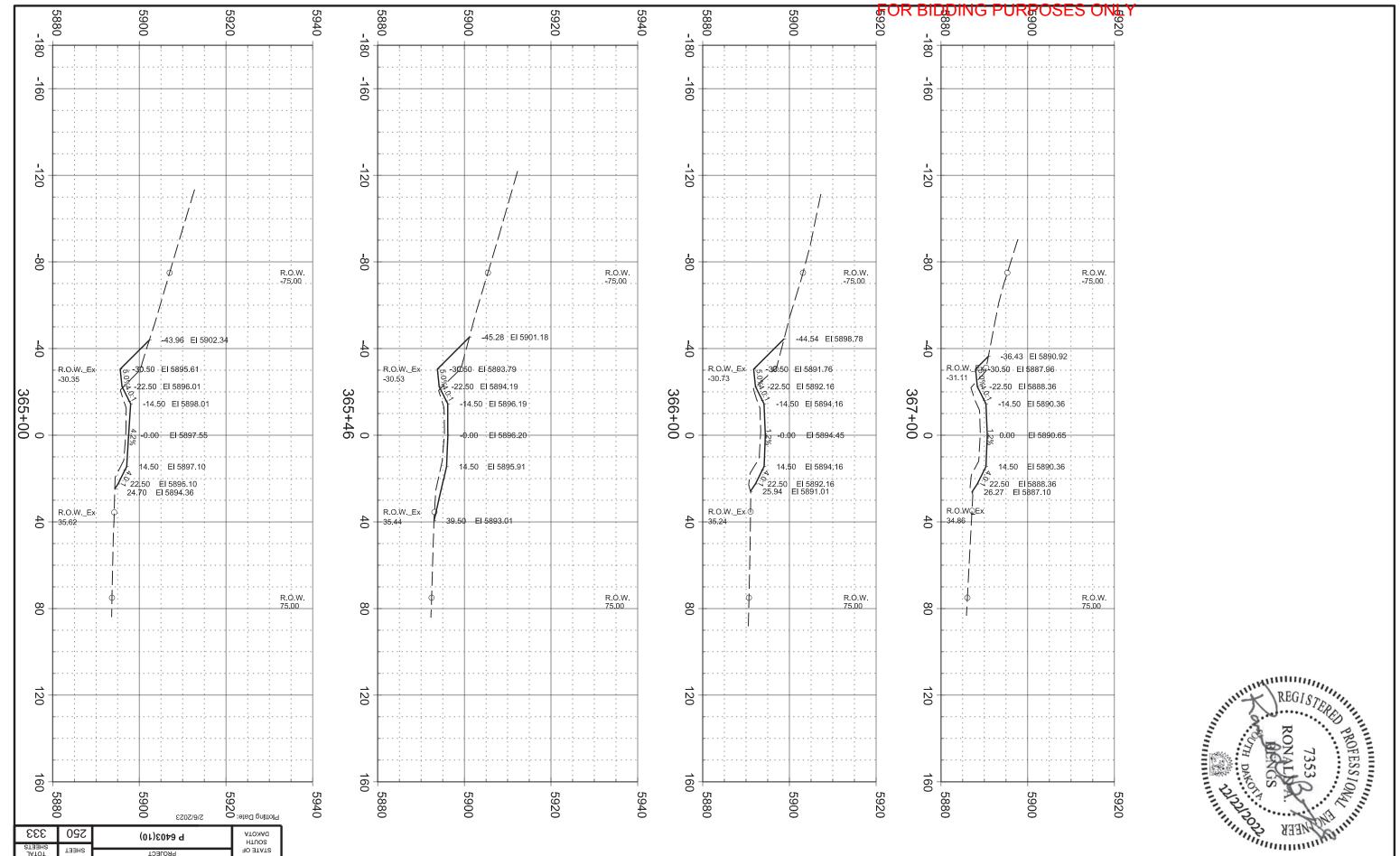


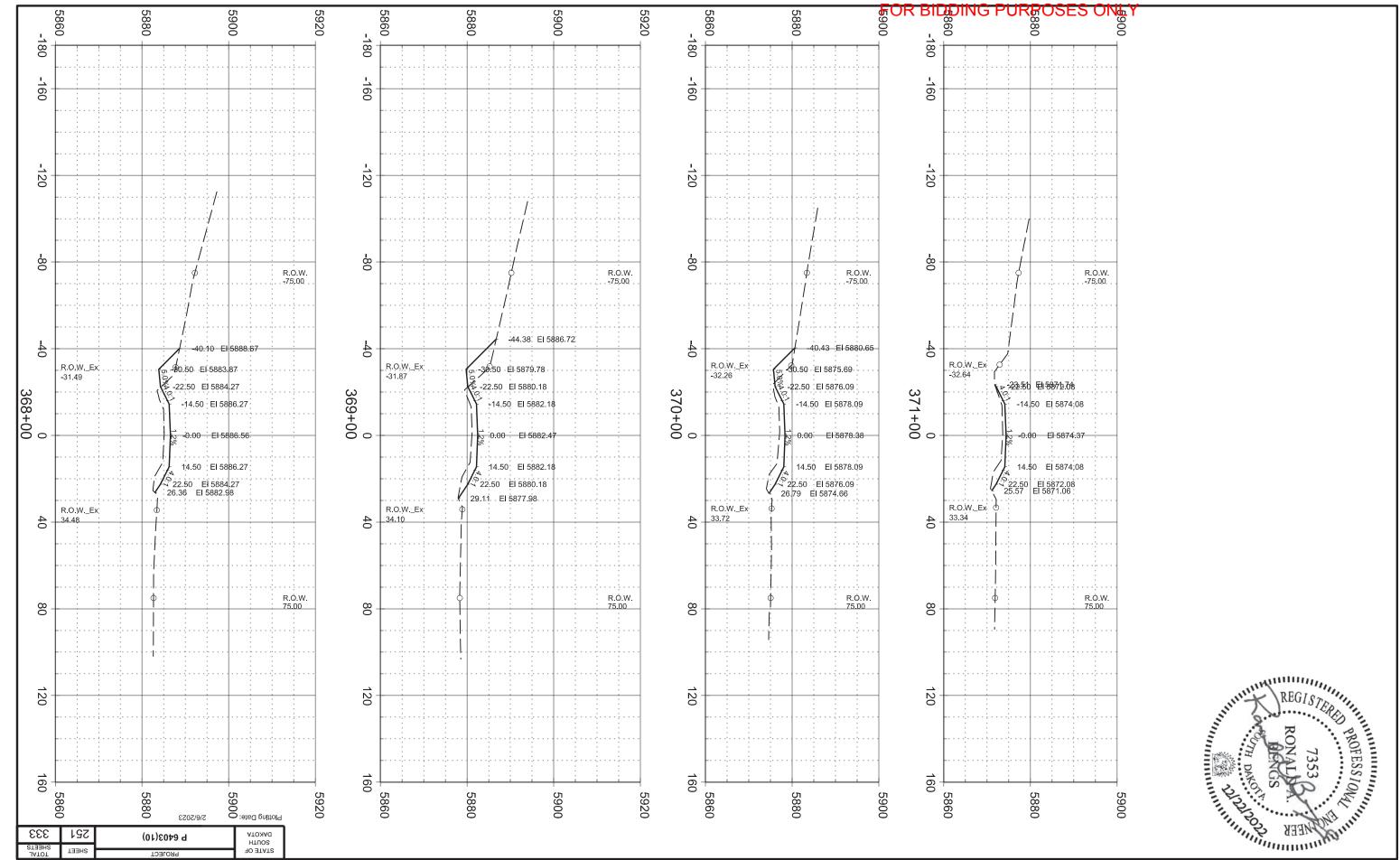


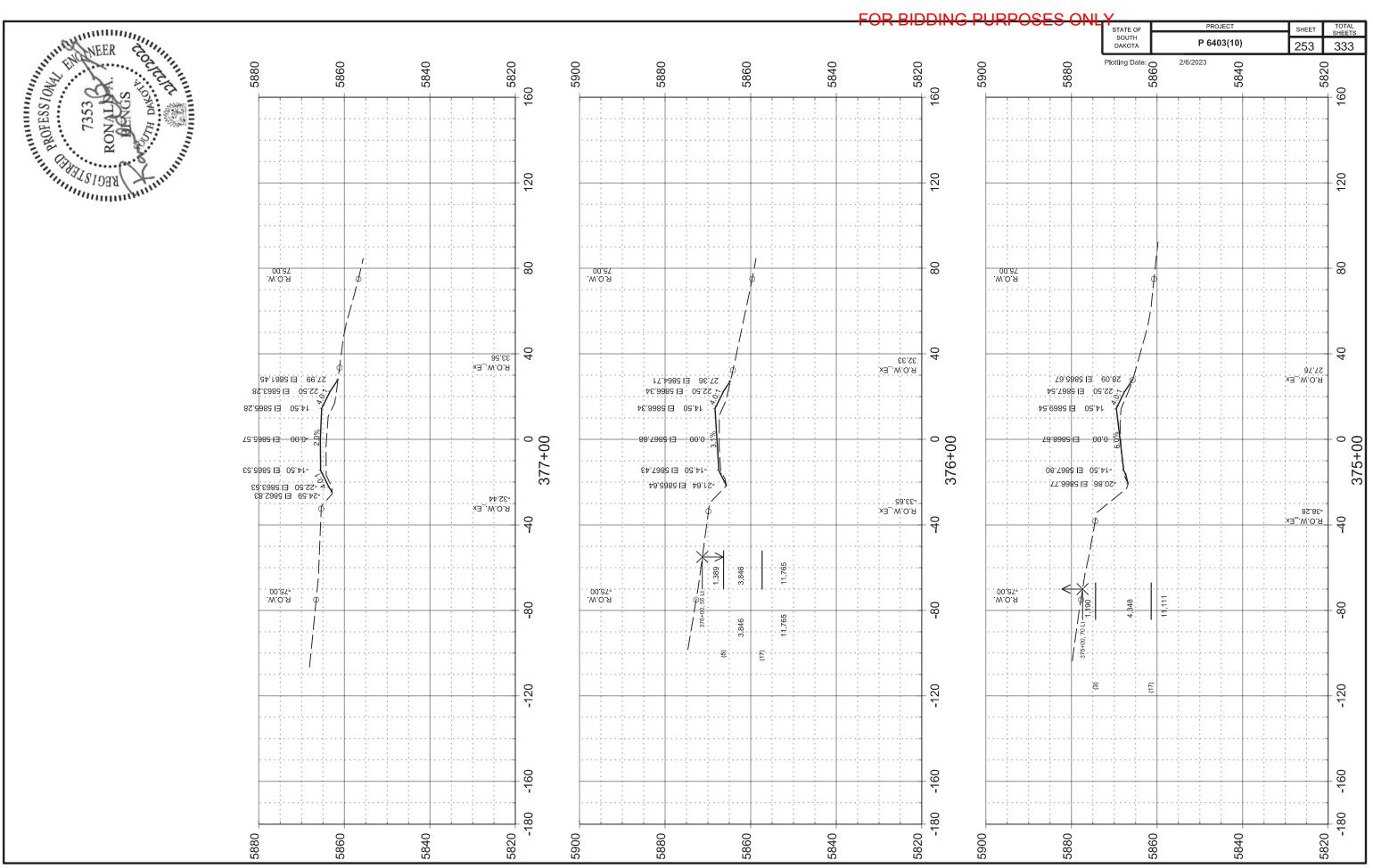


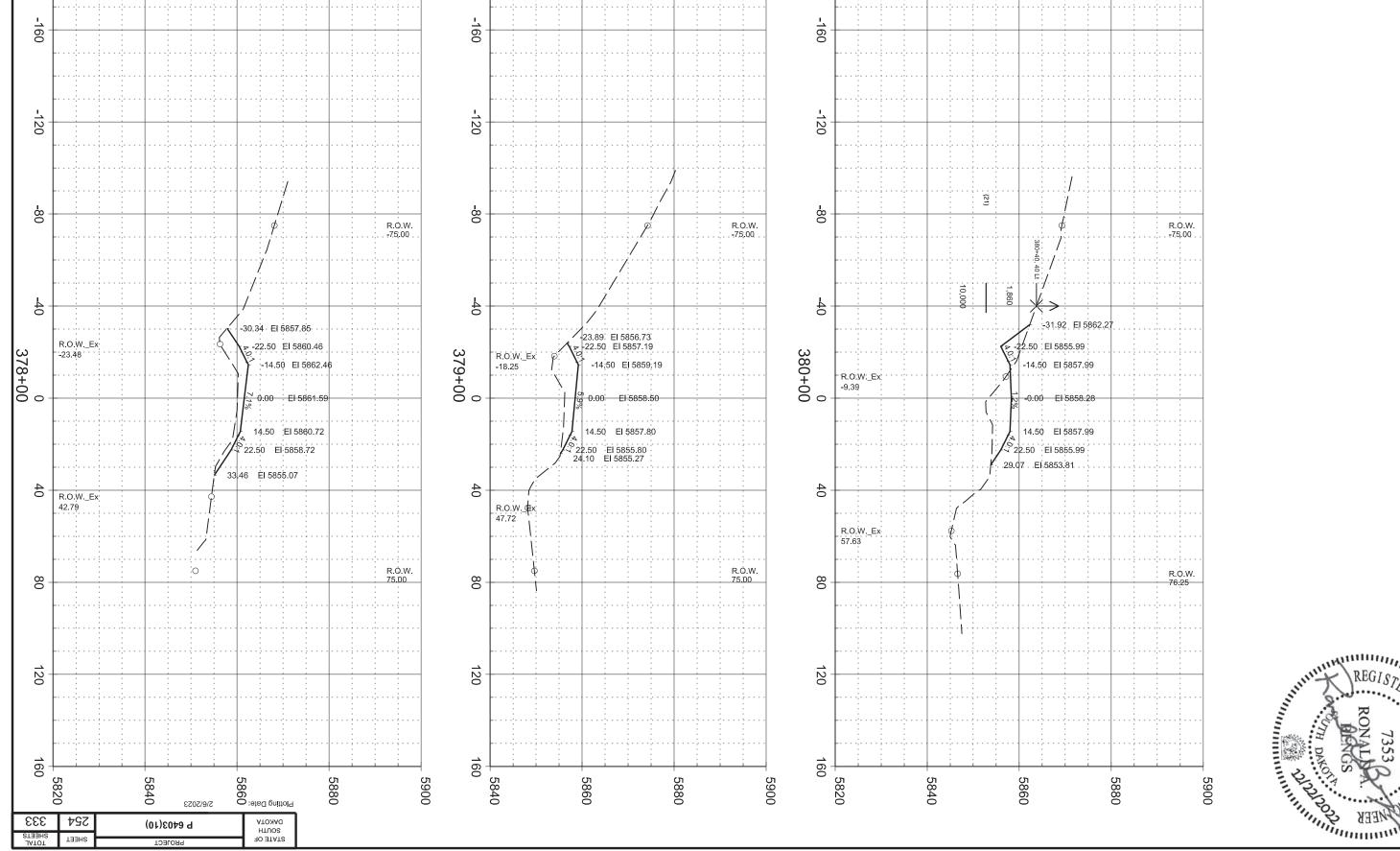






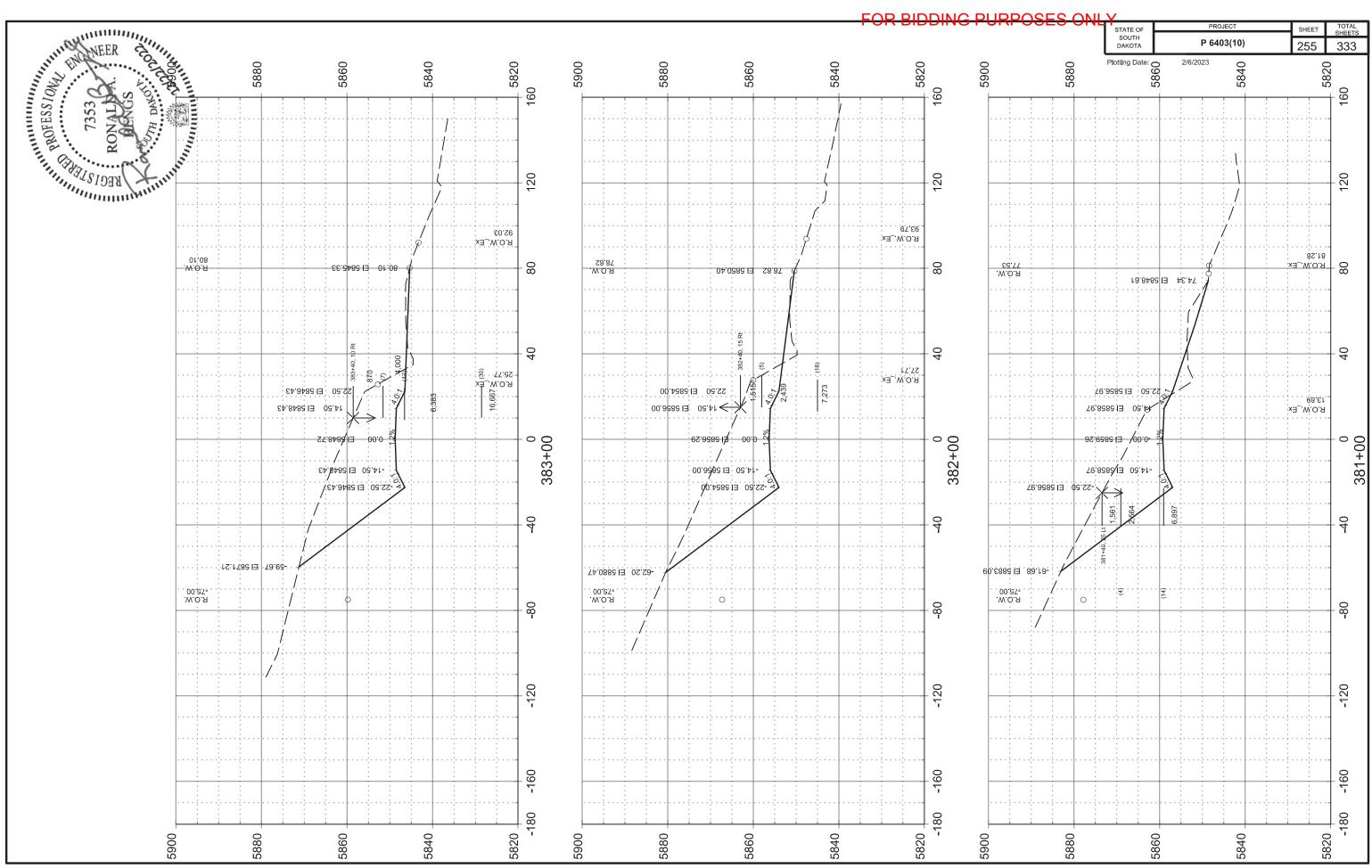


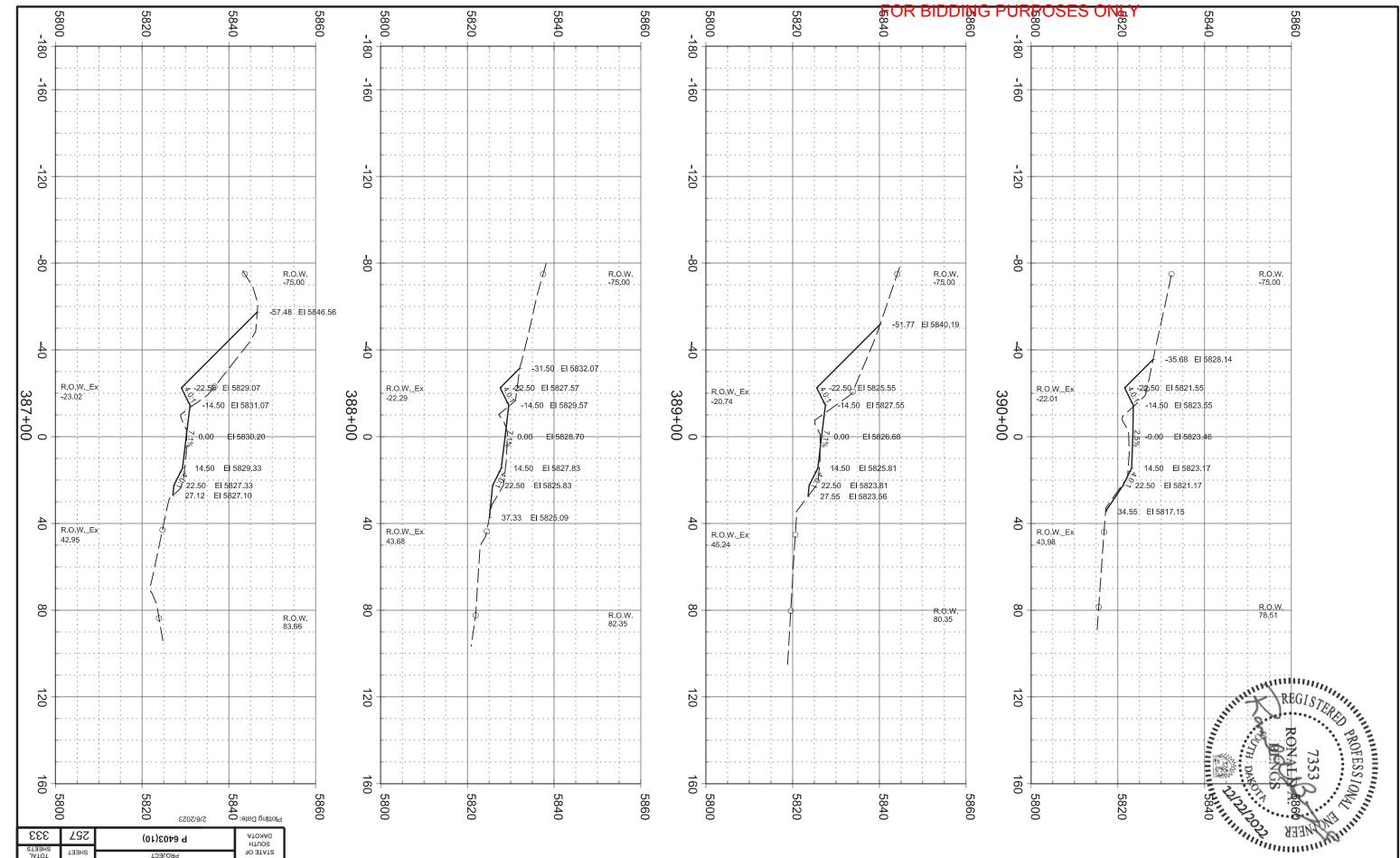


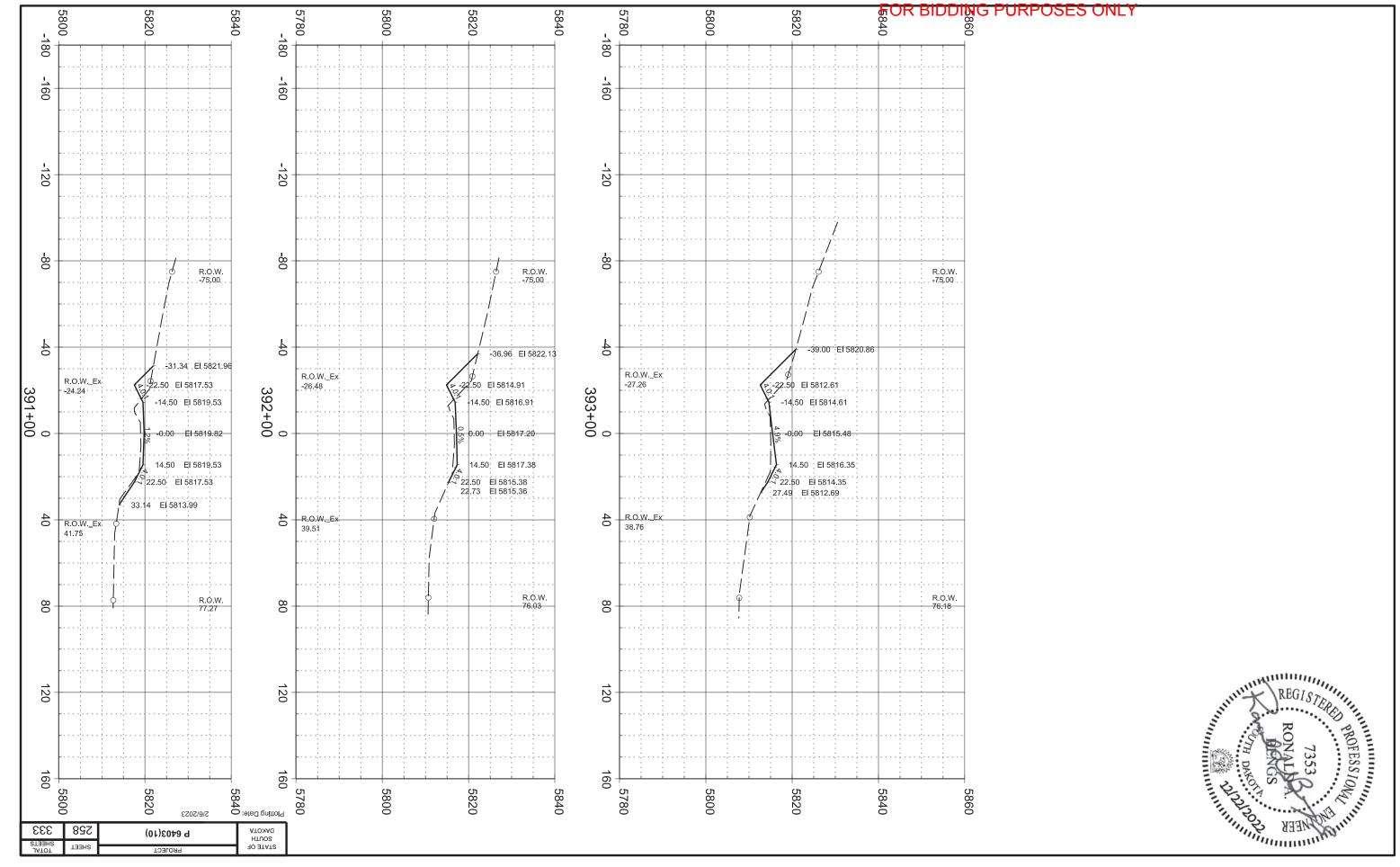


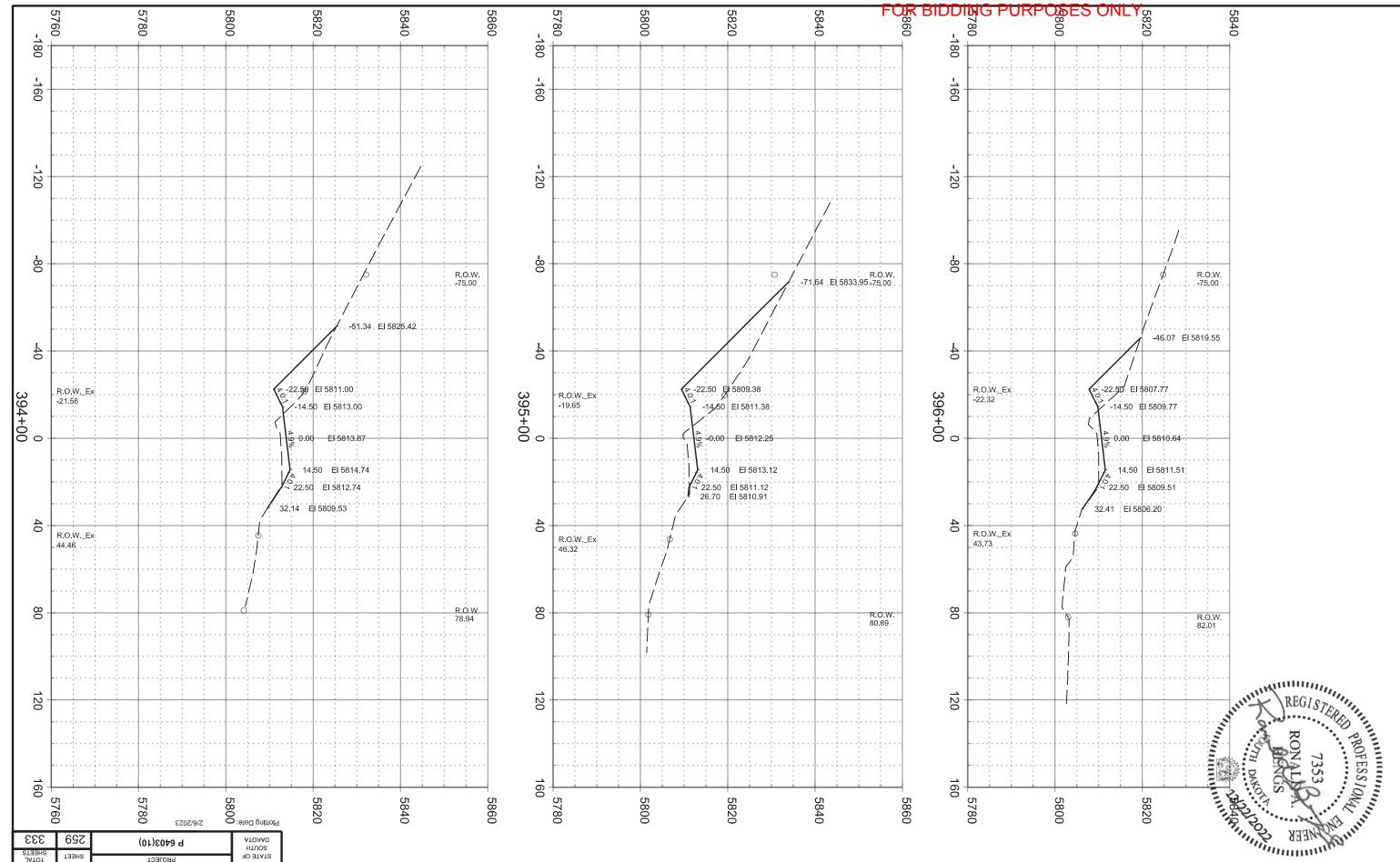
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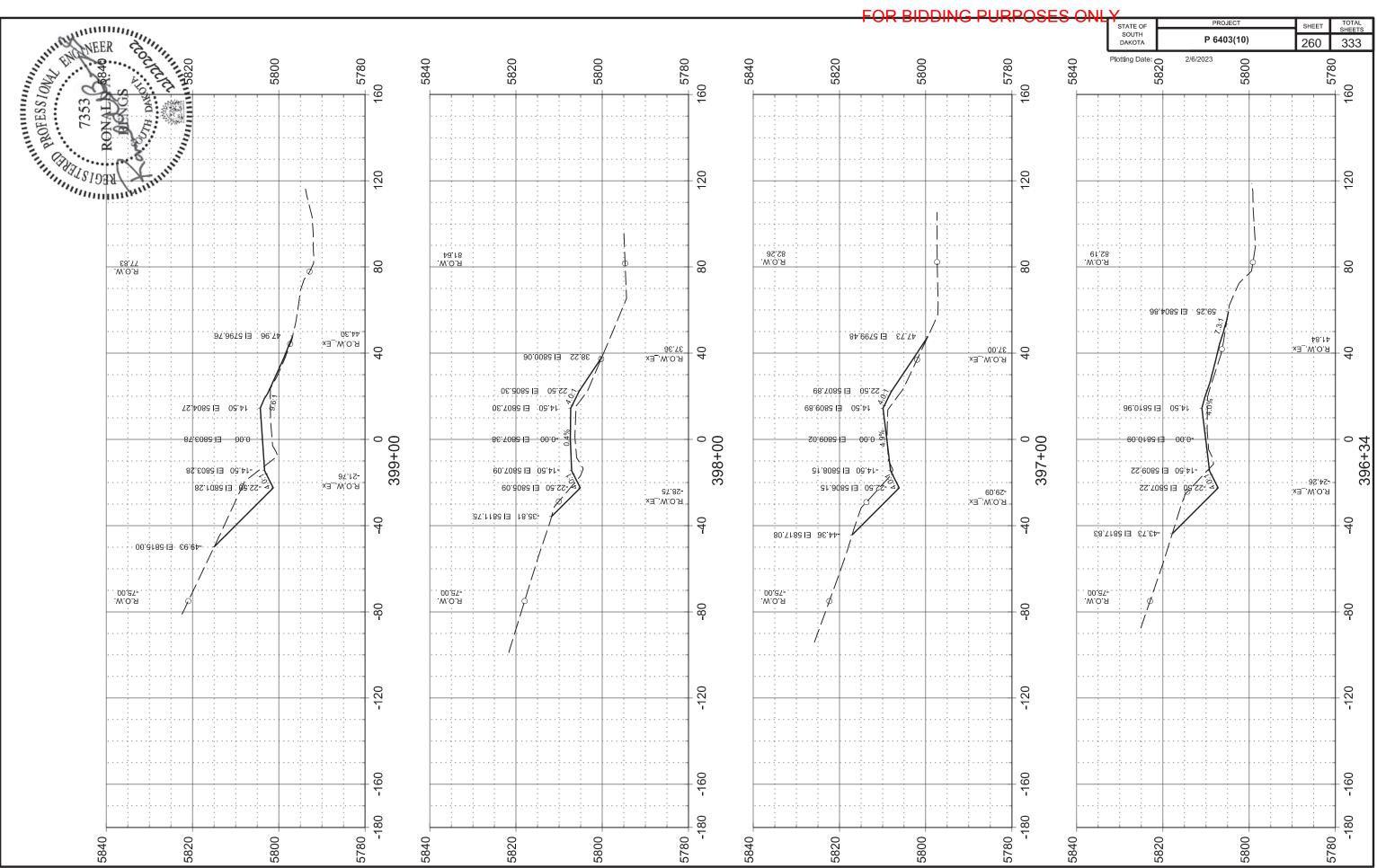


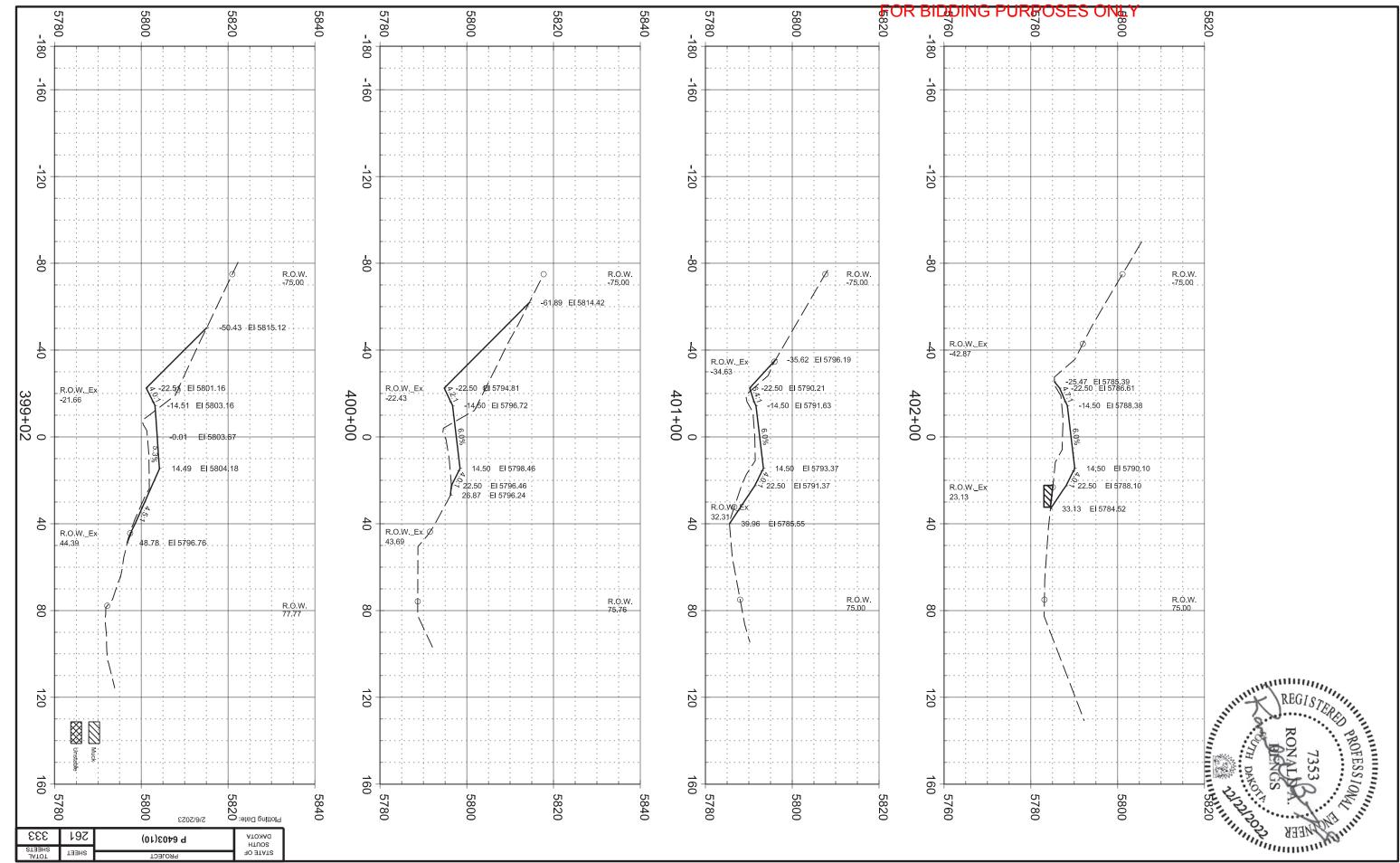


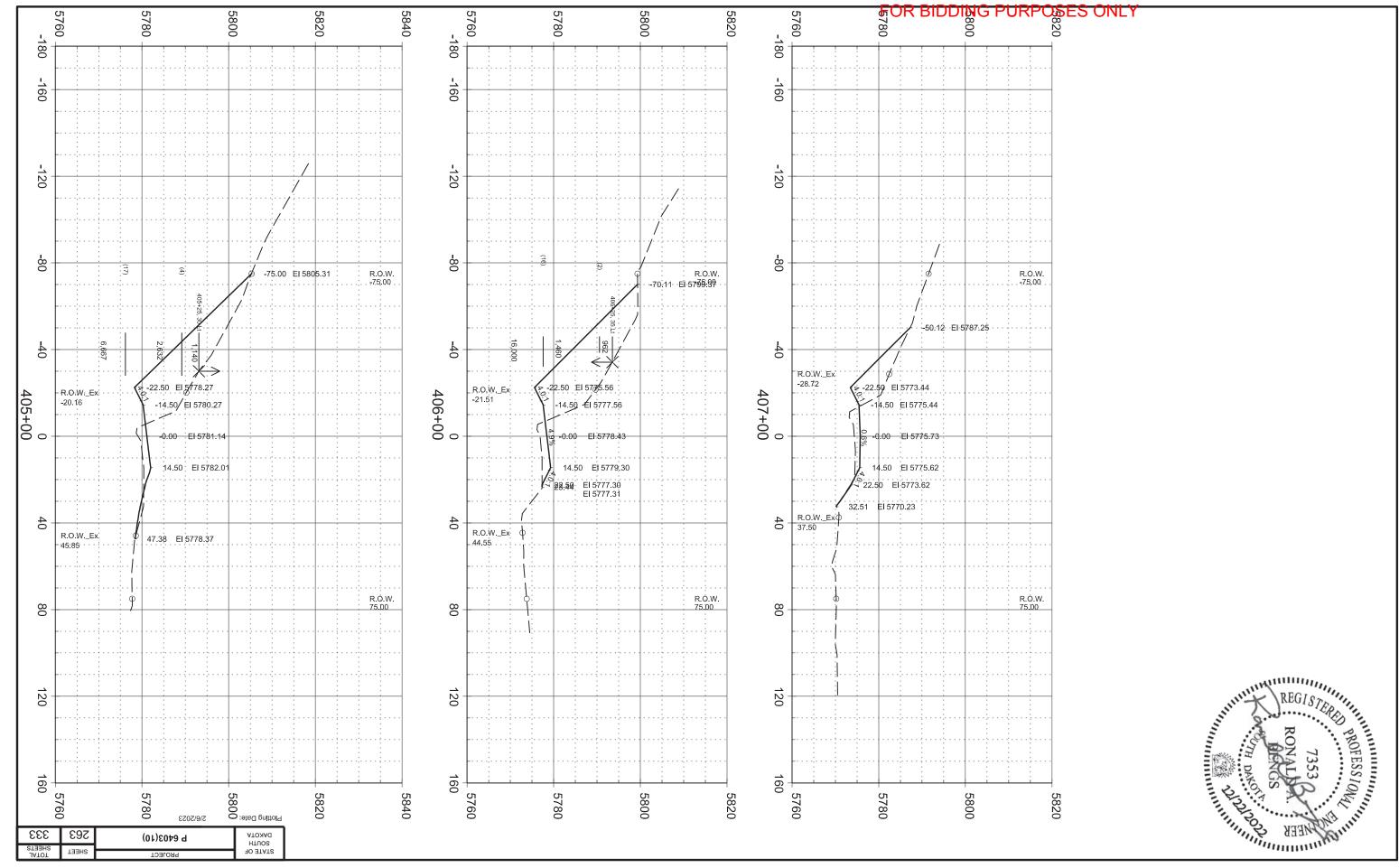


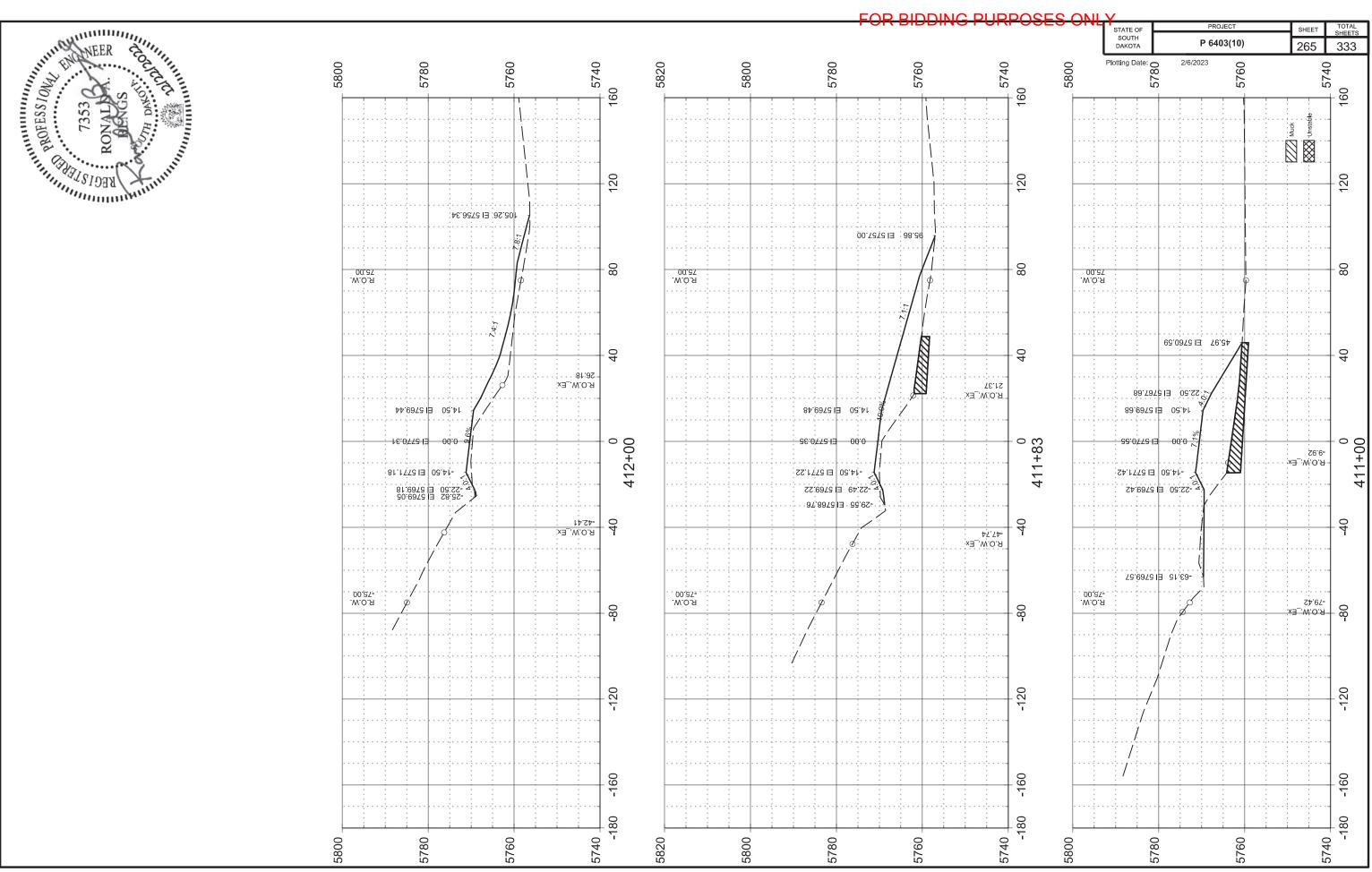


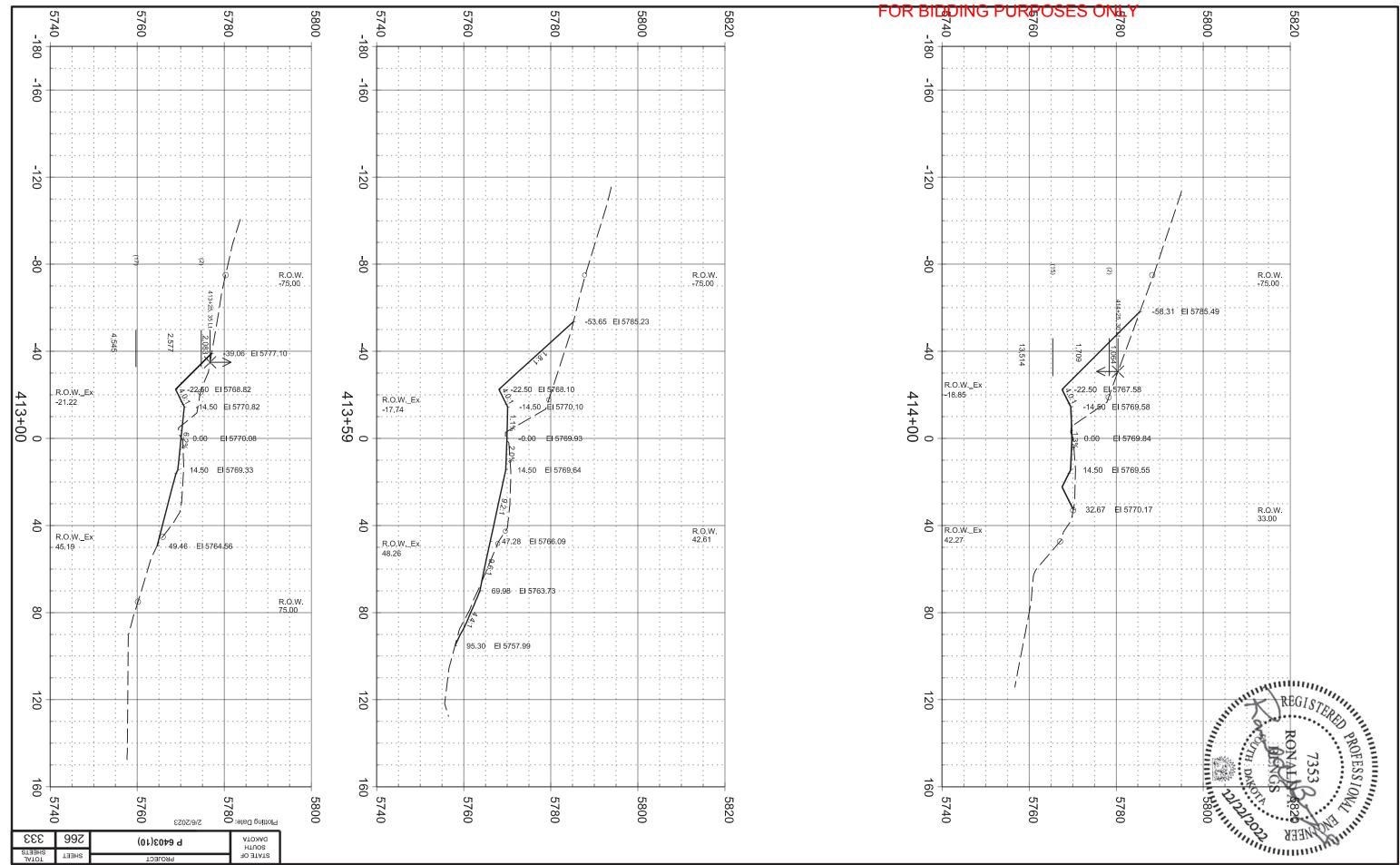


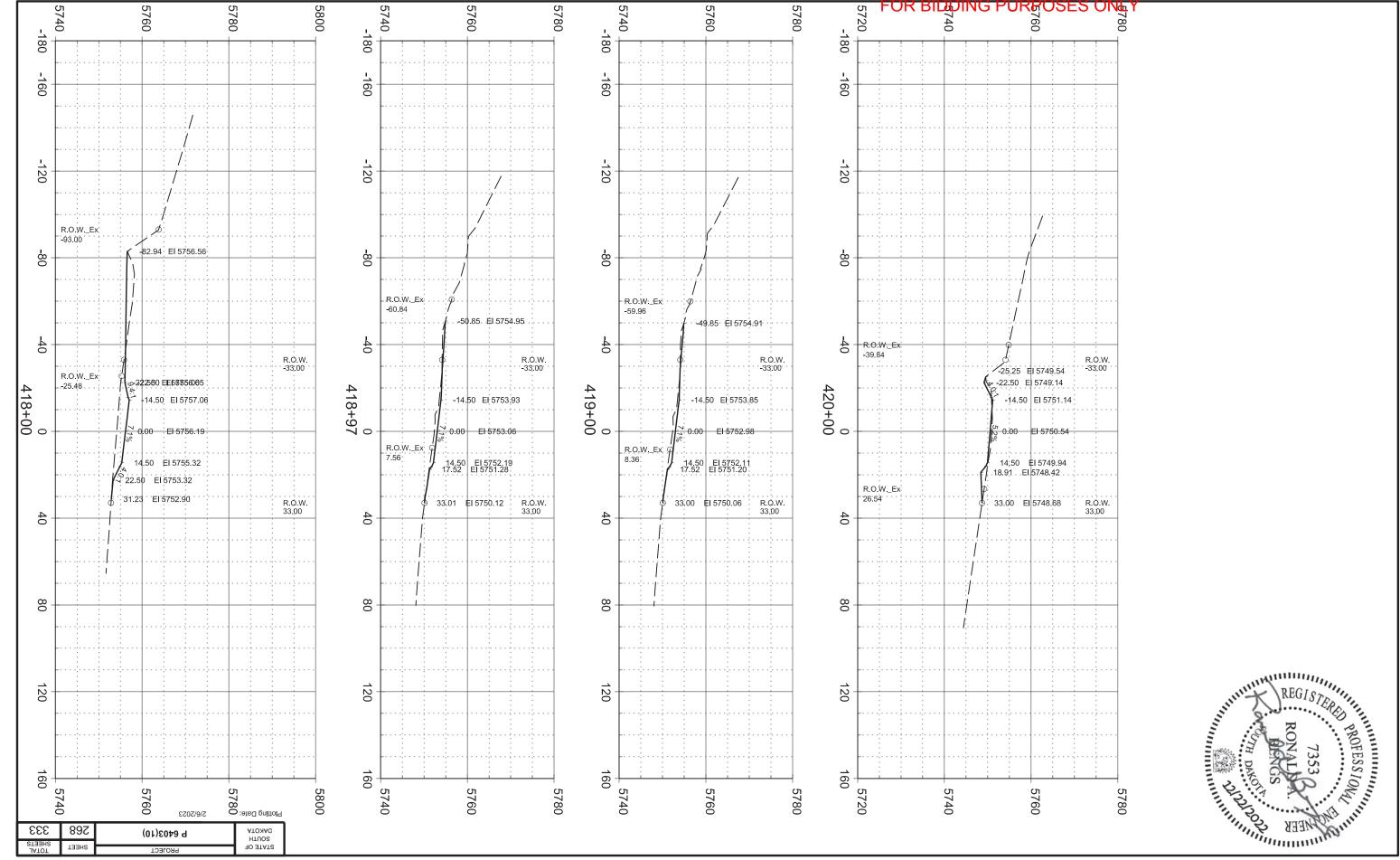


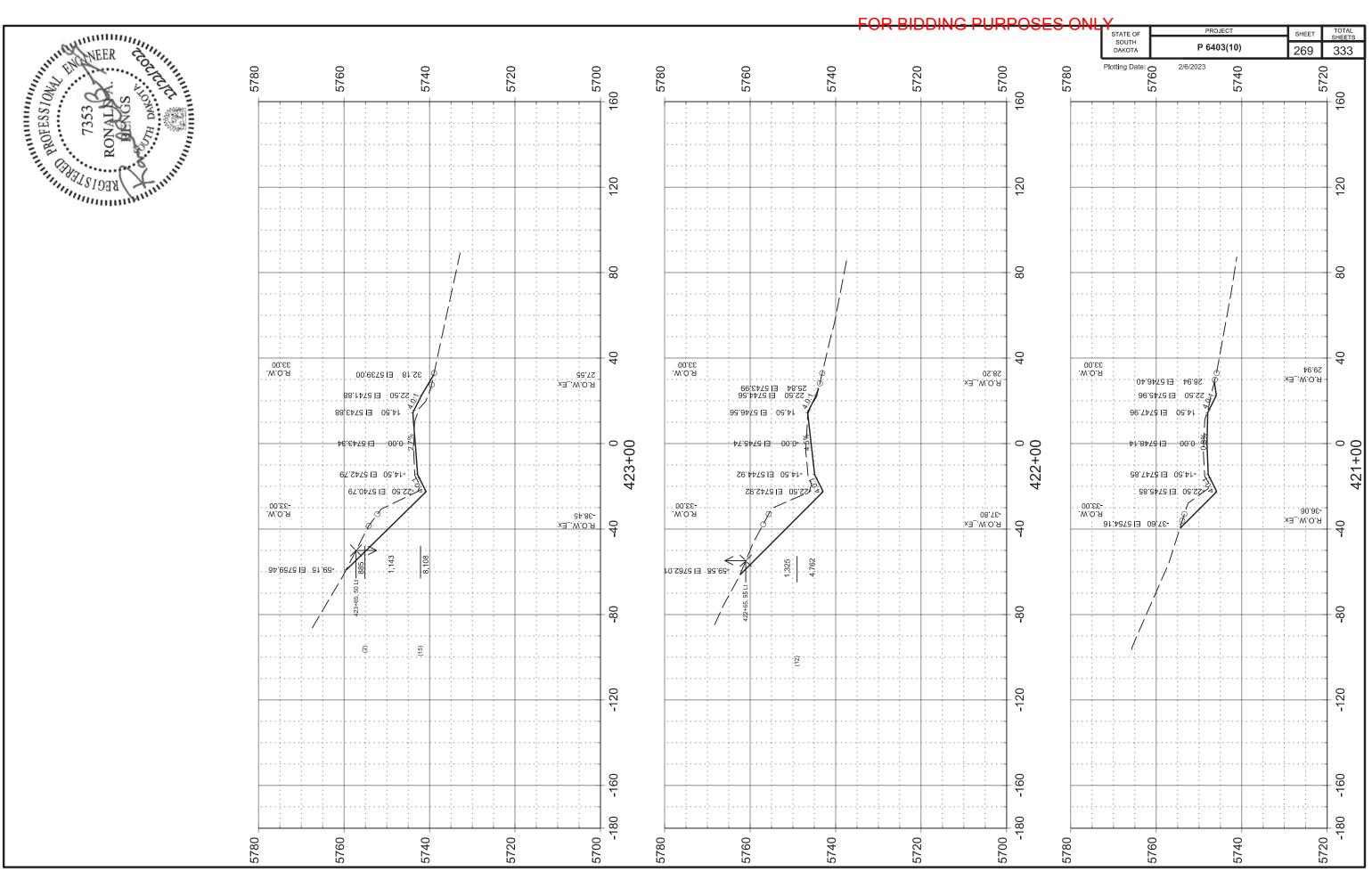


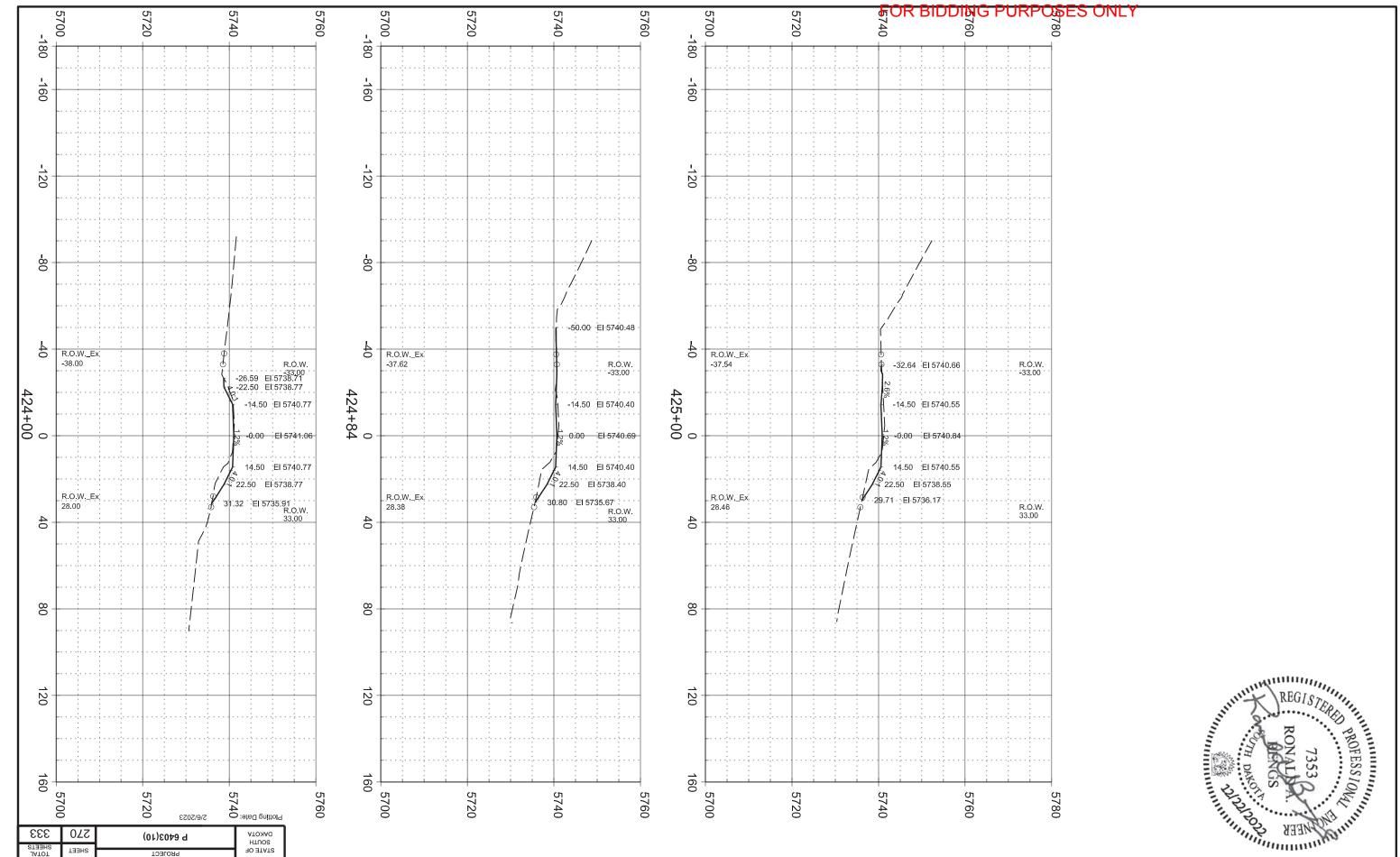


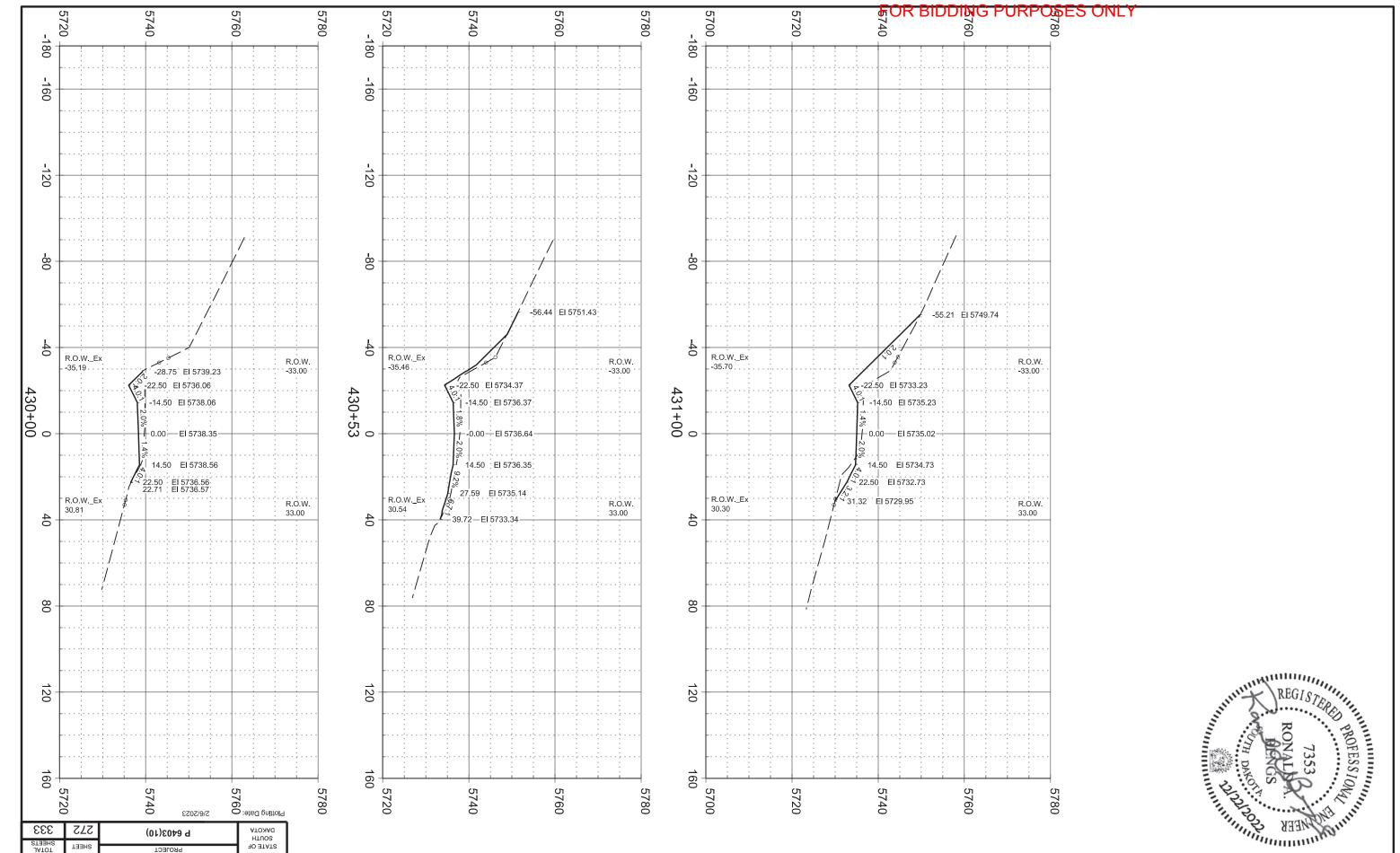


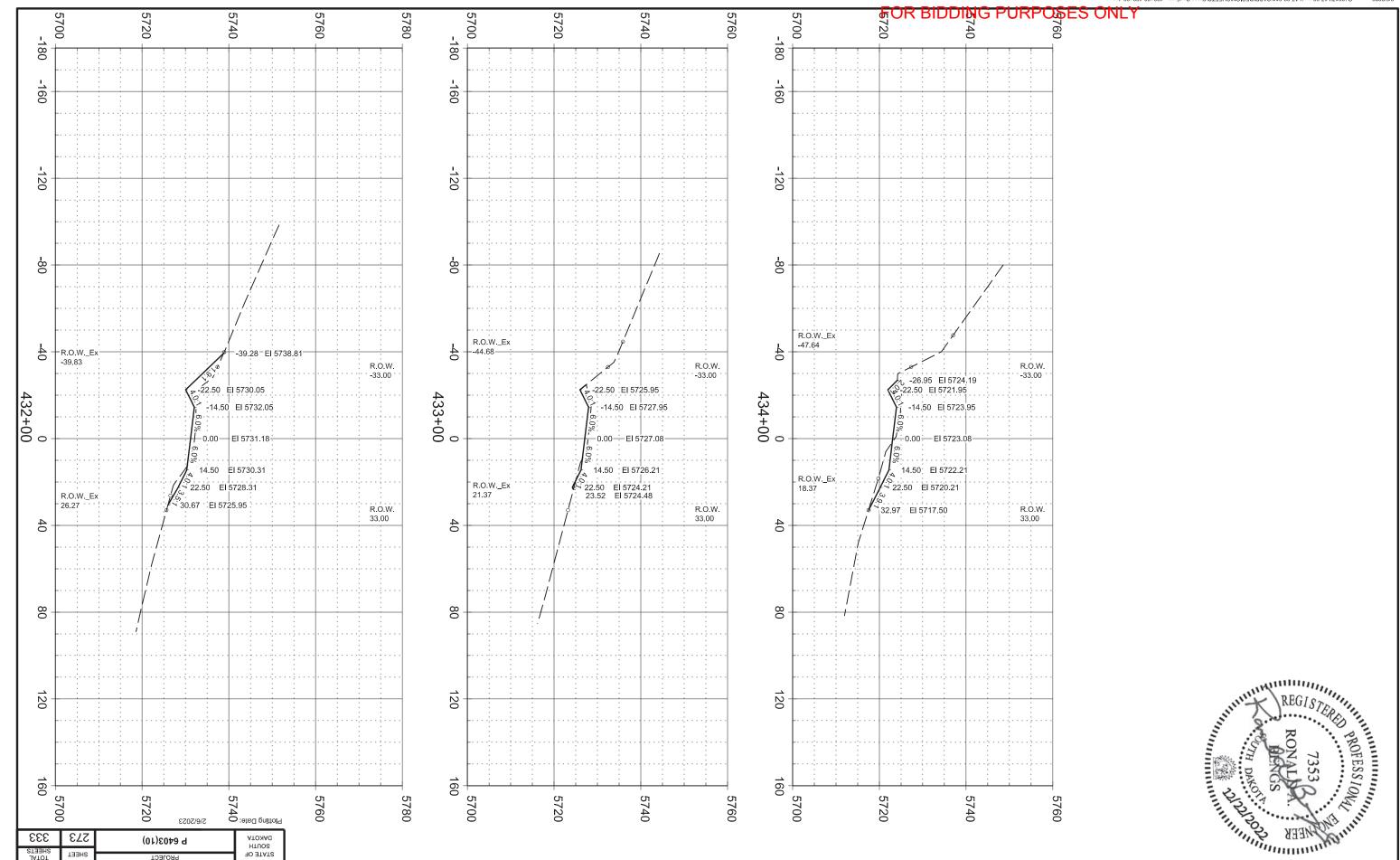


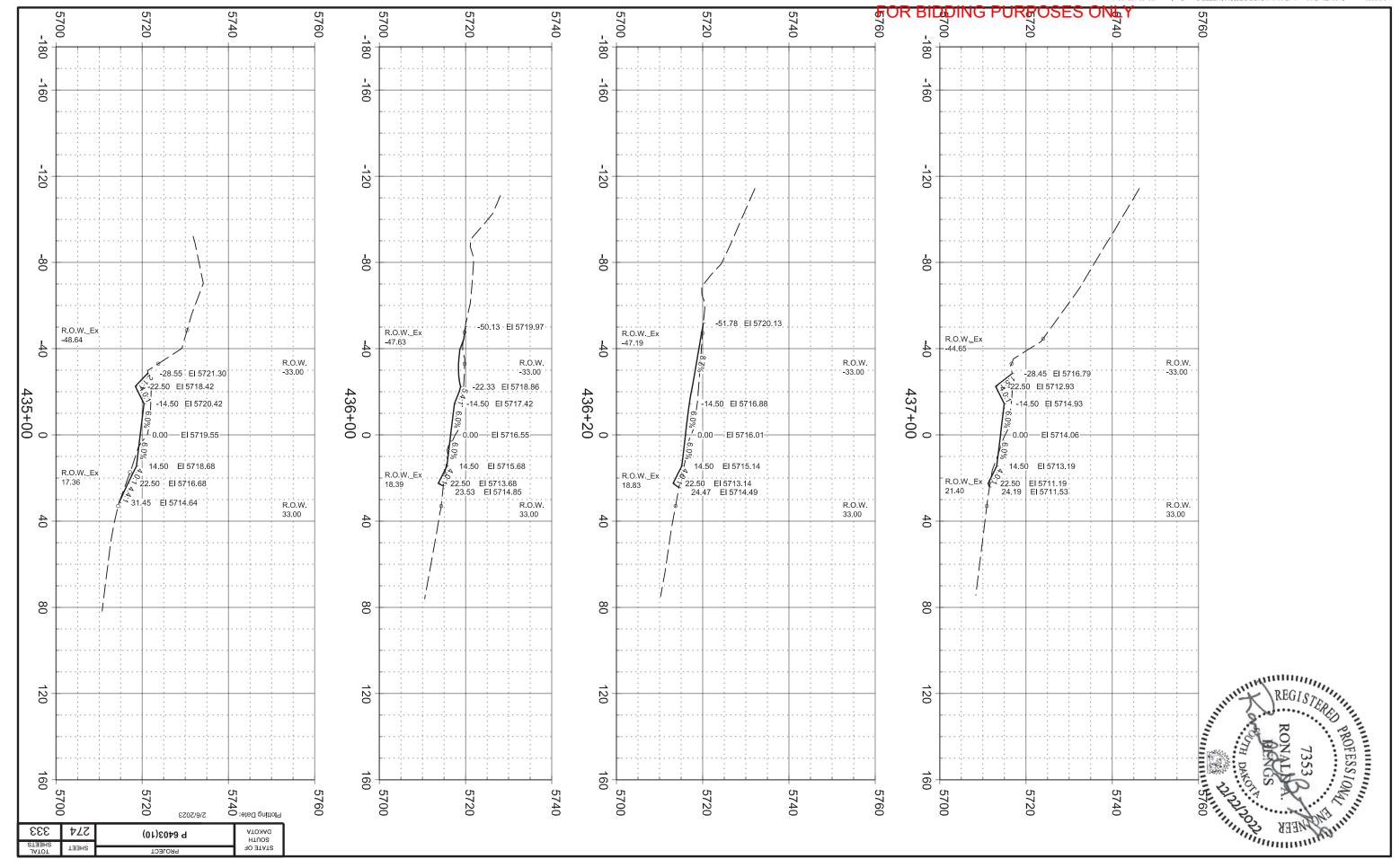


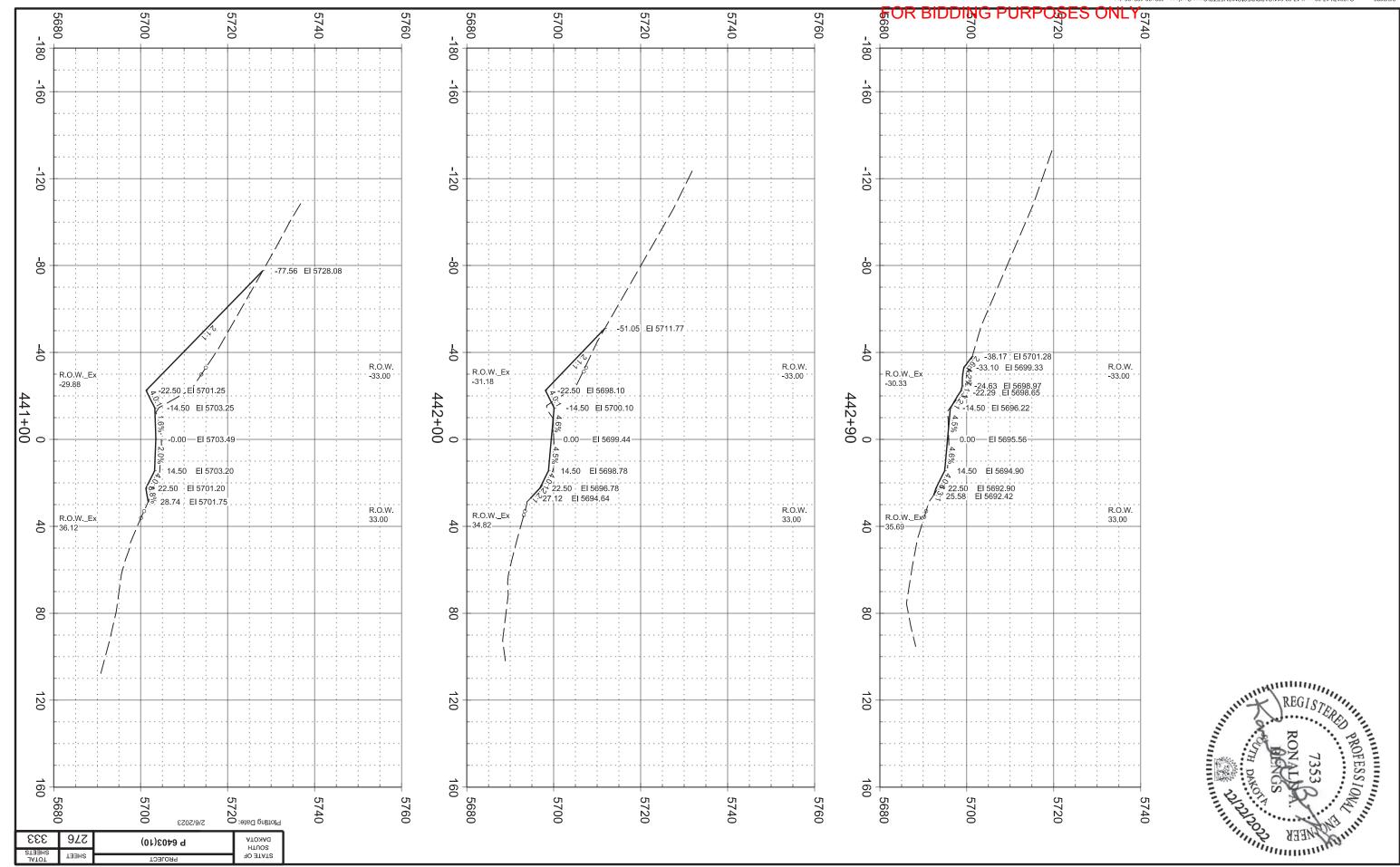




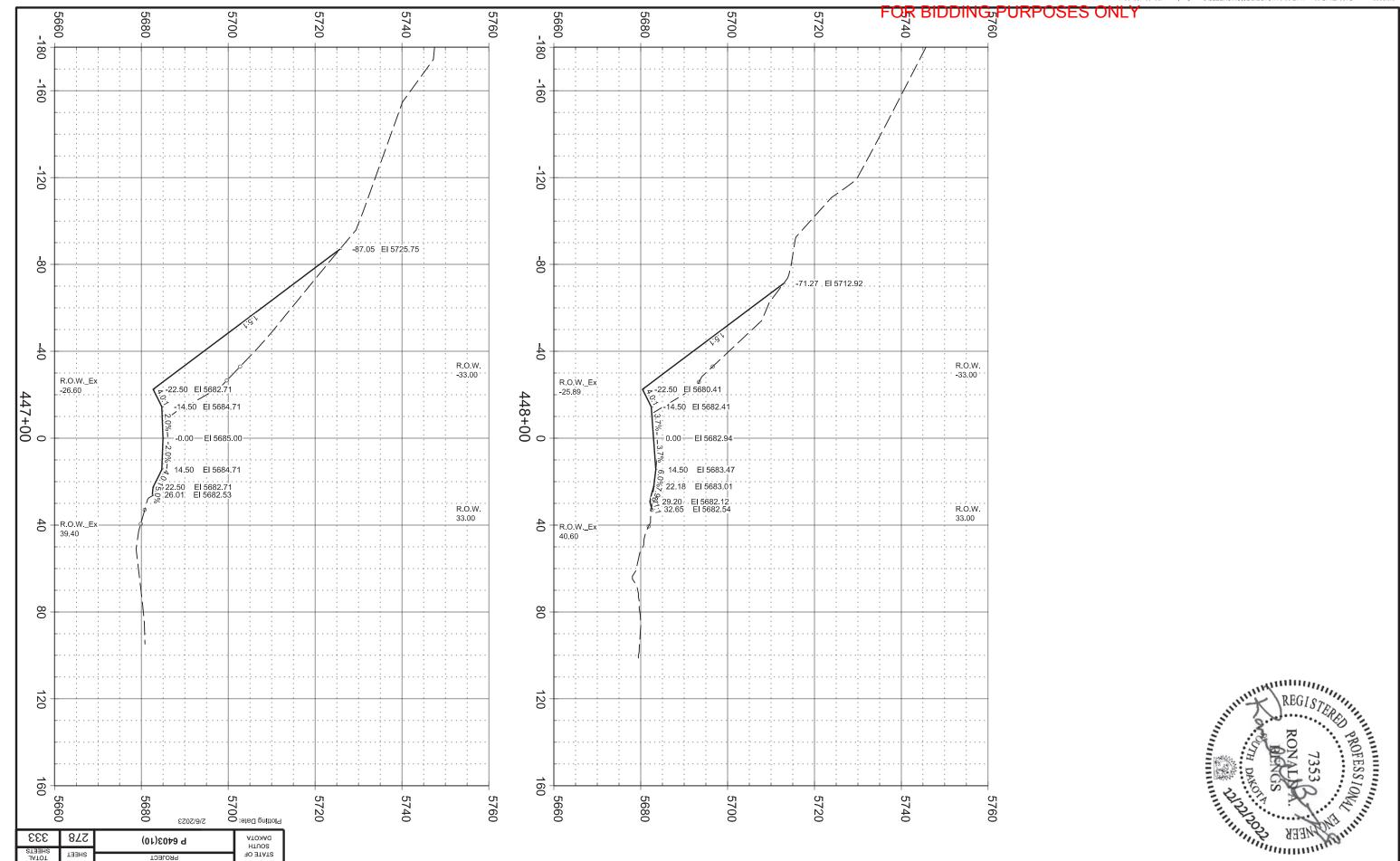


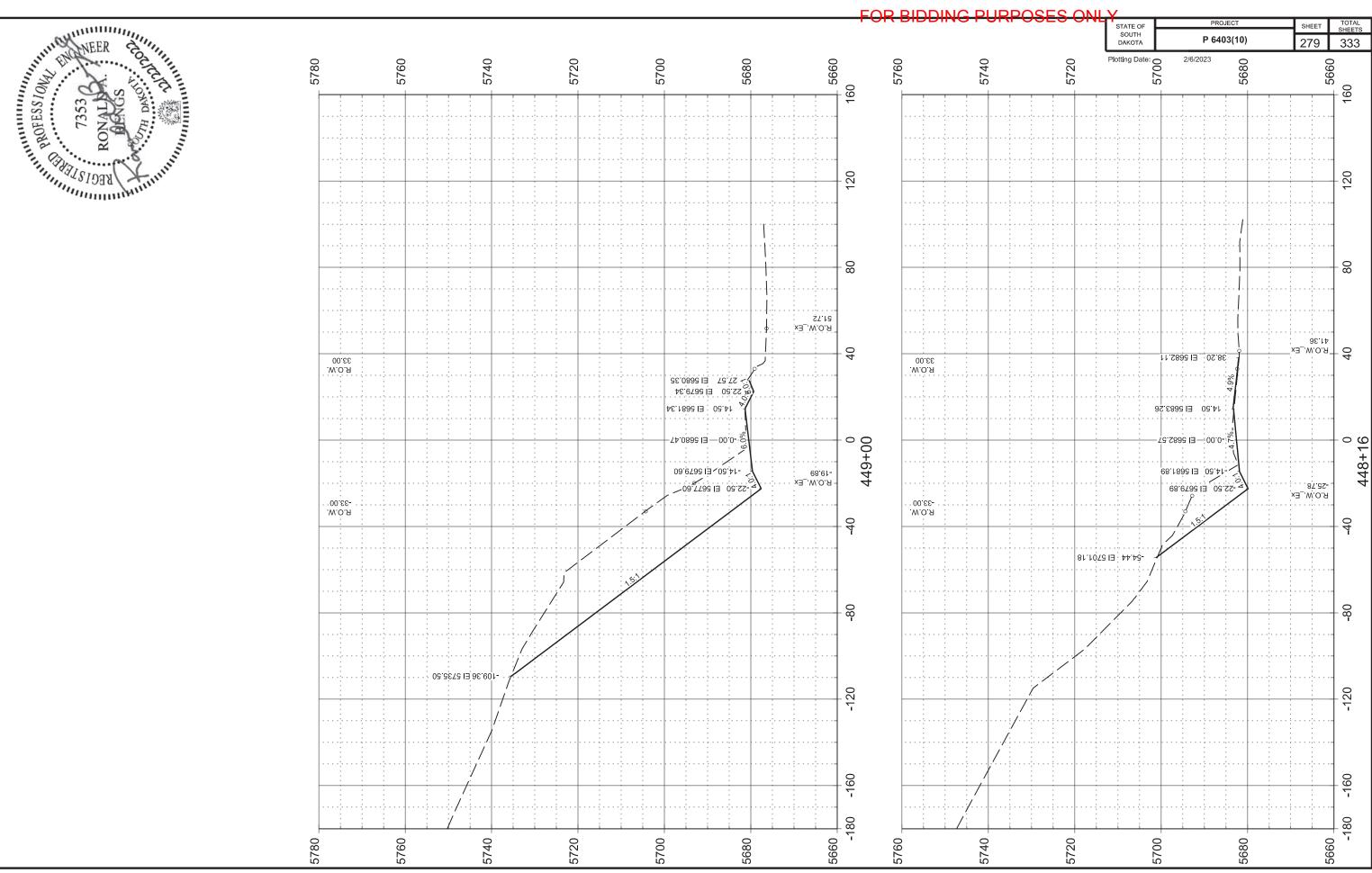


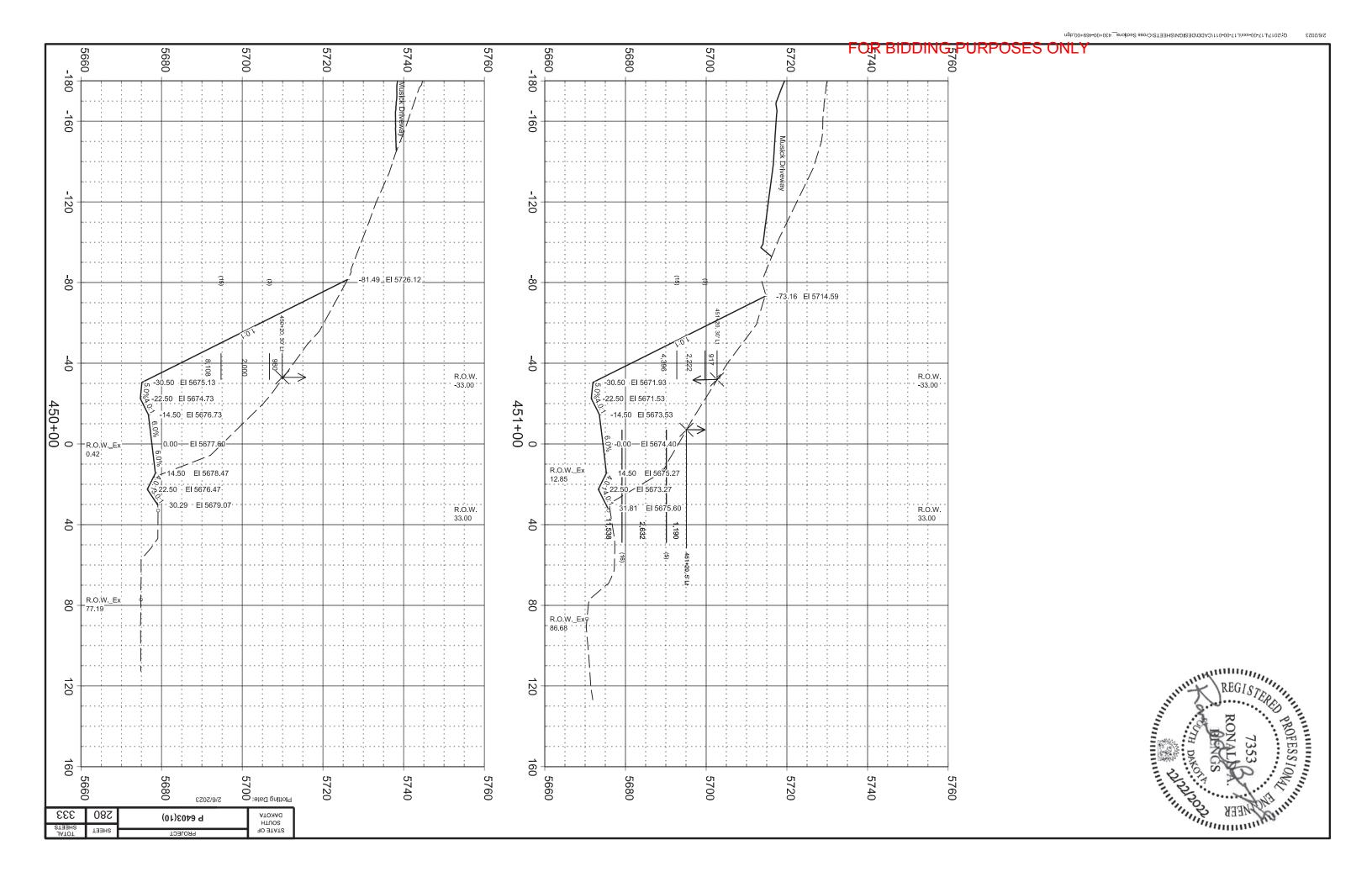


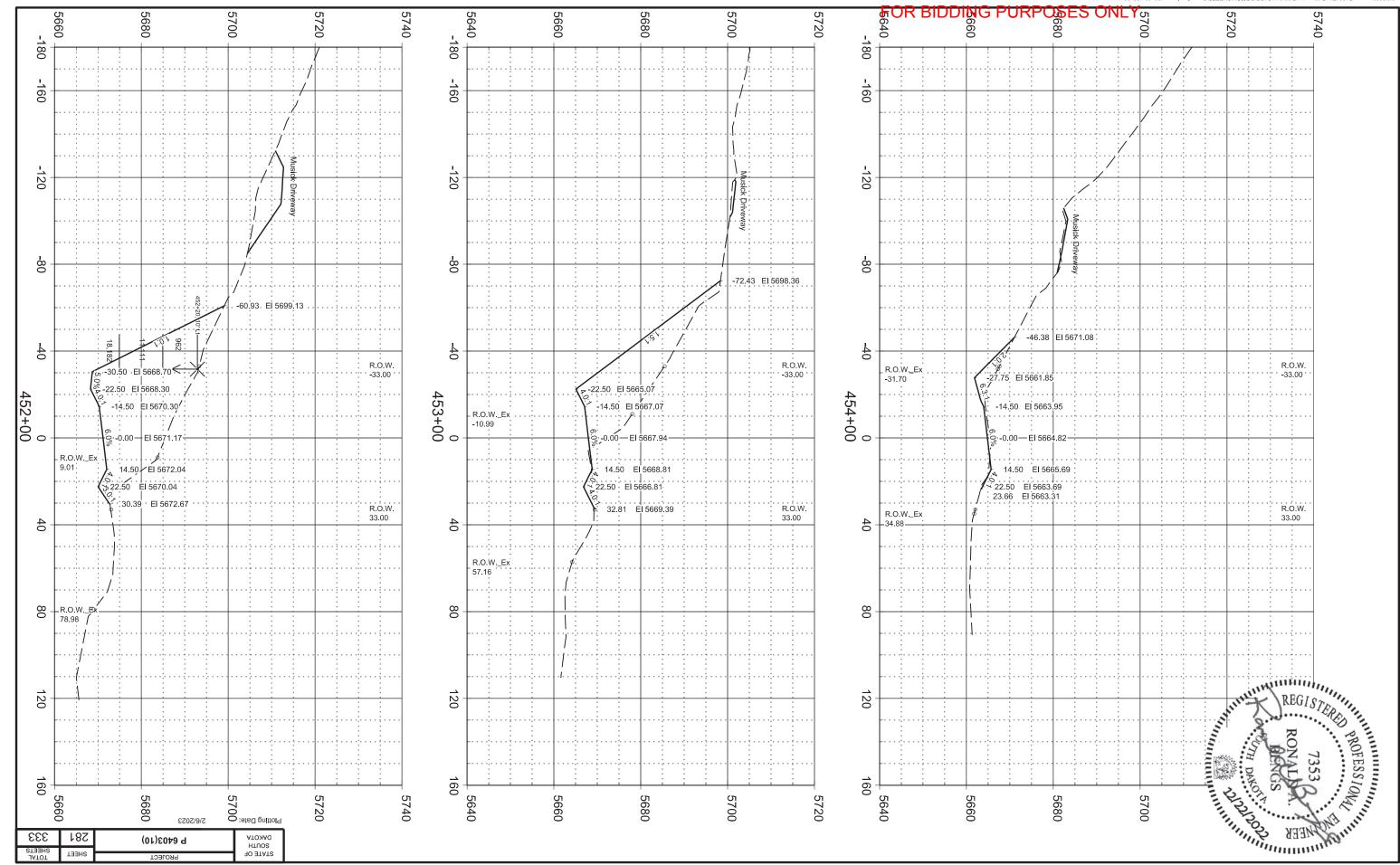


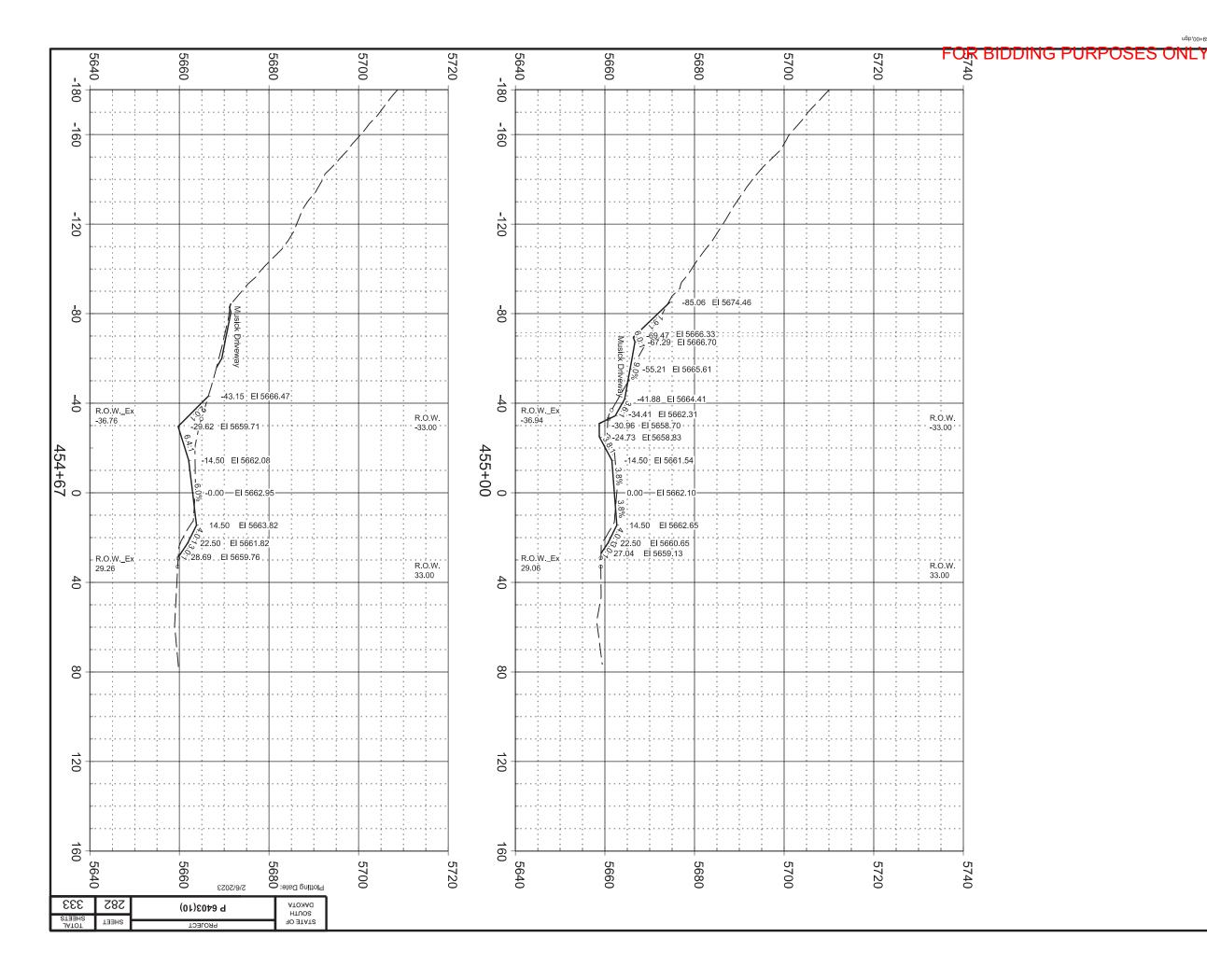
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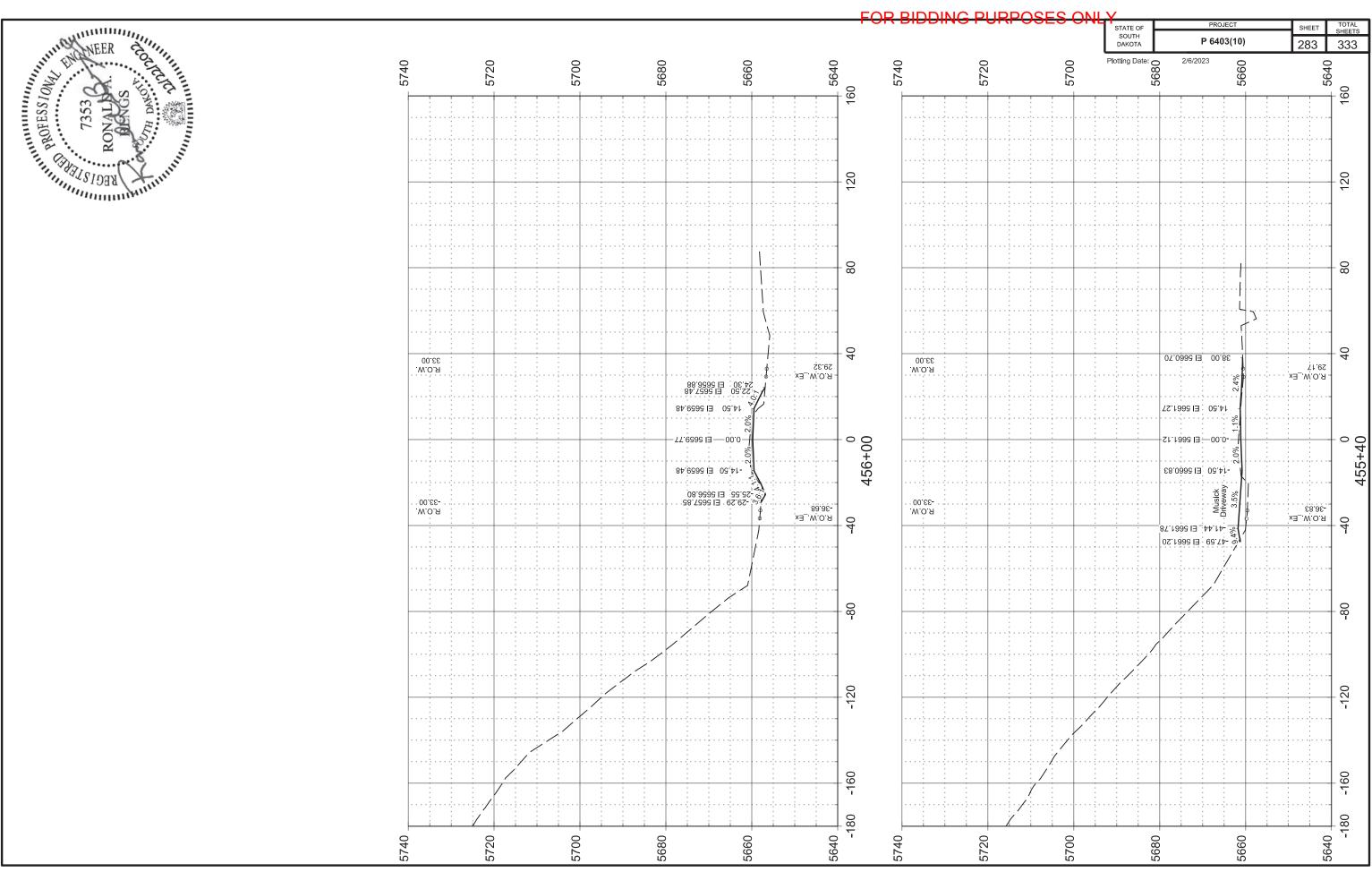


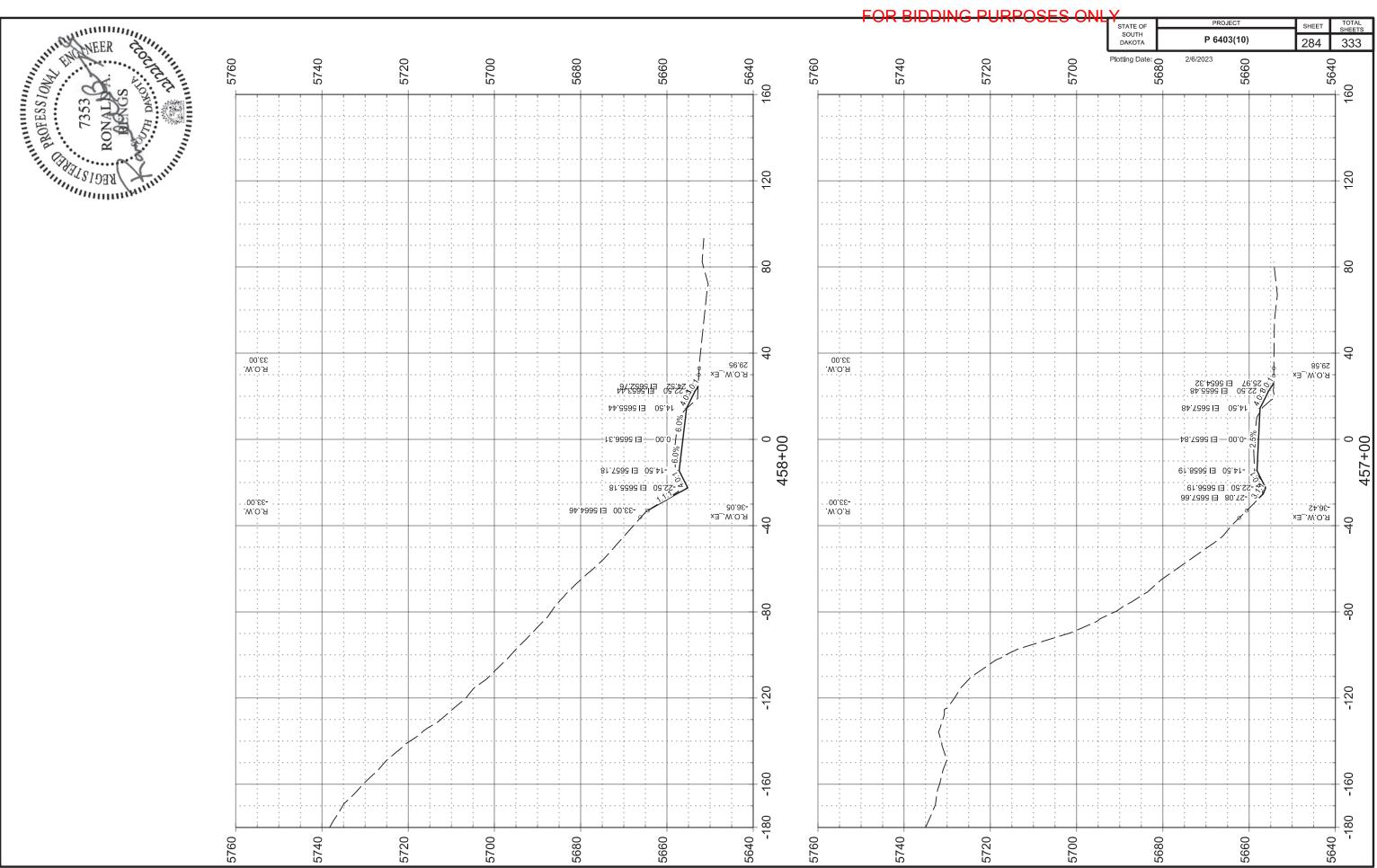


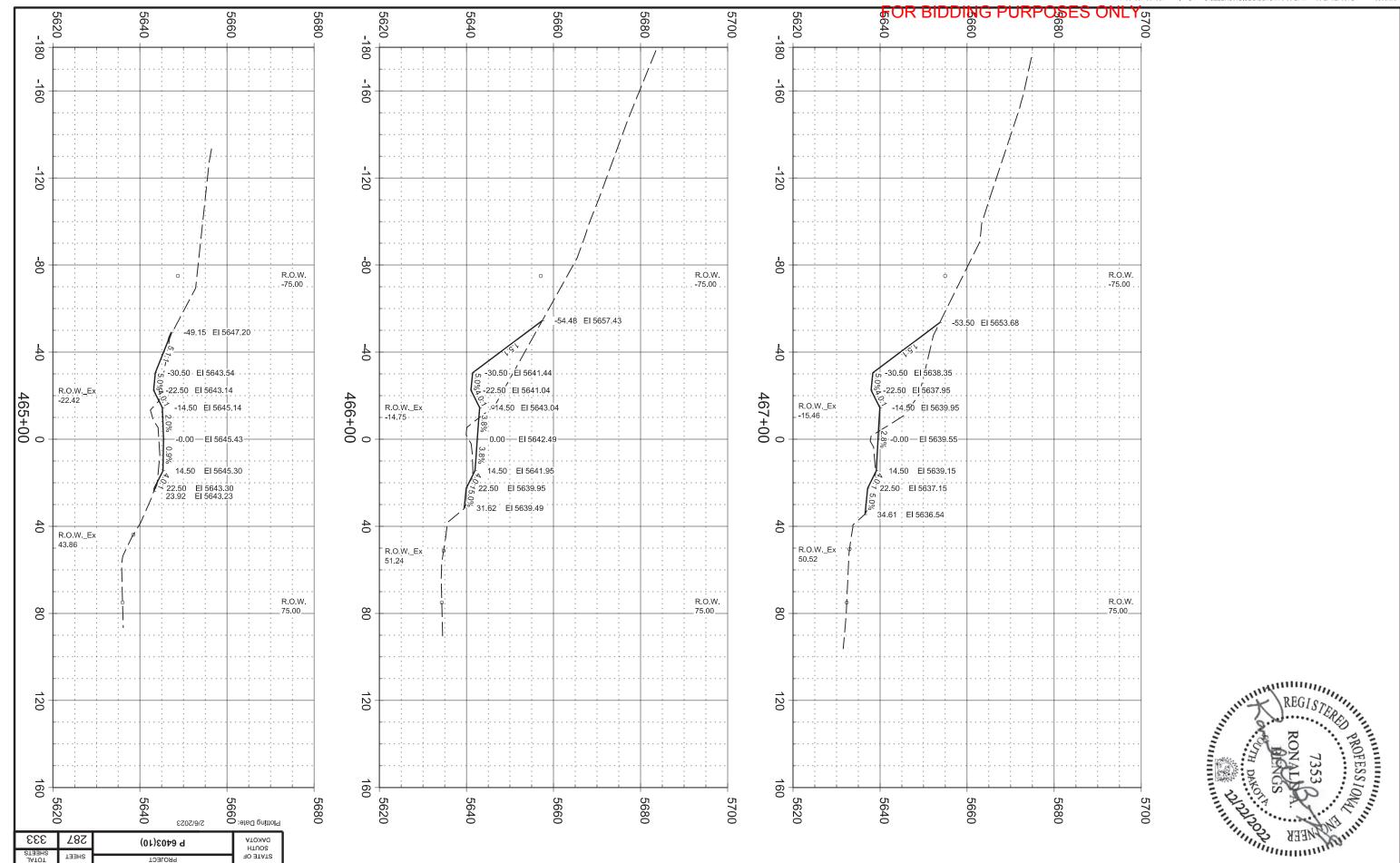


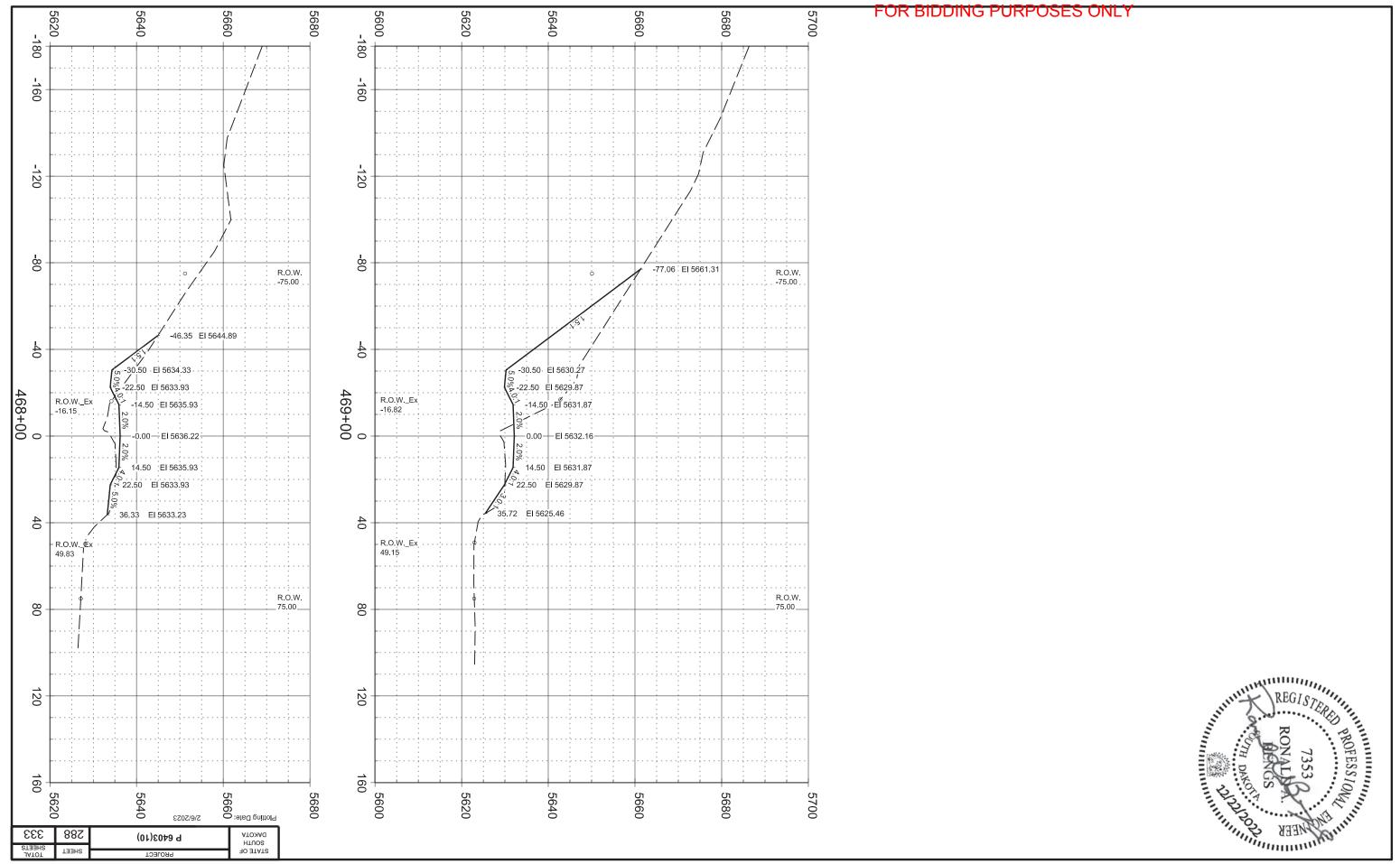


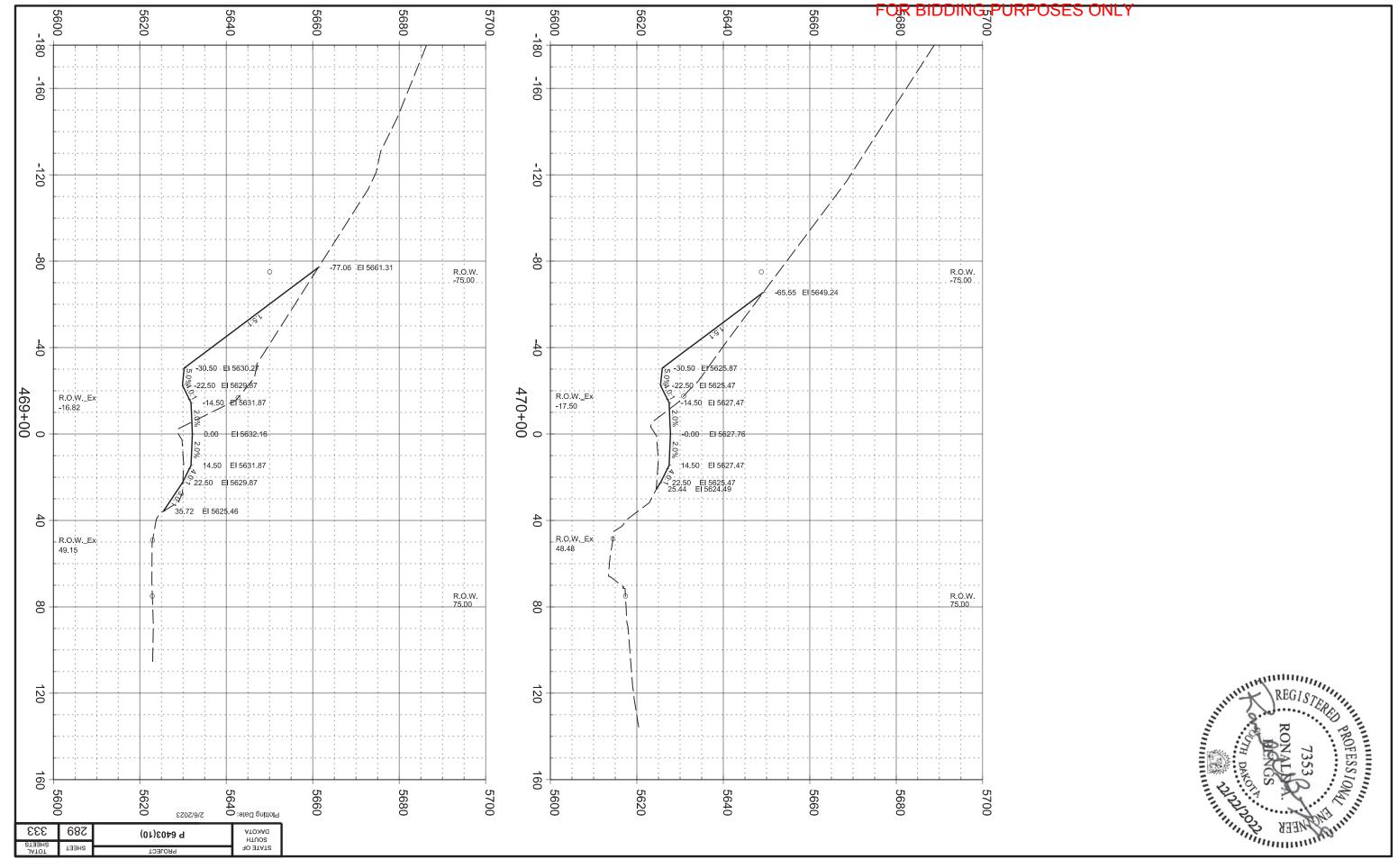


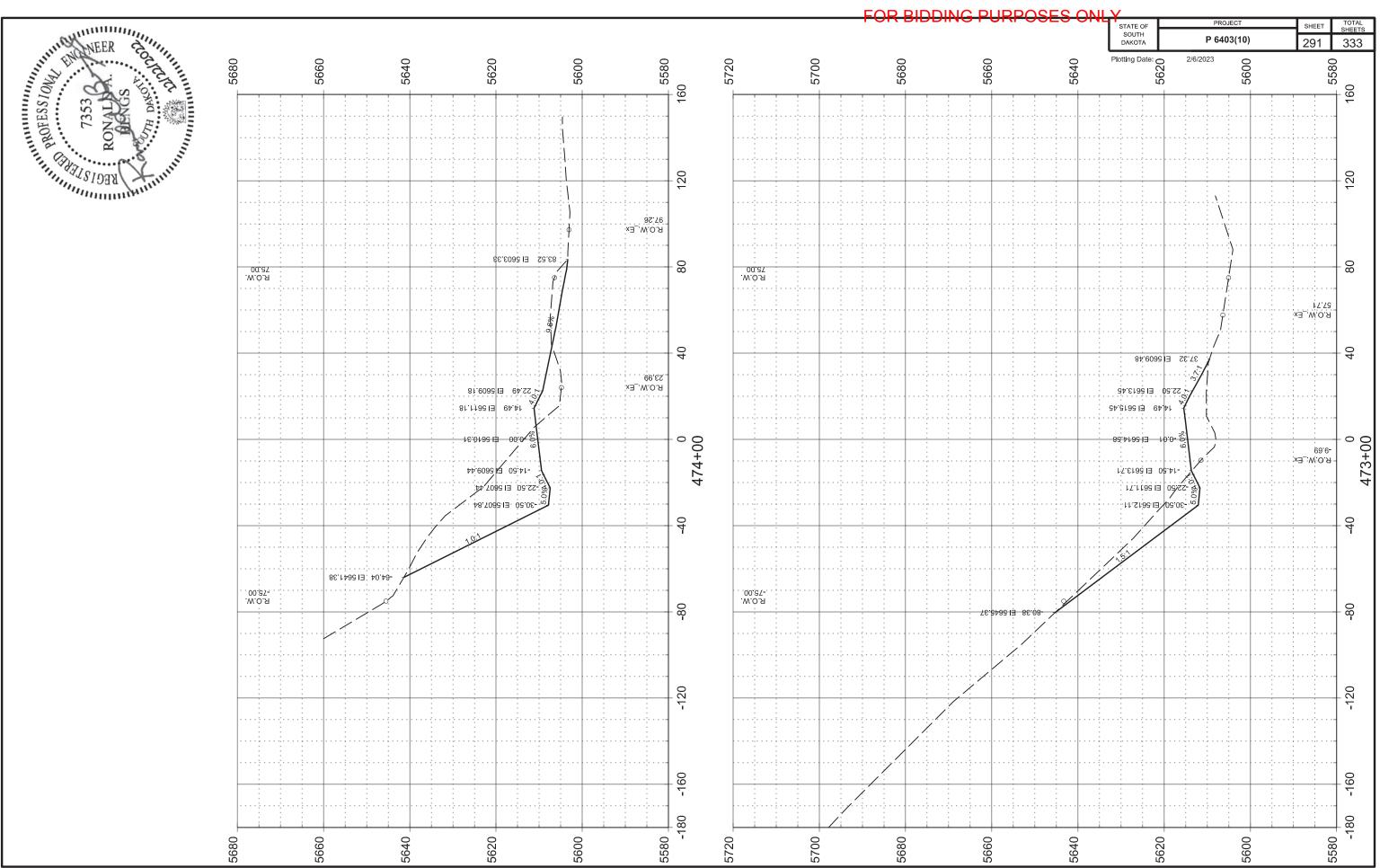


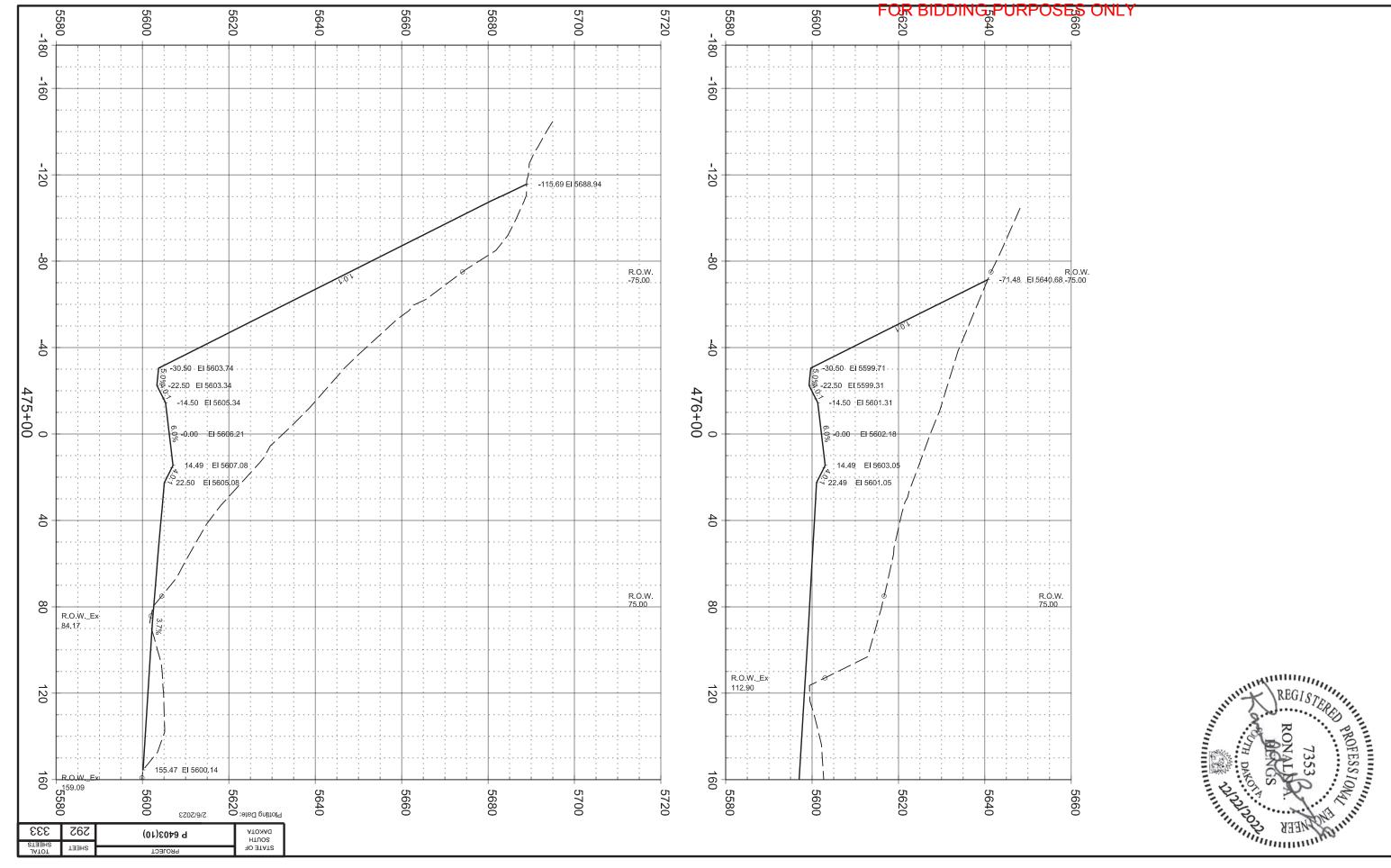


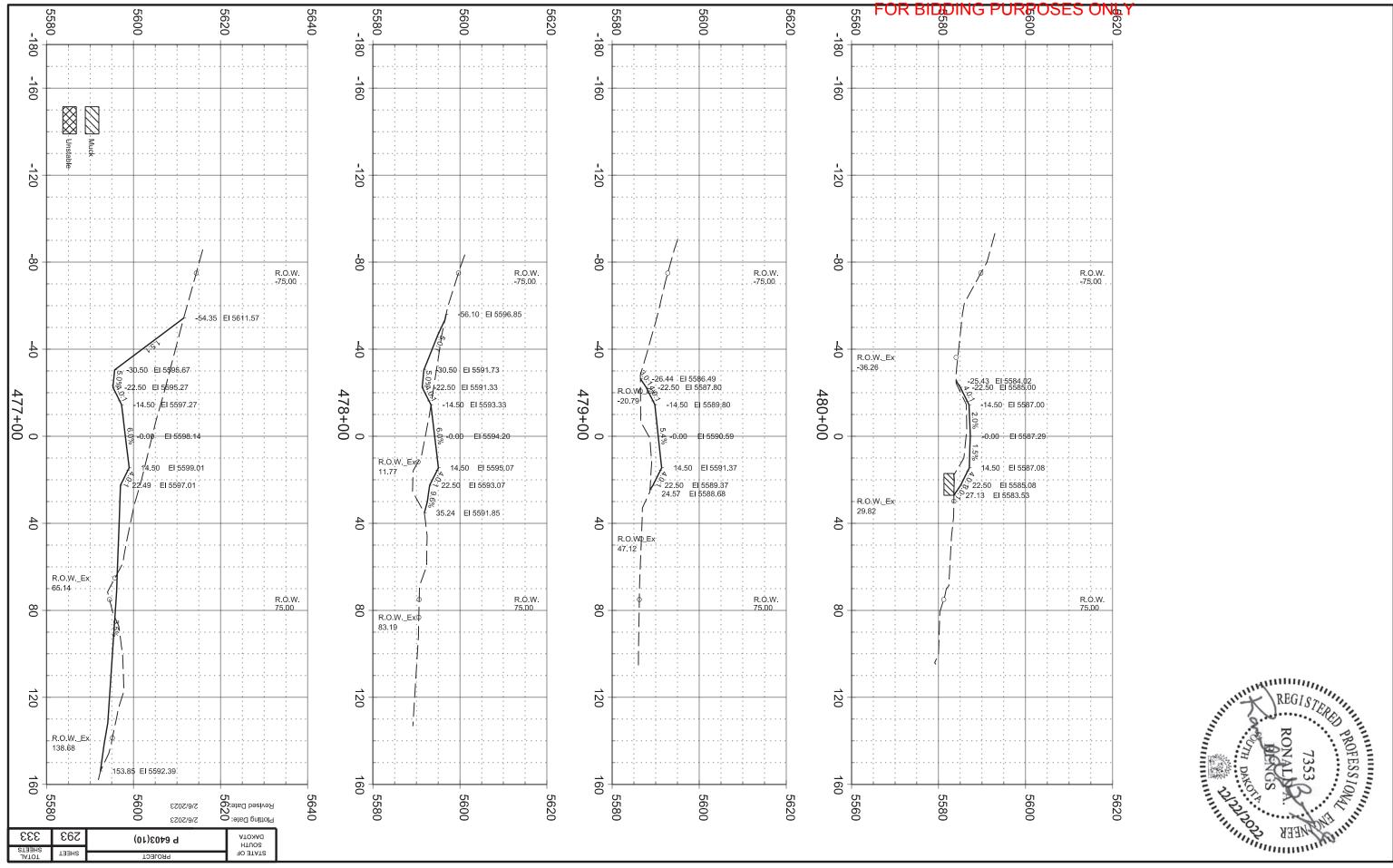


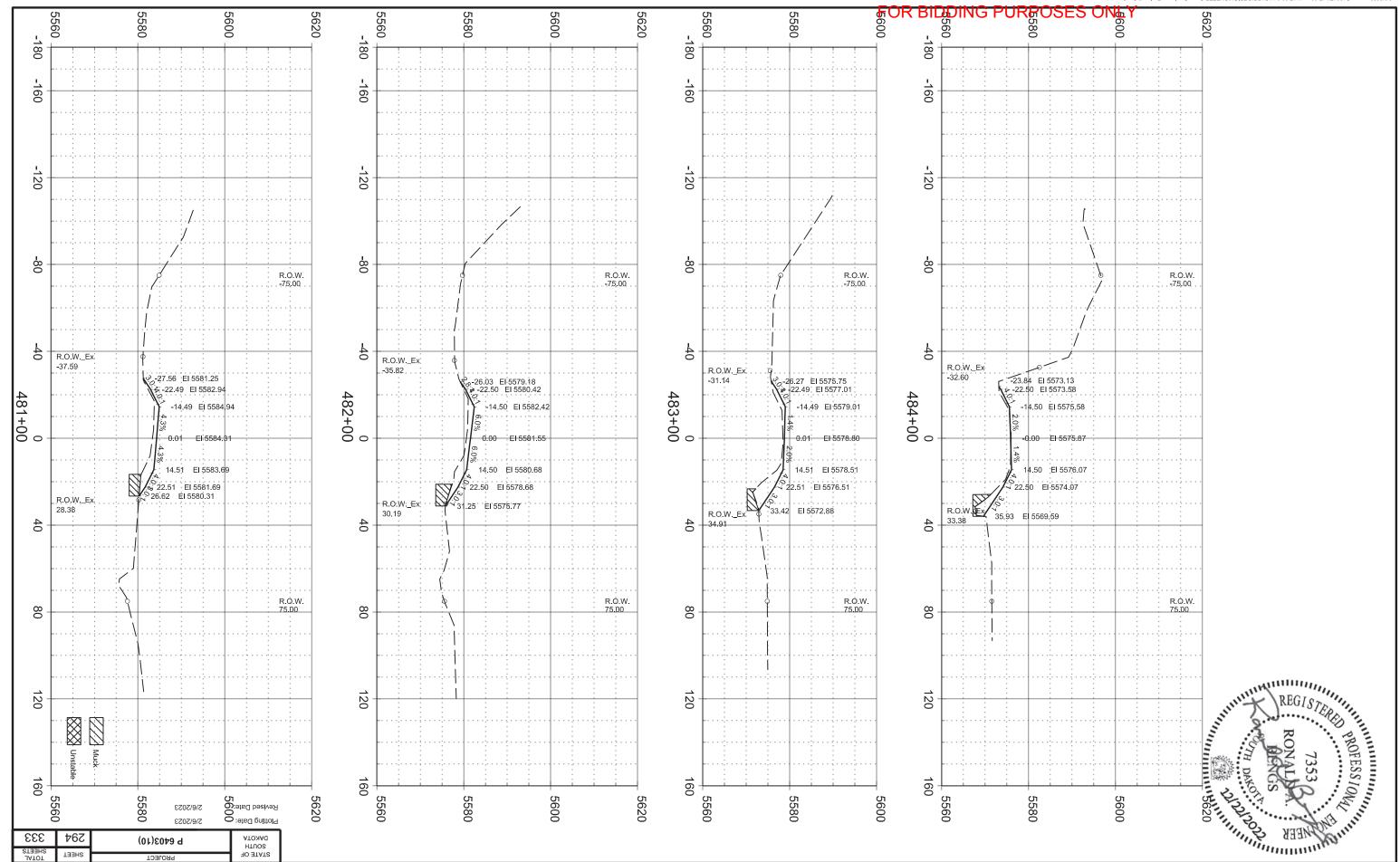


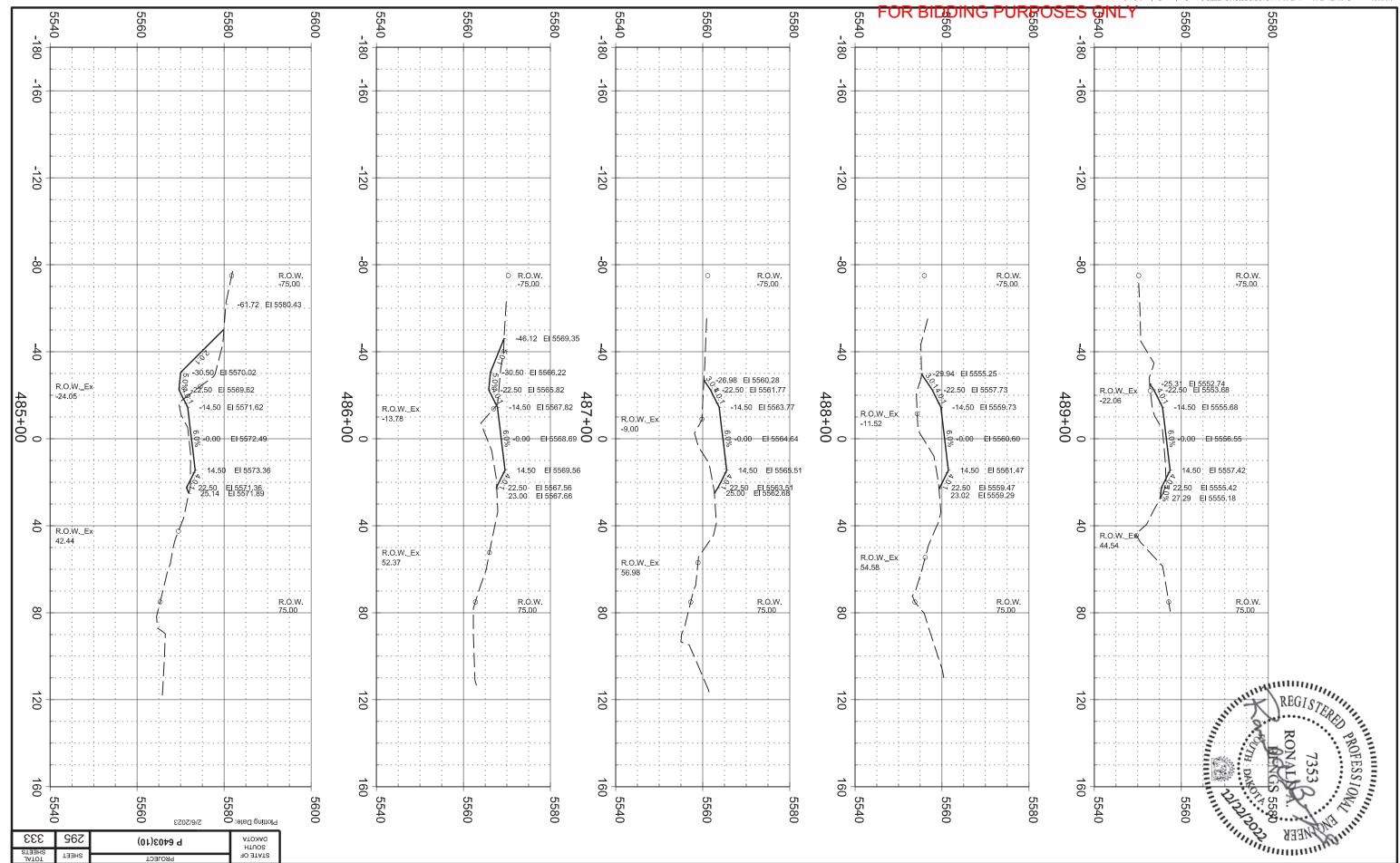






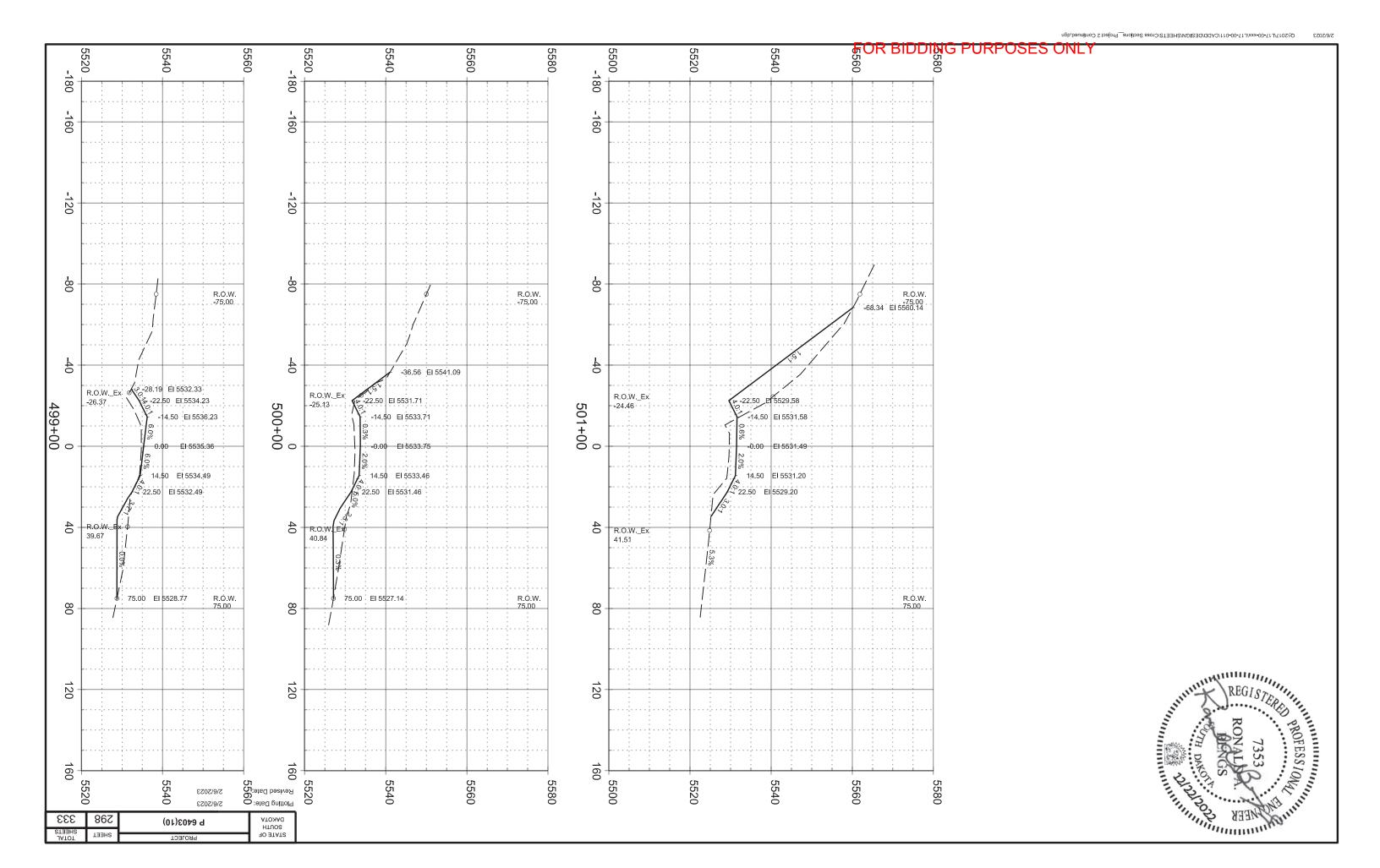


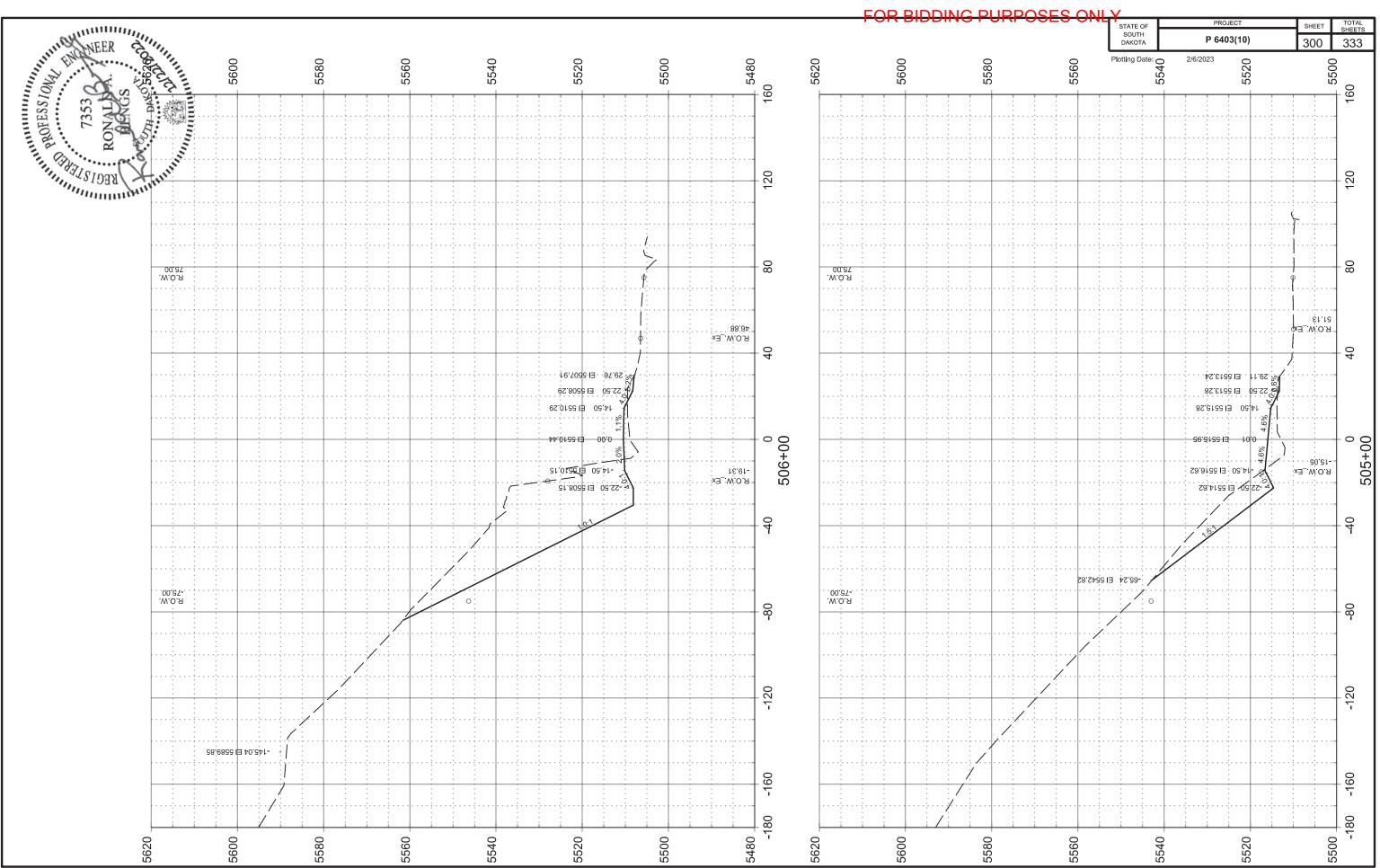


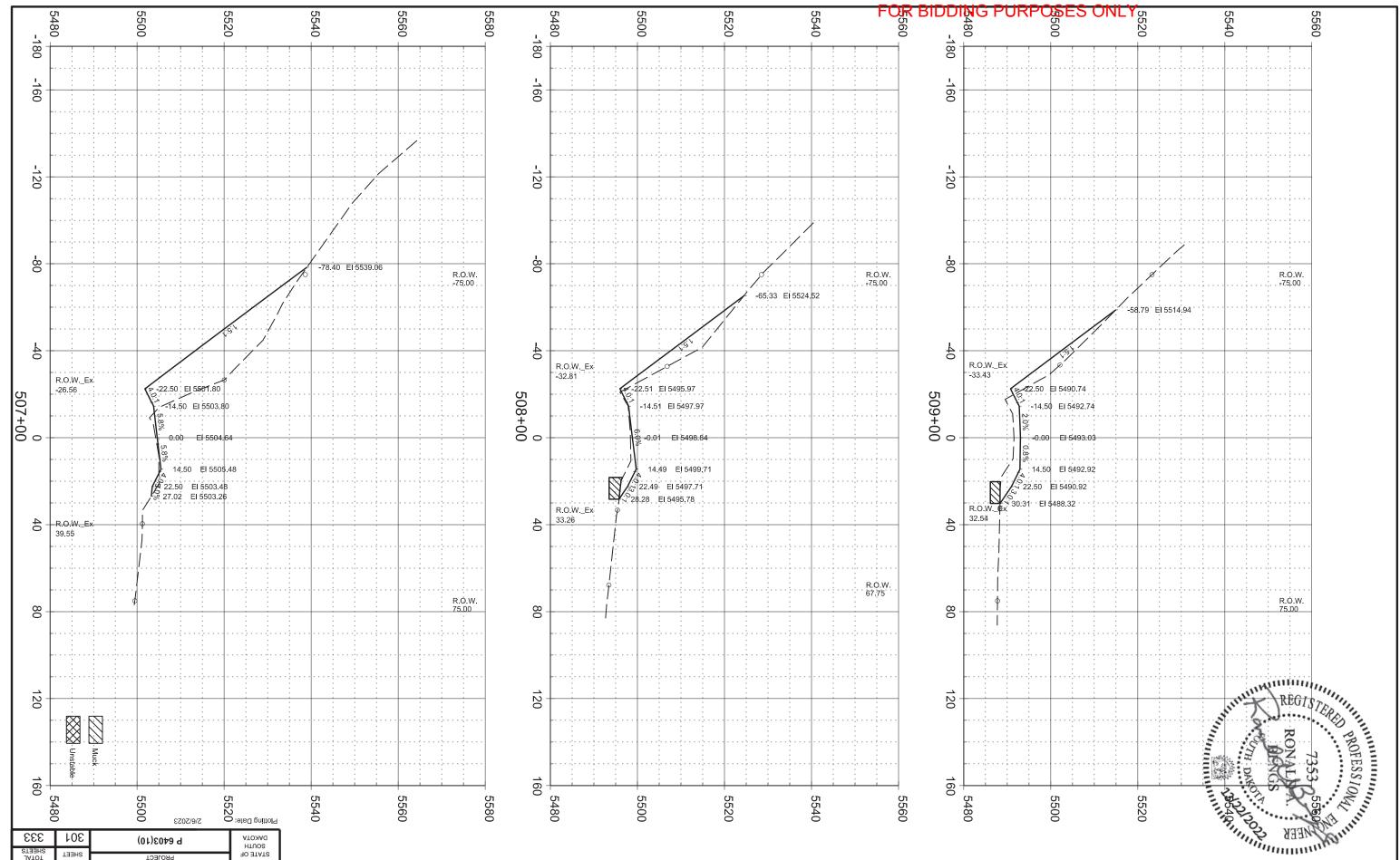


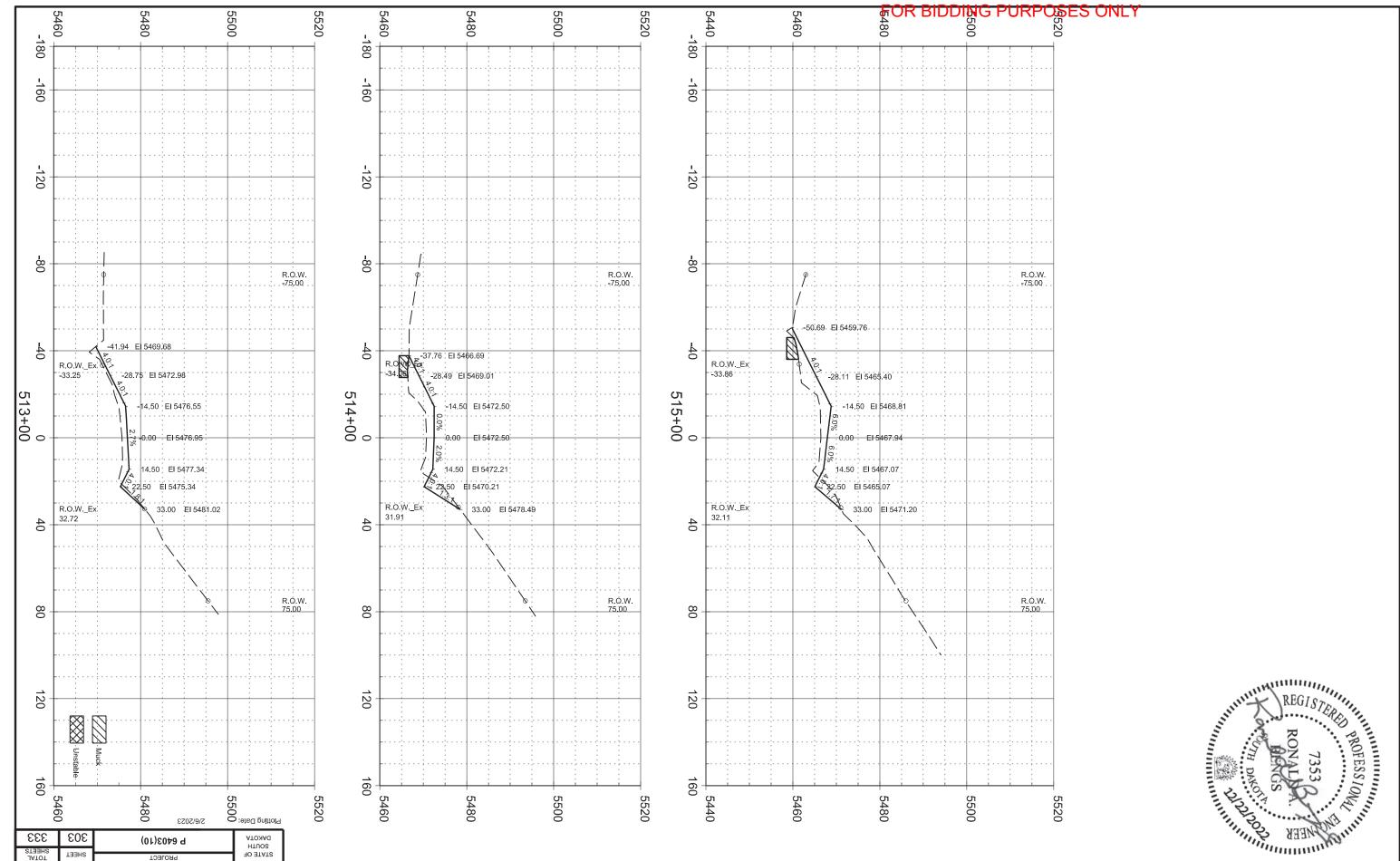
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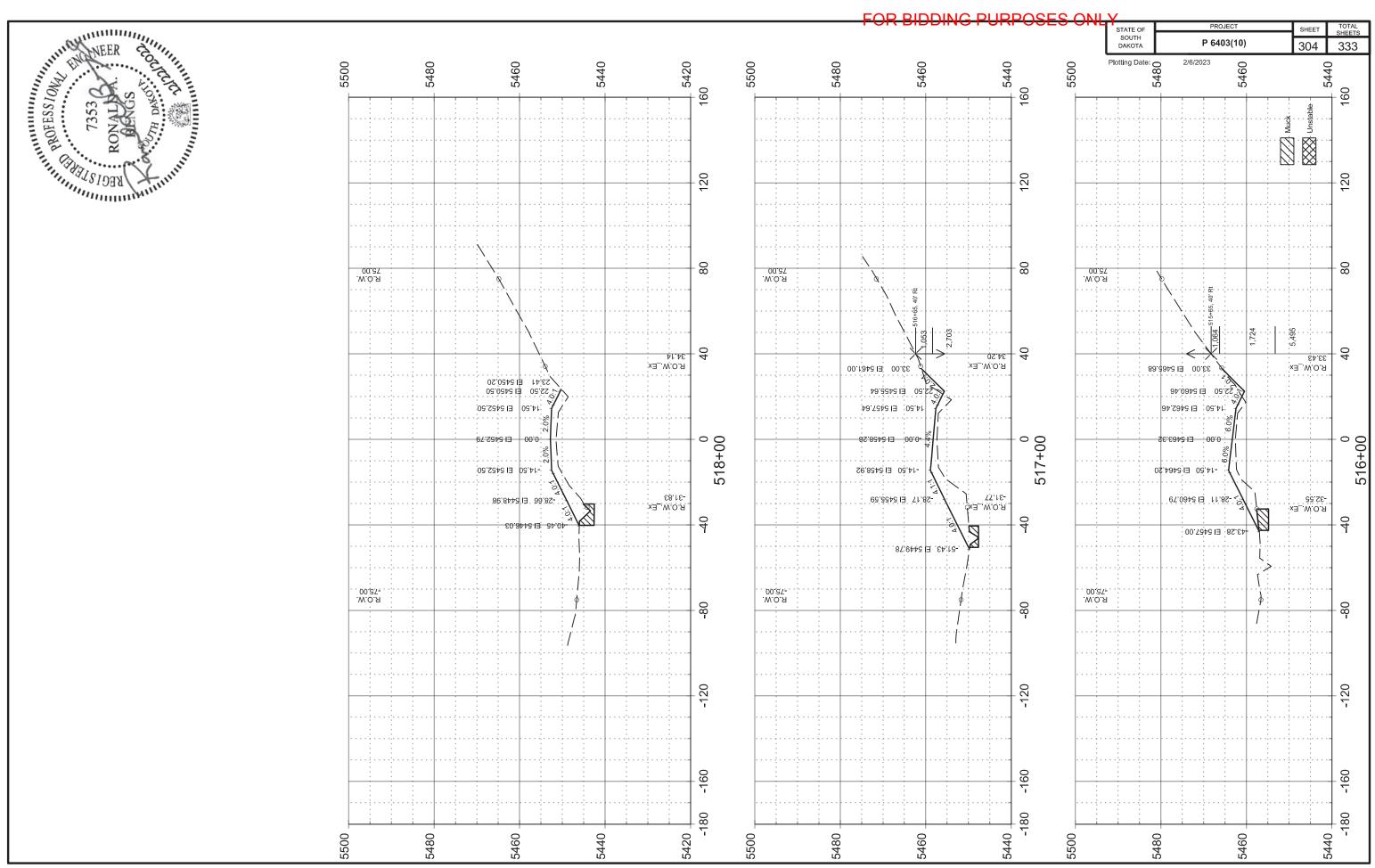
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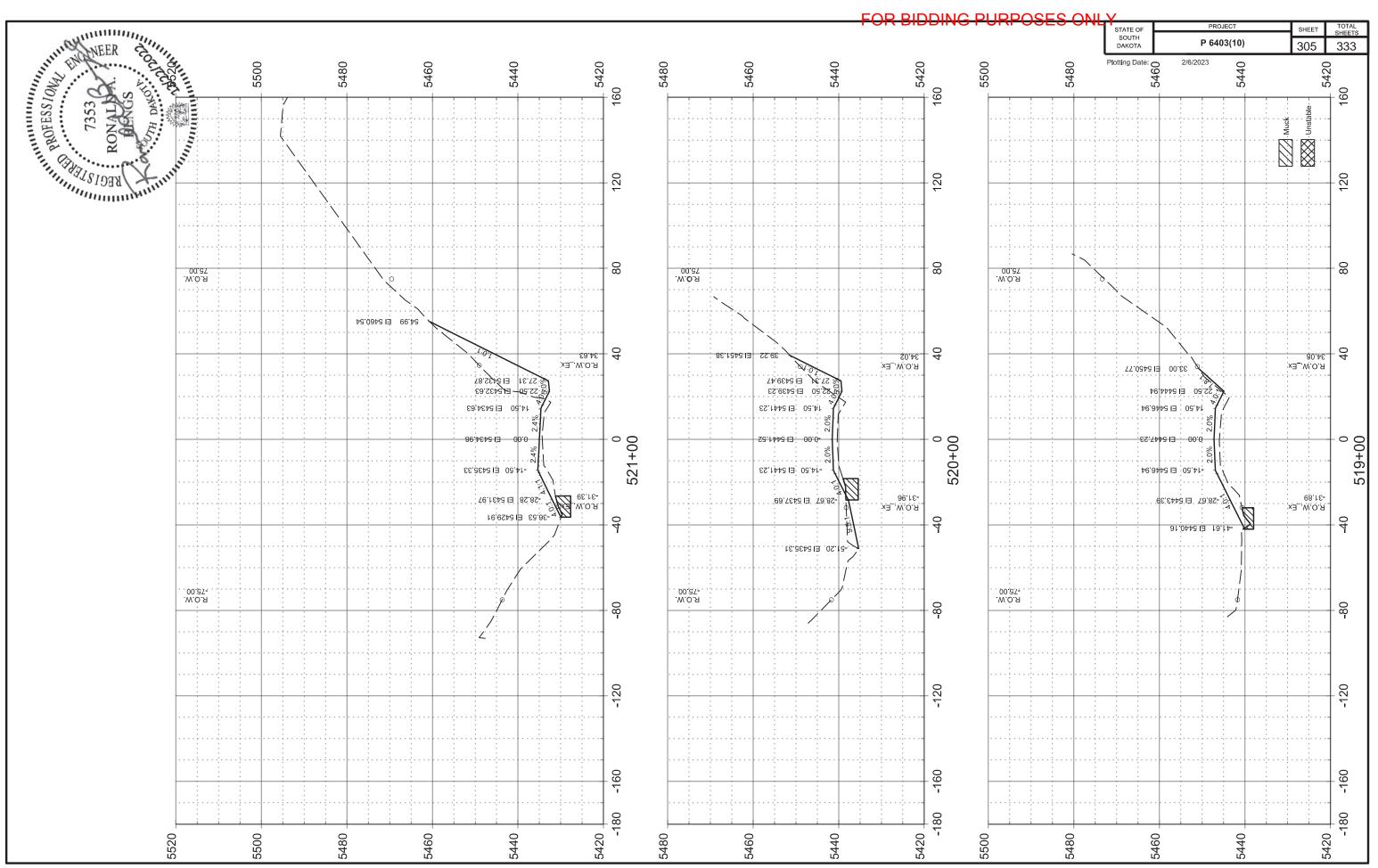


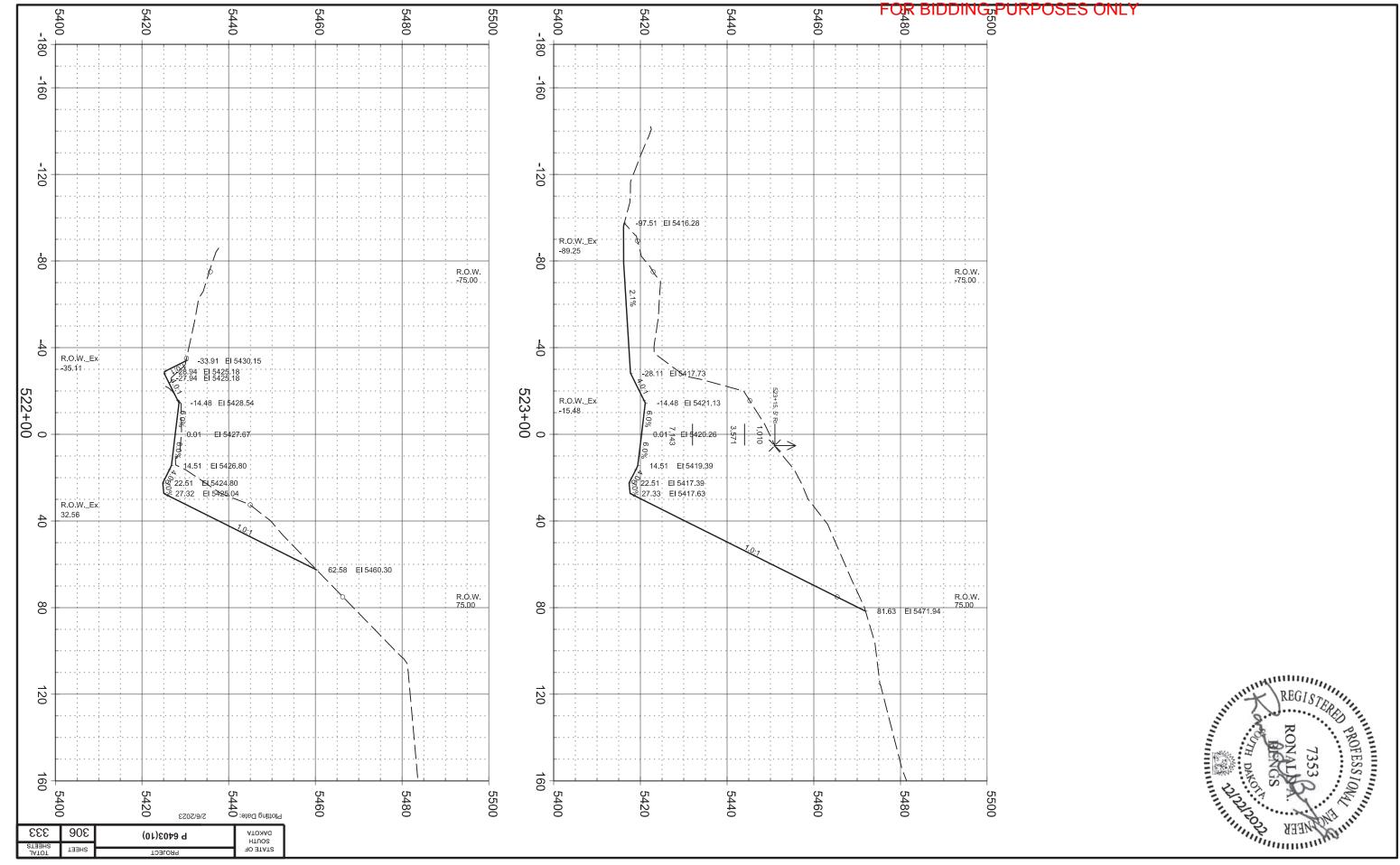


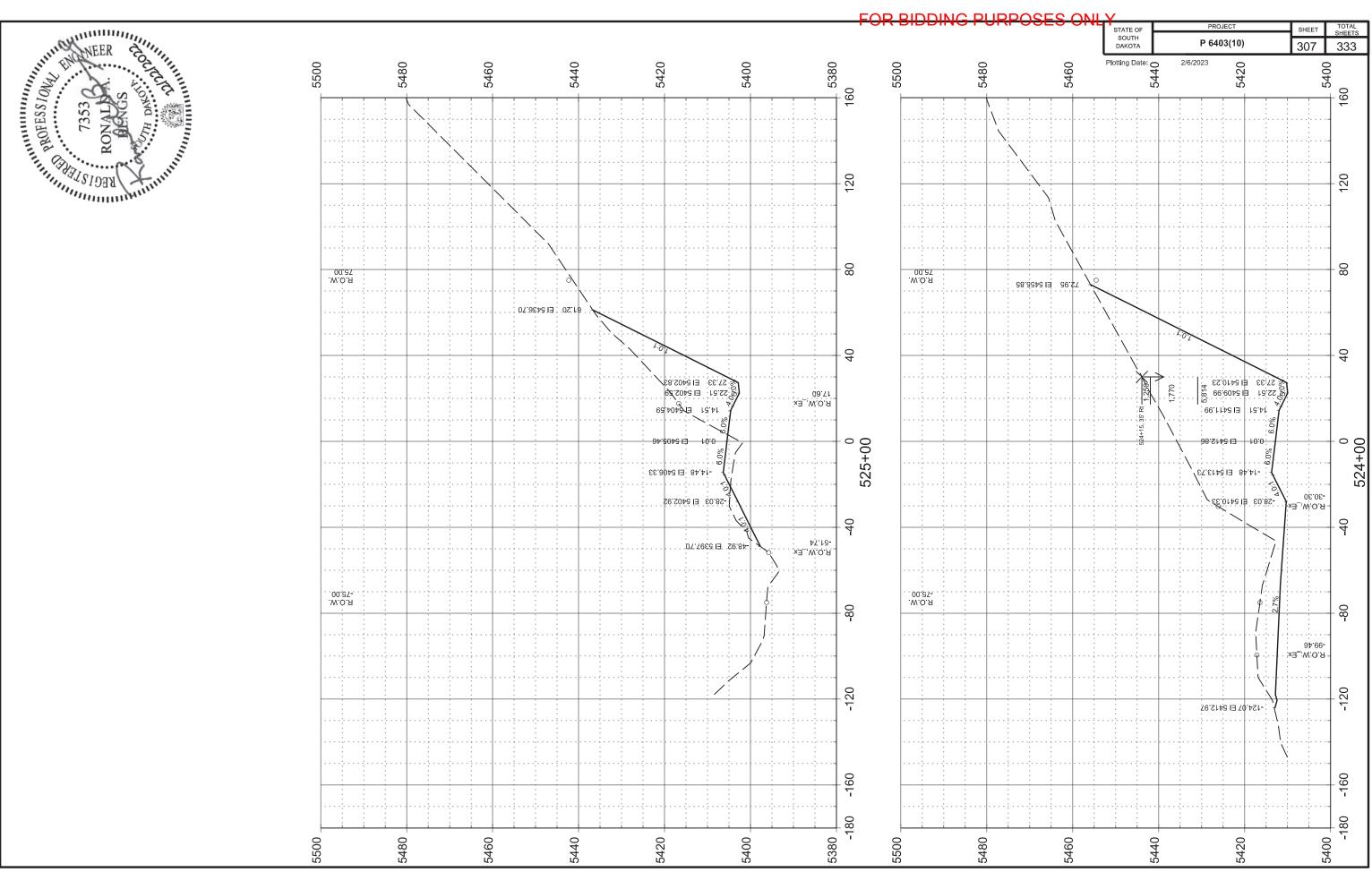


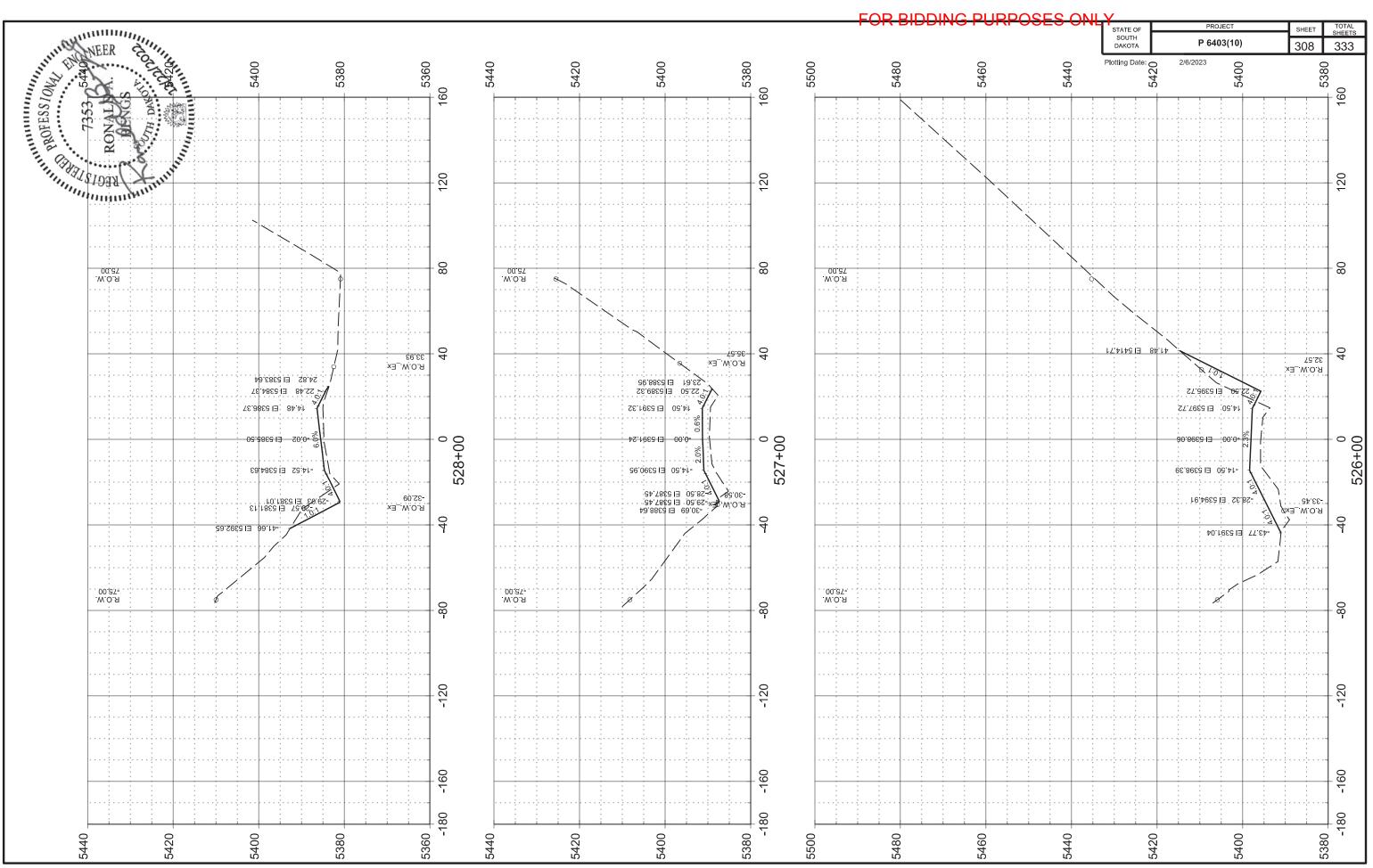


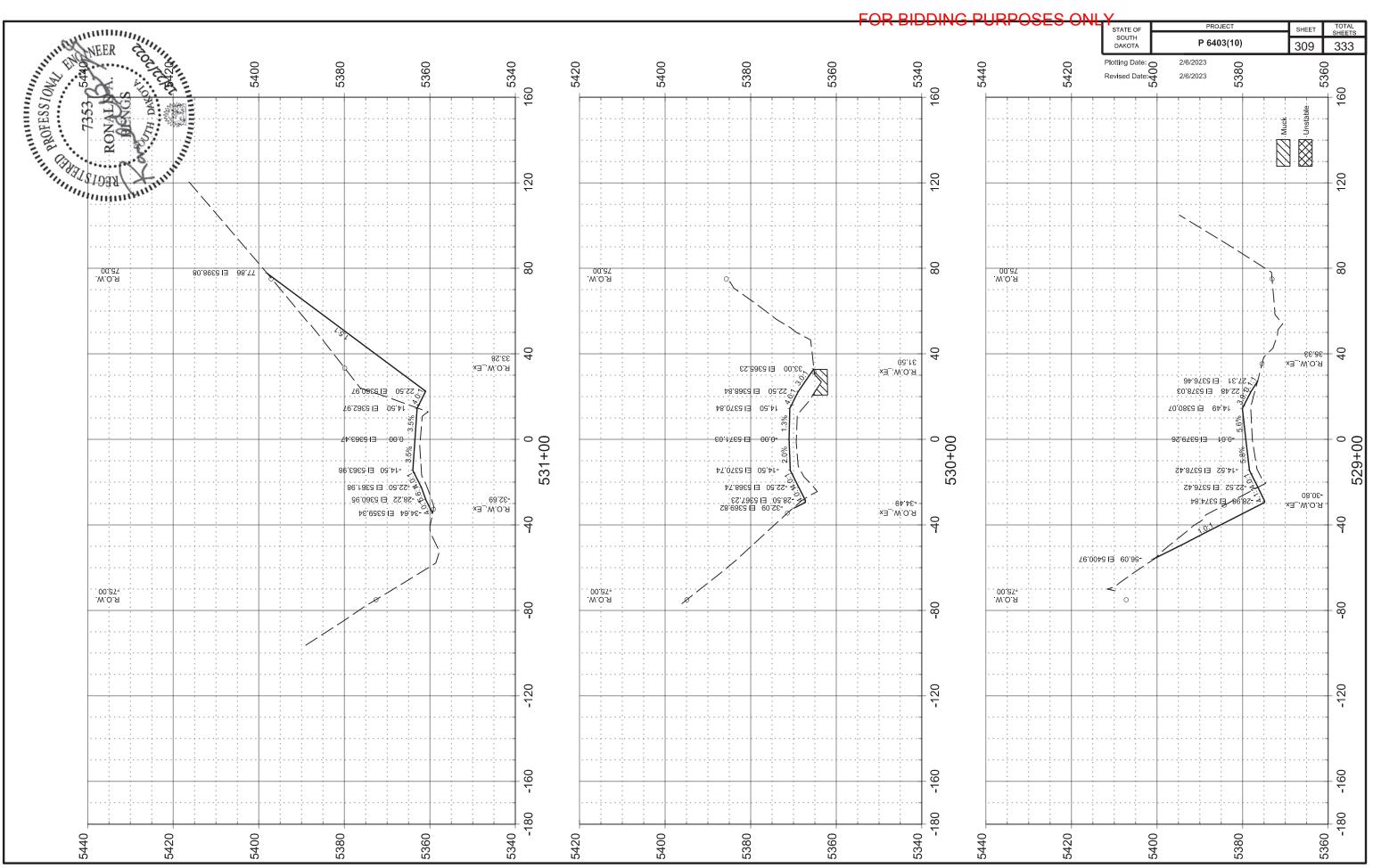


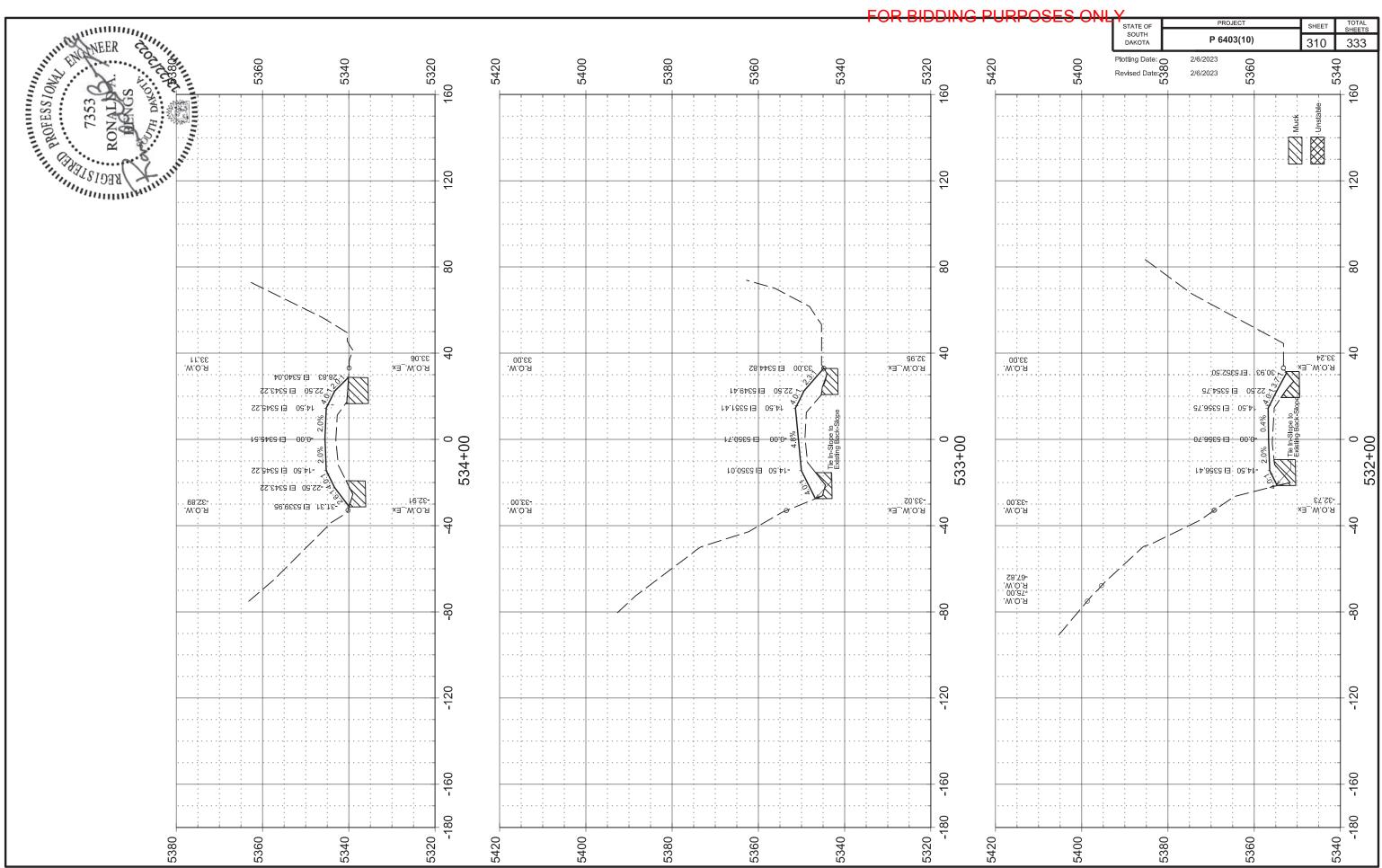








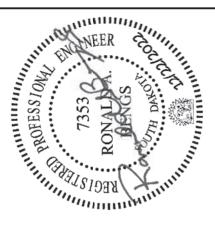


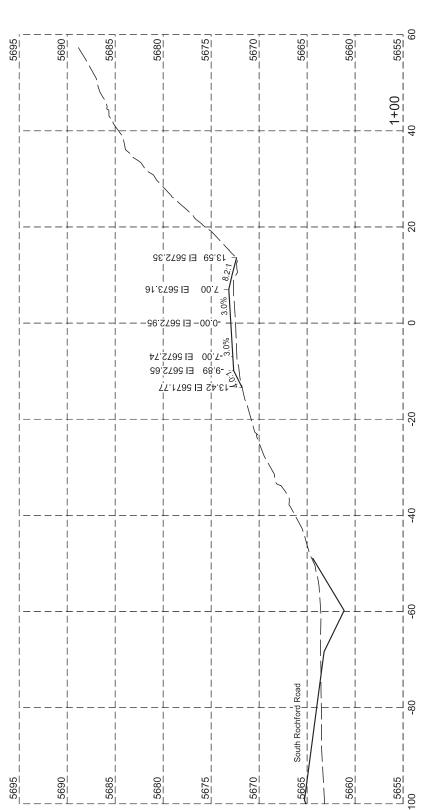


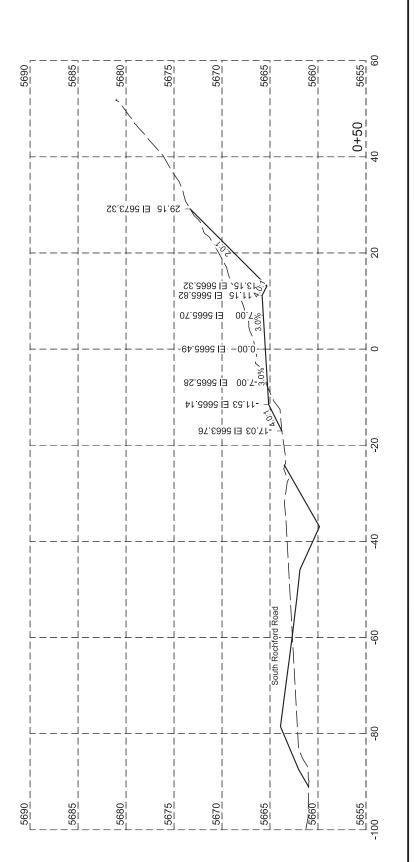
FOR BIDDING PURPOSES ONLY

STATE OF	PROJECT	SHEET	TOTAL
SOUTH DAKOTA	P 6403(10)	311	333

Plotting Date: 2/6/2023







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and and and	뒫			STATE OF SOUTH DAKOTA	PROJECT P 6403(10)	SHEET TOTAL SHEETS
THIS WEEK &	0		_		2/6/2023	312 333
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	50					h Rochford Road
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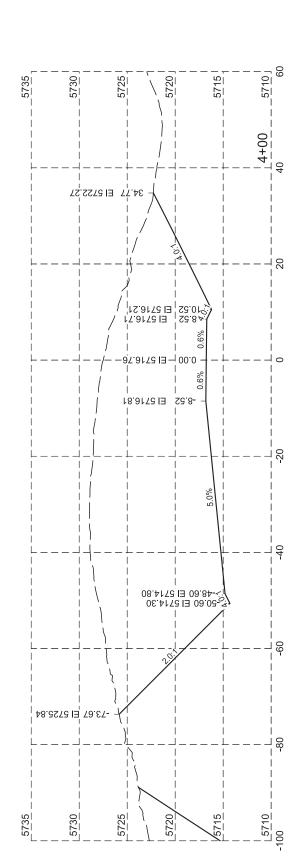
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riveway	28.7698 E3 80.84-	
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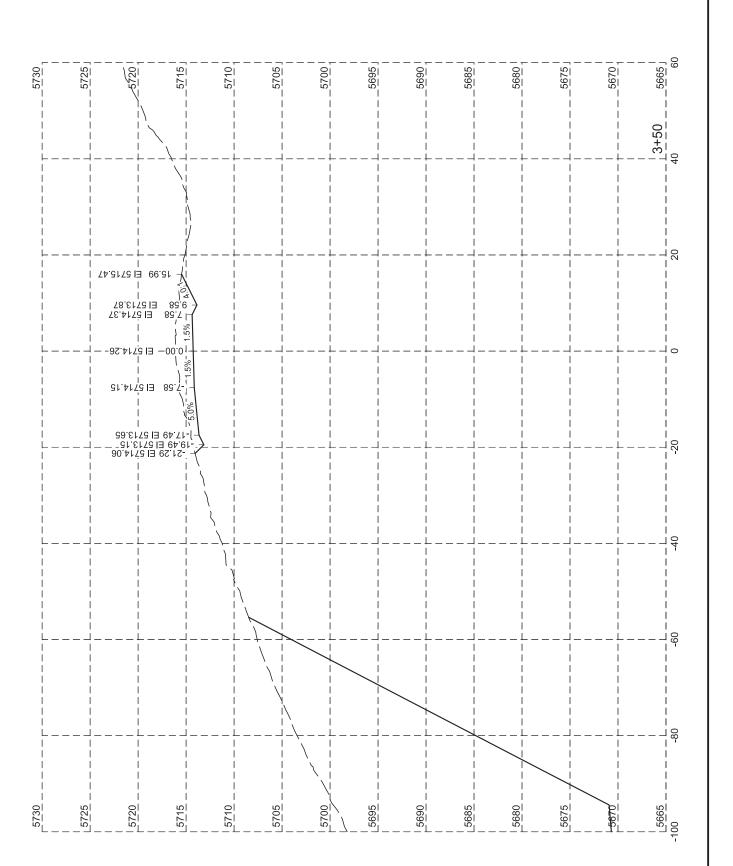
	STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	P 6403(10)	314	333	
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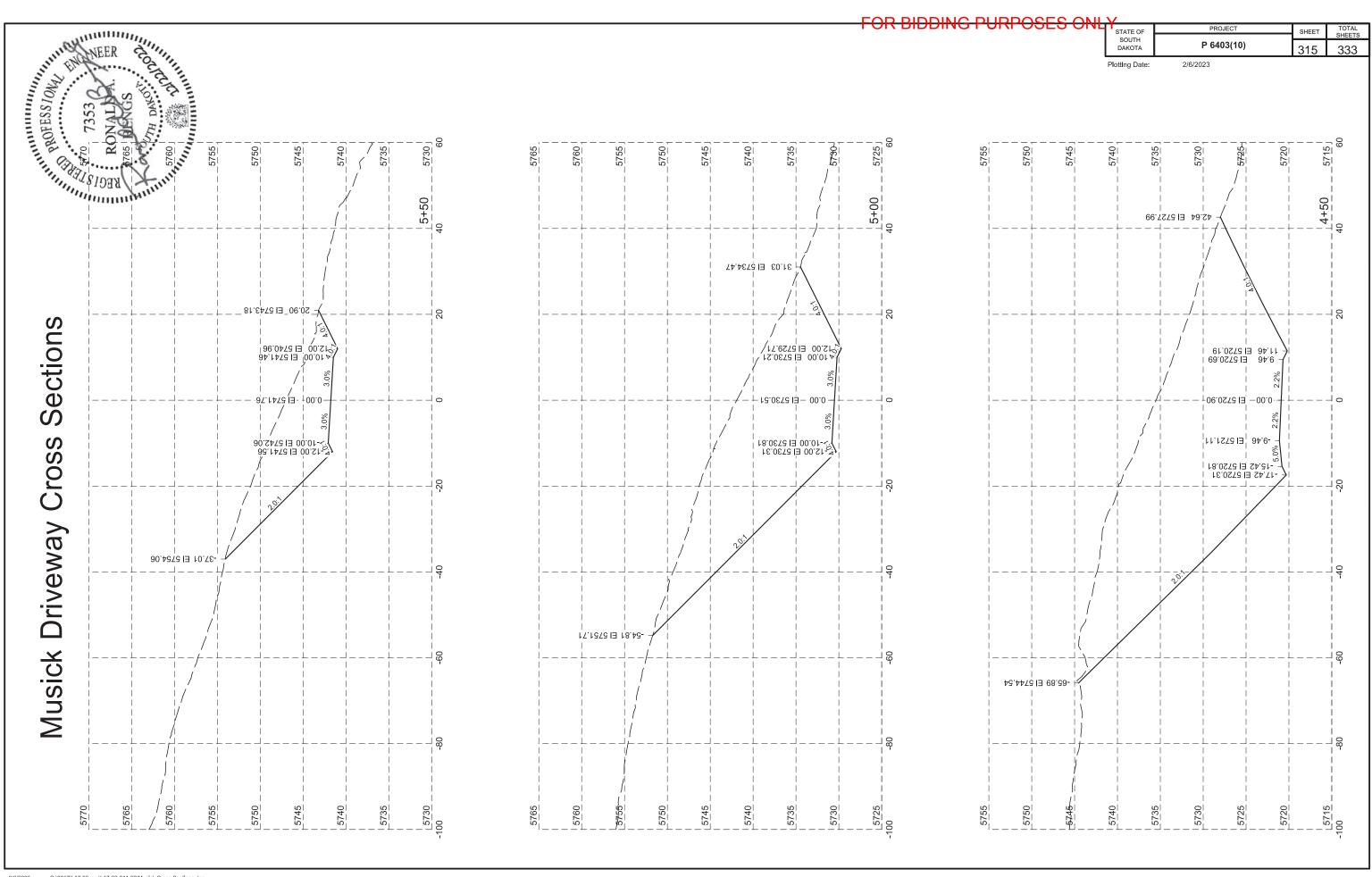
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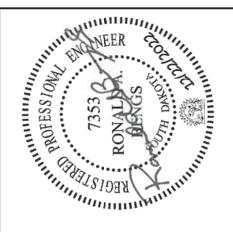




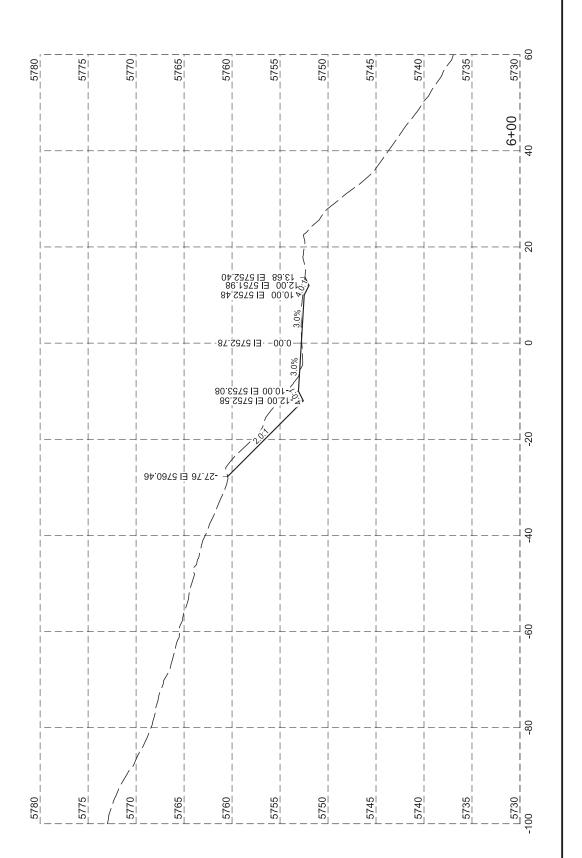
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STATE OF SOUTH DAKOTA P 6403(10) 333 316

Plotting Date: 2/6/2023

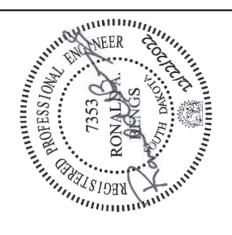


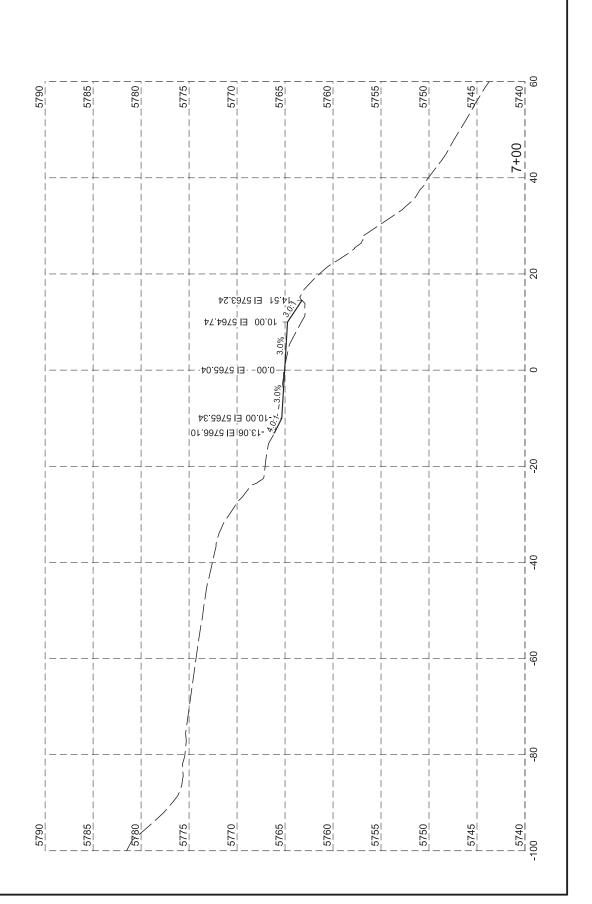
6+50 15 60 EI 5758 17 10.00 EI 5760.45 -94.0948 Hi - 00.0 20.1978 I3 60.11-

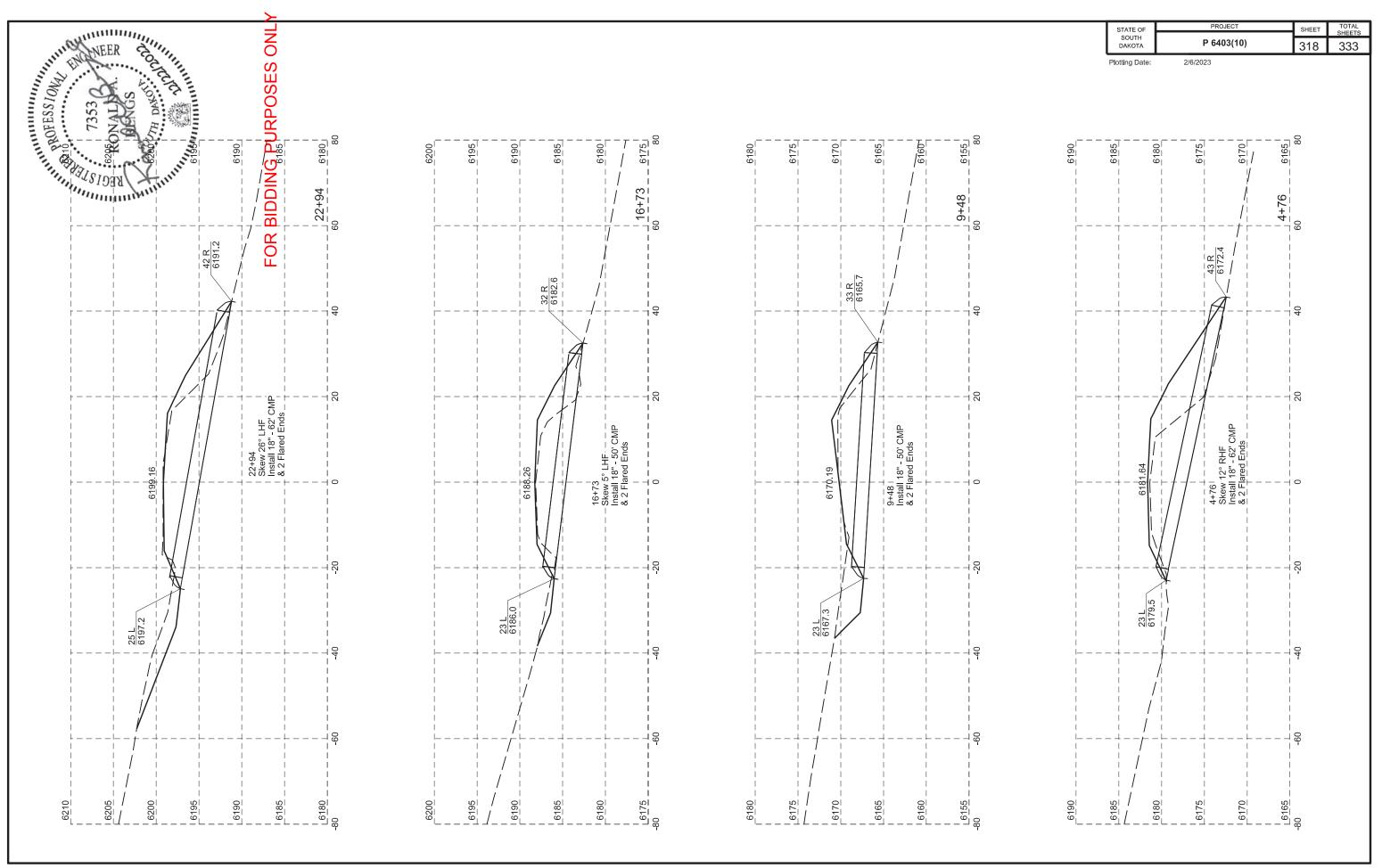


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Ŧ	STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	P 6403(10)	317	333	
F	Plotting Date:	2/6/2023		

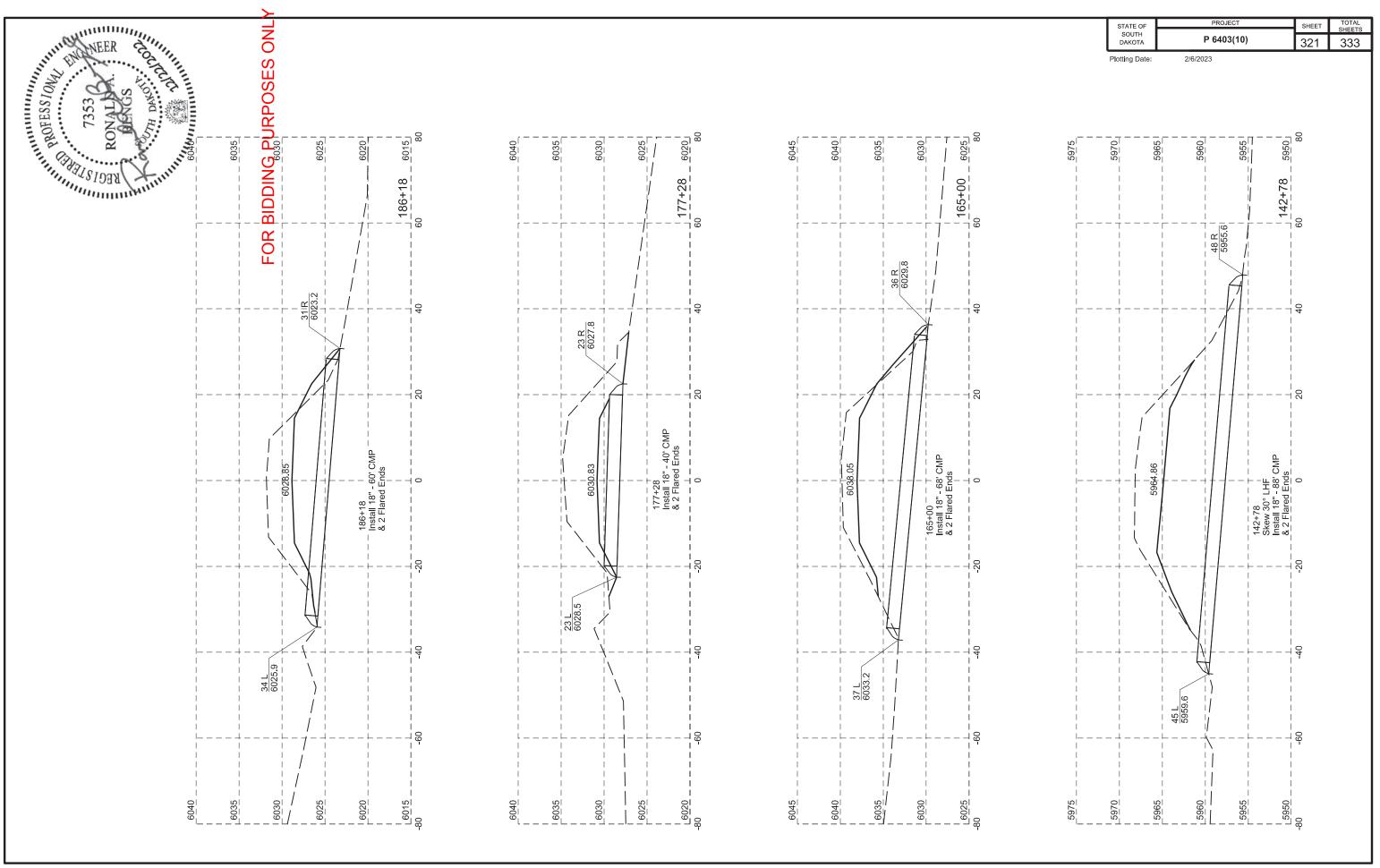




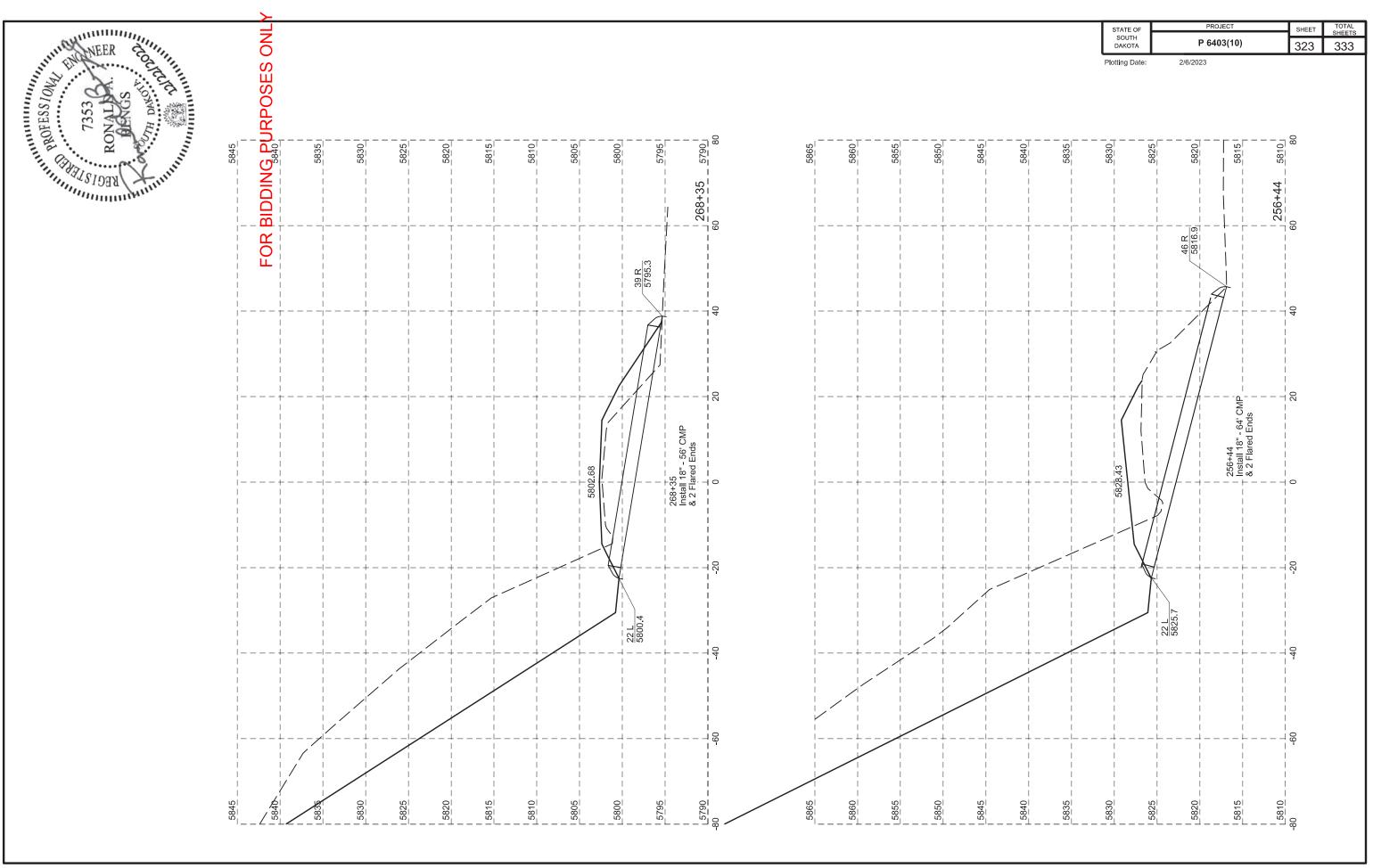


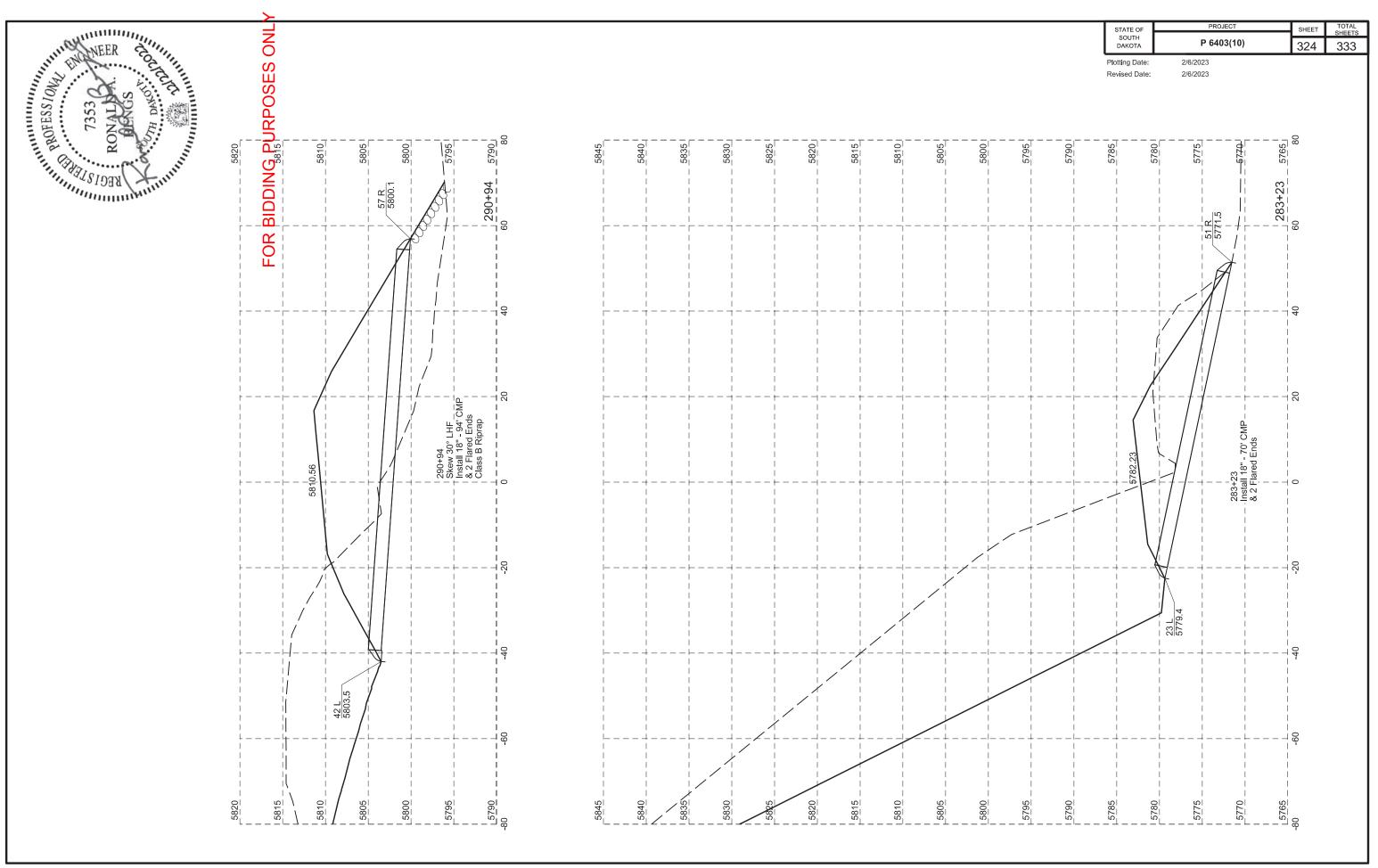
REGISTER TO THE DIENT OF THE DI STATE OF SHEET FOR BIDDING PURPOSES ONL SOUTH P 6403(10) 319 333 Plotting Date: 2/6/2023 <u>6105</u> 80 61<u>20</u>1 80 6120 6130 67+79 09 27+69 +12 78 R 6110.4 ___<u>57</u>+ 37 R 6124.9 27 R 6214.2 78 2 – 20 27+69 | |Install 18" - 44' CMP | & 2 Flared Ends 67+79 | Skew 31° LHF Install 18" - 120' (& 2 Flared Ends 57+12 Skew 43° RHF Install 18" - 72' CMP & 2 Flared Ends 6129.41 6126.83 6217 23 L 6215.8 40 L 6126.2 18 | 61<u>20</u> . 6130 6135 6130 6230

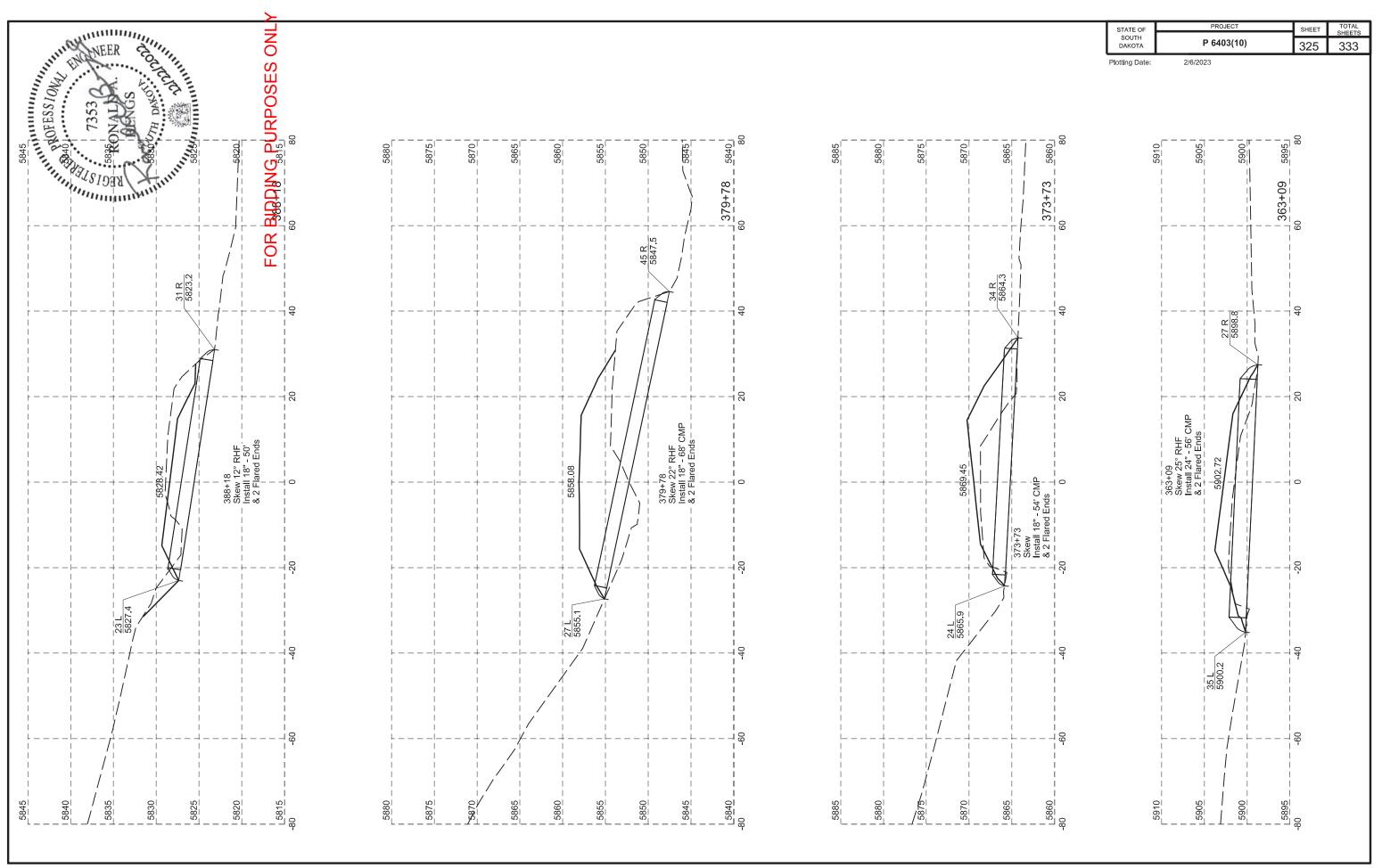
REGISTER TO THE DIENT OF THE DI STATE OF SHEET FOR BIDDING PURPOSES ONL SOUTH P 6403(10) 320 333 Plotting Date: 2/6/2023 6135₁ 80 139+84 114+59 81+76 60 39 R 5947.6 30 R 6149 9 -i ≈ 20 4" - 74' CMP-ed Ends 81+76 Install 18" - 72' CMP & 2 Flared Ends 139+84 | |Install 24" & 2 Flared Grade Inlet Area To Drain 34 L 5931.1 37 L 5950.1 |6135 |-80 5950 5945 5940 6170 6160 6150



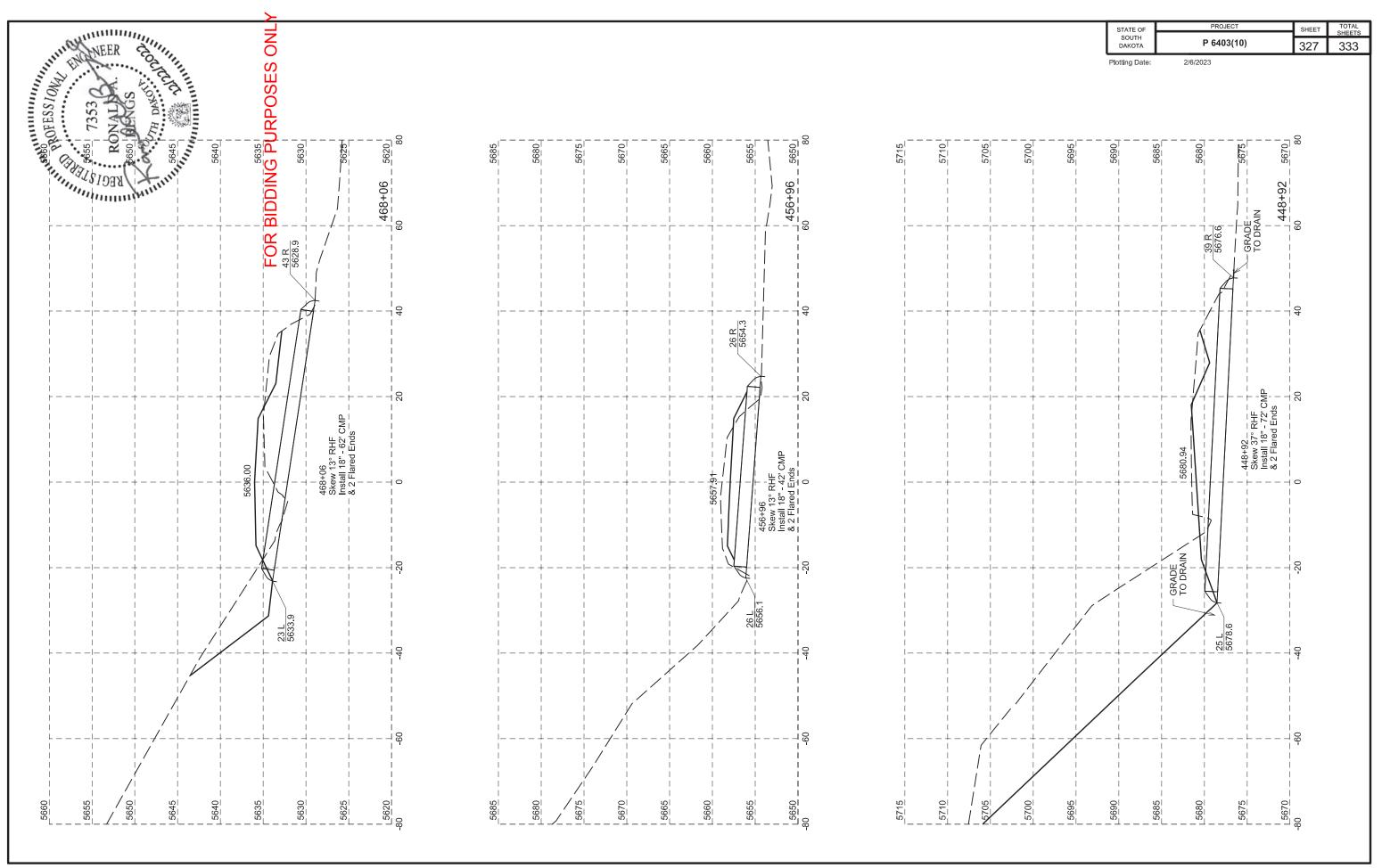
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RONAL PROPERTY OF THE PARTY OF		5850 5845 5840 5835 5835 5830 60 80	5900 5895	6005 6005 6005 6005 6005 6005 6005 6005
	45" - 76' CMP	38 R		32 R
- 	246+12 Skew 17° RHF Install Upstream Perchec 8.1 Flared End Pipe Design is perched to water flow through perme	34 R 5834.7 40		20 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
 -		243+66 Skew 44° RHF Install 24" - 45° Bend 5839 29	894.45	54
 - -		243+6 Skew, Install T 1nstall & 2 Fit		6017 1 1 18 18 18 18 18 18 18 18 18 18 18 18
 -				27.L 6013.0 1
 -	38 L 5829.5 5829.5 	37 L 1 37 L		
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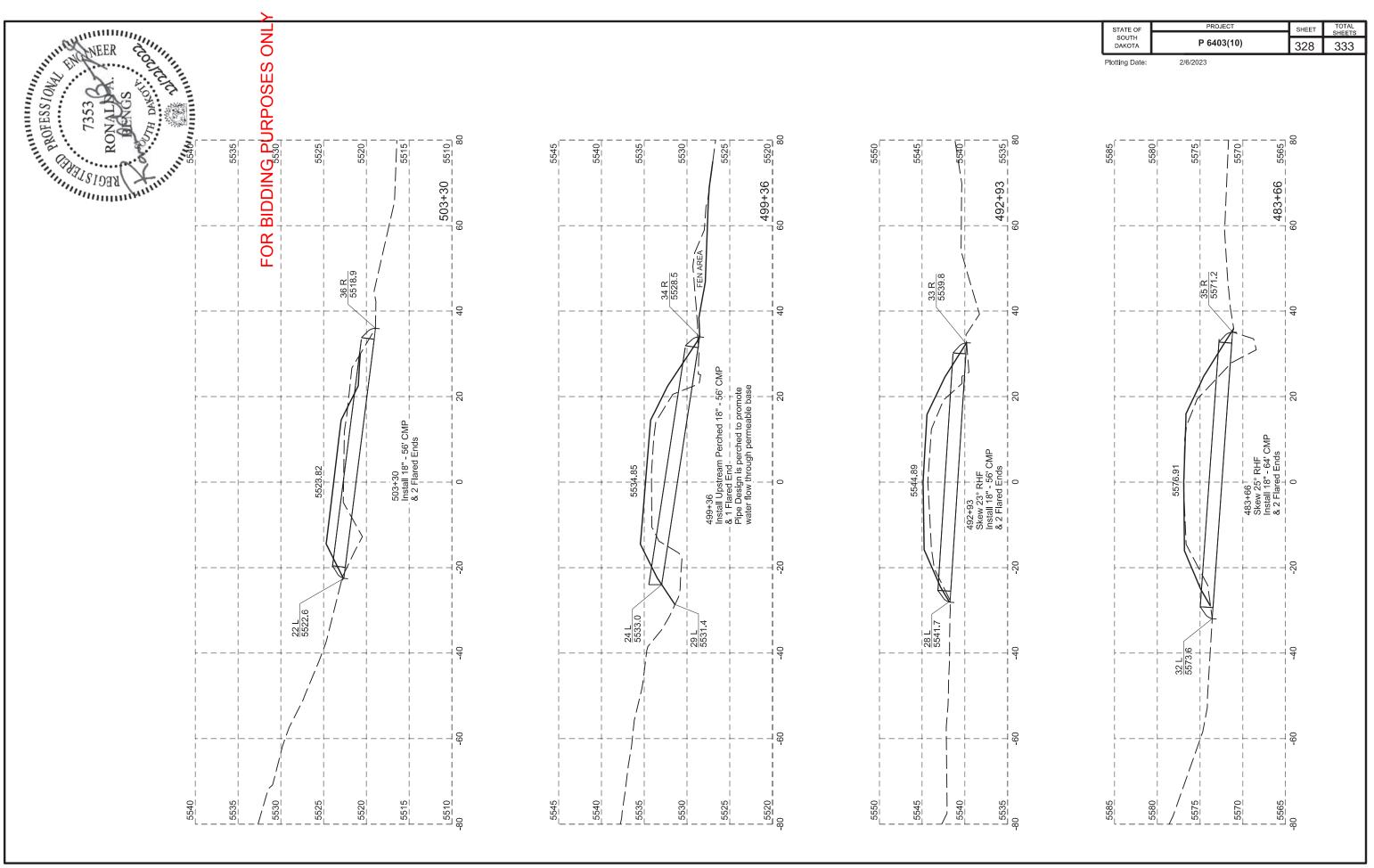






REGISTER TO THE DIENT OF THE DI STATE OF SHEET FOR BIDDING PURPOSES ONL SOUTH P 6403(10) 326 333 Plotting Date: 2/6/2023 435+83 424+30 408+47 38 R 5733.3 29 R 5767.8 25 R 5713.4 20 20 424+30 Skew 8° RHF Install 18" - 58' CMP ⁻ & 2 Flared Ends - 42' CMP Ends---57,17,03 5740.71 435+83 Install 18"|-& 2 Flared 21 L 5715.7 25 L 5736.9 32 L 5769.9 5725 5750 5770





REGISTER TO THE DIENT OF THE DI STATE OF SHEET FOR BIDDING PURPOSES ONL SOUTH P 6403(10) 329 333 Plotting Date: 2/6/2023 53Z0_I 5380 5380 510+48 530+17 527+65 9 9 41 R 5365.37 41 R 5480.6 35 R 5385.39 40 20 20 510+48 | -Skew 36° LHF — — -Install 30" - 86' CMP & 2 Flared Ends 527+65 -46° Skew RHF ——— Install 36" - 66' CMP & 2 Flared Ends CMP 5485.81 5387.40 0 -20 -20 40 33 L 5364.24 41 L 5380.30 53 U 5477. 9-5480 5360 5385 5380

